



EROSION CONTROL PLAN REVIEW FOR LAND DEVELOPMENT

By: Justin Hasenfus, CPESC

WHY DO WE REVIEW PLANS?

Sedimentation Pollution

Control Act:

Purpose: "...permit development of this State to continue with the least detrimental effects from pollution by sedimentation."





AGENDA

Part 1

- Responsibilities
- Requirements
- Tools
- Pointers

Part 2

- Application
- Design Standards*
- Miscellaneous



*Per the NCDEQ Erosion and Sediment Control Planning and Design Manual

PART 1



RESPONSIBILITIES

Compliance

- Plan must comply with applicable local, state, and federal laws and regulations
- Plan must not allow violation
- Plan must be legal

Judgement

- Best approximation with least erosion
- Feasible
- Practical
- Economical

Consider

- The design professional
- The regulating agencies
- The grading contractor
- Property owners
- The general public

REQUIREMENTS: WHAT DOES THE SPCA REQUIRE?

§ 113A-51.

- Preconstruction meeting (subject to staff availability)

§113A-56.(C)

- NCDEQ has concurrent jurisdiction with local programs

§ 113A-57.

- Require erosion control measures to retain sediment within boundaries of tract(s) of land to be disturbed
- Graded slope requirements
- Require permanent ground cover sufficient to restrain erosion



- Local government ordinance shall at least meet and may exceed the minimum requirements of SPCA
- Provisions for residential plans
- No erosion control measures required at high sides of project





REQUIREMENTS: WHAT DOES THE SPCA REQUIRE?

§ 113A-61.

- Review timeframe (15,30 day) →
- Approve, disapprove, or approve with modifications
- Grant conditional approval for environmental and other development approvals & permits
- Disapproval for buffer authorizations allowed
- Disapproval based on compliance record allowed
- Person submitting plan entitled to public hearing to appeal

Review Timeframe Guidance

PLAN REVIEWS

1. **Plans are to be reviewed and a decision rendered to the applicant within 30 calendar days of receipt of a complete plan.** Unlike with other permits, there is no pausing this “review clock”. A complete plan is defined as follows:
 - a. An erosion and sediment control plan
 - b. Calculations in support of the design, if applicable
 - c. An authorized statement of financial responsibility
 - d. A letter of consent or permission letter from the landowner for the land-disturbance, if the landowner and the Financially Responsible Person/Party are not the same entity
 - e. Documentation of property ownership (e.g., deed)
 - f. Fees paid in full
2. **Plans are to be reviewed and a decision rendered to the applicant within 15 calendar days of receipt of a revised plan.** There is no pausing this “review clock” once it has started.

Reference: G.S. 113A-61(b)

TOOLS

- Local Ordinances
- Sedimentation Pollution Control Act
- NCDEQ Erosion & Sediment Control Planning and Design Manual
- Recorded Plats & Legal Instruments
- Reviewing Agencies
- Design Firm





TOOLS



WATERSHED PROTECTION DEPARTMENT
P.O. Box 548
Pittsboro, NC 27312

Phone: (919) 545-8344 • E-mail: justin.hasenfus@chathamcountync.gov • Website: chathamcountync.gov

Soil Erosion and Sedimentation Control Checklist for Plan Submittals

The following information should be provided for all erosion control plan submittals.
Mark NA for any items that are not applicable.

General Information

- ☐ Name of Project
- ☐ Name, address, and phone number of owner
- ☐ Registered Engineer's/Landscape's/Land Surveyor's Seal
- ☐ Registered Engineer's/Landscape's/Land Surveyor's name, address, phone
- ☐ Permit Application & Financial Ownership/Responsibility Form
- ☐ Deed(s) & easement documentation for all property to have disturbance
- ☐ Parcel numbers
- ☐ Zoning of property
- ☐ Total acres of all property to have disturbance
- ☐ Disturbed acres
- ☐ Land-disturbing Permit fee
- ☐ Plan review fee

Plan Features

- ☐ Vicinity Map (1"=2,000', Subdivision phase for residential plans)
- ☐ North arrow and scale
- ☐ Legend
- ☐ Property lines & adjoining property owners
- ☐ Lot lines and lot/building numbers
- ☐ Project narrative describing the nature & purpose of activity
- ☐ Existing and Proposed Contours
- ☐ Limits of disturbance (include all proposed soil disturbance and clearing; include utilities)
- ☐ Proposed disturbed areas shown on NRCS Soils map and USGS Topography map
- ☐ Existing roads, buildings, utilities, etc.
- ☐ Proposed roads, buildings, utilities, etc.
- ☐ Phased erosion control plans
- ☐ Construction sequence (site overall)
- ☐ Construction sequence (stream crossing)
- ☐ Drainage areas map
- ☐ Retaining Wall: (include grading and drainage, profiles, , geogrid, location and wall number)
- ☐ Rock outcrops, seeps, springs
- ☐ Wetlands
- ☐ 100-year floodplain limits and elevations
- ☐ Easements
- ☐ Streams, lakes, ponds, drainage ways, etc.
- ☐ Riparian buffers
- ☐ Borrow/Waste Areas
- ☐ Parking/staging/material storage areas
- ☐ Soil stockpile locations¹

1. Soil stockpile locations are required to be 50' from storm drain inlets, sediment basins, site perimeter sediment controls and surface waters. Silt fence is required on low sides.

Erosion and Sediment Control Measures

- ☐ Standard details for all measures
- ☐ Construction entrance(s)
- ☐ Silt fence (1/4 acre of drainage per 100 linear feet of silt fence, and not in concentrated flow)
- ☐ Silt fence rock outlets
- ☐ Temporary Diversion Ditches
- ☐ Check Dams
- ☐ Slope drains with inlet & outlet protection (primarily where slope drains enter basin)
- ☐ Inlet Protection
- ☐ Clean water conveyance(s) (where applicable)
- ☐ Concrete Washouts²
- ☐ Seed and sod specifications chart(s)
- ☐ Skimmer basins (dimensions, volume, and surface area)
- ☐ Size, type, and invert elevations for all storm sewer
- ☐ Rock bench for skimmers
- ☐ Ditch profiles with lining requirements & supporting calculations
- ☐ Permanent liners in channels/ditch lines
- ☐ Temporary liners in diversion ditches
- ☐ Energy Dissipaters (sizes, dimensions, lining requirements & supporting calculations)
- ☐ Label slopes greater than 3:1 and label required stabilization liner (e.g. SC150)
- ☐ Temporary stream crossing details and sequence
- ☐ Other proposed measures & necessary calculations

Other Requirements

- ☐ Design calculations for culverts, storm sewers, and channels
- ☐ Design calculations for sediment basins/traps
- ☐ Fill in drainage easement (channel evaluation/design)
- ☐ Stormwater plan (greater than 20,000 ft² disturbed), if applicable
- ☐ Floodplain development permit, if applicable
- ☐ Permit and flood study required (work in floodway), if applicable
- ☐ Army Corp. of Engineers Section 404 Permit, if applicable
- ☐ NCDEQ 401 Certification, if applicable
- ☐ Chatham County/Pittsboro Planning (Buffer authorizations, construction plan approval, etc.), if applicable
- ☐ NCDOT permits and approvals, if applicable
- ☐ NCG01 Ground Stabilization and Materials Handling Plan Sheet from NCDEQ webpage (projects subject to NPDES)
- ☐ NCG01 Inspection, Recordkeeping and Reporting Plan Sheet from NCDEQ webpage (projects subject to NPDES)

NCDEQ's webpage with NCG01 updates:

<https://deq.nc.gov/NCG01>

Chatham County Erosion Control Standard Details:

<https://www.chathamcountync.gov/government/departments-programs-i-z/watershed-protection/erosion-control/standard-details>

Chatham County GIS:

<https://chathamncgis.maps.arcgis.com/apps/webappviewer/index.html?id=65367d5f69774726828390a90e5cde1e>

2. Concrete washouts are required to be 50' from storm drain inlets and surface waters.



TOOLS

NCG01 Permit Checklist (pages 7-9)

Permit No. NCG010000

PART II – STORMWATER POLLUTION PREVENTION PLAN

The Stormwater Pollution Prevention Plan (SWPPP) for this permit shall include the approved **Erosion and Sedimentation Control (E&SC) Plan** as well as any requirements in this Part that exceed the approved E&SC Plan. Items that are required in the SWPPP but are not part of the approved E&SC Plan may include, at a minimum, Section F, Item (1) [*Ground Stabilization Timelines*], Section F [*Materials Handling*]. DEMLR provides two sample plan sheets that permittees may add to their E&SC Plan set to fulfill Sections E (1) and F of this permit at <https://dec.nc.gov/NCG01>. [NCGS 113A– 57, 15A NCAC 04b .0107]

Recommendations for preparing the E&SC plan as well as for designing, constructing, and maintaining the erosion and sedimentation control practices are contained in the *North Carolina Erosion and Sediment Control Planning and Design Manual*.

SECTION A: REQUIRED COMPONENTS OF THE EROSION AND SEDIMENTATION CONTROL PLAN

The E&SC Erosion and Sedimentation Control Plan shall include, at a minimum, the following components and **those components shall be in compliance with all conditions of this permit**. Hard and/or digital copies shall be submitted in accordance with the specifications of the E&SC plan authority. [15A NCAC 04b .0107]

1. Location Information

- Project location & labeled vicinity map (roads, streets, landmarks)
 - North arrow and scale
 - Identification of the River Basin
 - A copy of site disturbed area located on applicable USGS quadrangle and hardbound copy of the NRCS Soils maps to scale
 - Latitude and longitude (in decimal degrees) at the project entrance
- [NCGS 113A– 57, 15A NCAC 04b .0107]

2. Narrative and Construction Sequence

- Narrative describing the nature & purpose of the construction activity
 - Construction sequence related to erosion and sediment control (including installation of critical measures prior to the initiation of the land-disturbing activity & removal of measures after areas they serve are permanently stabilized). Address all phases of construction and necessary practices associated with temporary stream bypasses and/or crossings
 - Estimated start and end dates
- [NCGS 113A– 57, 15A NCAC 04b .0107]

3. General Site Features

- Property lines
- Existing and proposed contours (topographic lines)
- Stockpiled topsoil or subsoil locations
- Limits of disturbed area (with acreage labeled) within which all construction, material storage, grading, and related activities occur, including the following items as applicable:

Permit No. NCG010000

- Access to E&SC measures, lots that will be disturbed, and utilities that may extend offsite.
- Temporary access and haul roads, other than public roads, constructed or used in connection with the land-disturbing activity
- Borrow and waste areas created by the applicant. If the land-disturbing activity and any related borrow or waste activity are not conducted by the same person, they shall be considered separate land-disturbing activities
- Offsite borrow pits if the borrow pit is a construction support activity to the development

- Planned and existing building locations and elevations, if applicable
 - Planned & existing road locations & elevations, including temporary access roads, if applicable
 - Profiles of streets, utilities, and permanent ditch lines, if applicable
 - Lot lines and/or building numbers, if applicable
 - Easements and drainageways, particularly required for offsite affected areas, if applicable
 - Location and details associated with any onsite stone crushing or other processing of material excavated, if applicable. A mining permit will be required if the affected area associated with excavation, processing, stockpiles and transport of such materials comprises one or more acres, and materials will be leaving the development tract
- [NCGS 113A– 57, 15A NCAC 04b .0107]

4. Site Drainage Features

- Existing and planned drainage patterns (include off-site areas that drain through project and address temporary and permanent conveyance of stormwater over graded slopes)
 - Drainage area map
 - Surface waters, including the limits of wetlands, streams, lakes and ponds and all required local or state buffer zones as well as impact maps by the construction activity to these sensitive areas.
 - Method used to determine acreage of land being disturbed and drainage areas to all proposed E&SC measures (e.g. delineation map)
 - Size, pipe material and location of culverts and sewers
 - Soil information throughout the site and below culvert storm outlets, including soil type and special characteristics
 - Name and classification of receiving water course where discharges are to occur
- [NCGS 113A– 57, 15A NCAC 04b .0107]

5. Plans Showing E&SC Measures

- Legend (provide appropriate symbols for all measures and reference them to the construction details)
- Location of temporary and permanent E&SC measures
- Location of permanent stormwater quality and quantity control measures
- Construction drawings and details for temporary and permanent measures, including outlet structures. Show measures to scale on plan and include proposed contours where necessary. Ensure design storage requirements are maintained through all phases of construction.
- Specifications for ground stabilization
- Maintenance requirements for measures
- Contact person responsible for maintenance, if the permittee wishes to designate one. If not, the financially responsible organization will be the contact for maintenance.

Permit No. NCG010000

- A note stating that material handling procedures for the items required in Part II, Section F will be followed.
 - Standard details for structural BMPs to be installed to manage the anticipated materials listed in Part II, Section F such as construction debris management, concrete washout, paint washout, petroleum product storage and pesticide/herbicide handling, along with spill prevention practices.
- [NCGS 113A– 57, 15A NCAC 04b .0107]

6. Calculations

- Calculations for peak discharges of runoff from each outlet at pre-development, during construction and at completion. Provide all supporting data for the computation methods used (rainfall data for required storm events, time of concentration/storm duration, and runoff coefficients).
 - Design calculations for culverts and storm sewers (include headwater, tailwater and outlet velocities)
 - Discharge and velocity calculations for open channel and ditch flows (easement & rights-of-way)
 - Design calculations for cross sections and method of stabilization for existing and planned channels (include temporary linings). Include appropriate permissible velocity and shear stress.
 - Design calculations and construction details for energy dissipaters below culvert and storm sewer outlets (include stone/material specs & apron dimensions). Avoid discharges on fill slopes.
 - Design calculations and dimensions for sediment traps and basins
 - Total and disturbed drainage areas for silt fencing and other sediment controls
- [NCGS 113A– 57, 15A NCAC 04b .0107]

7. Vegetative Stabilization Shown on Plans

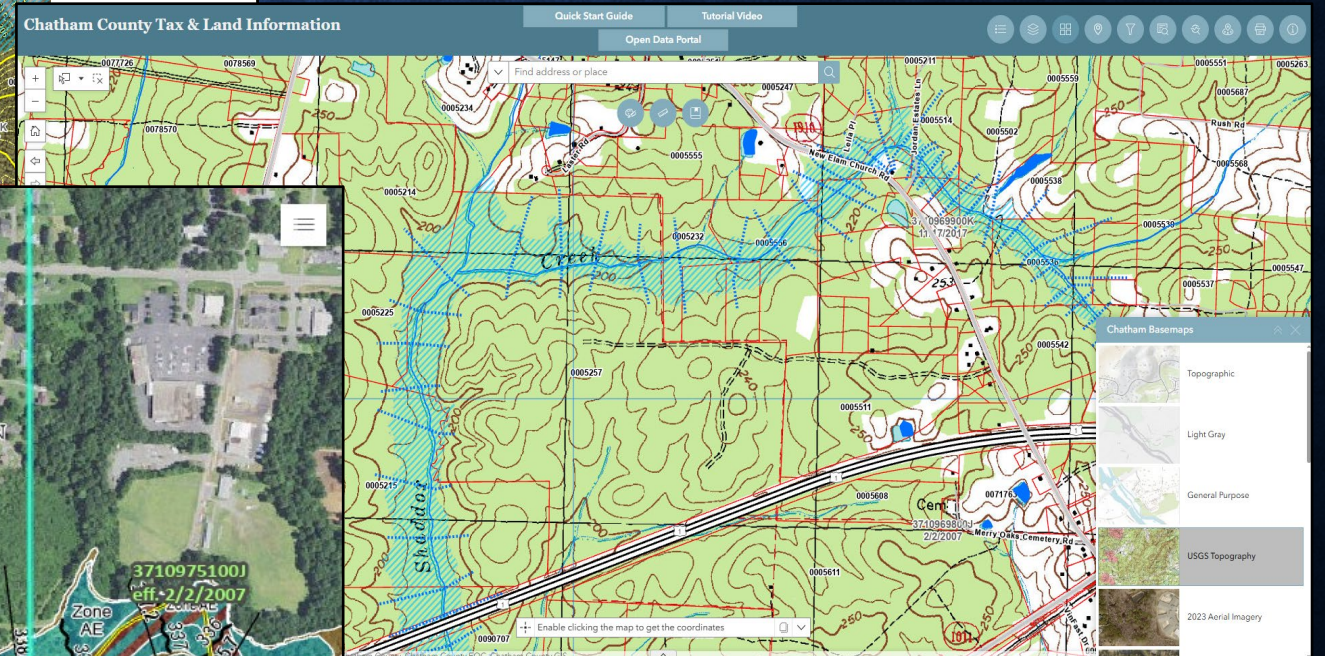
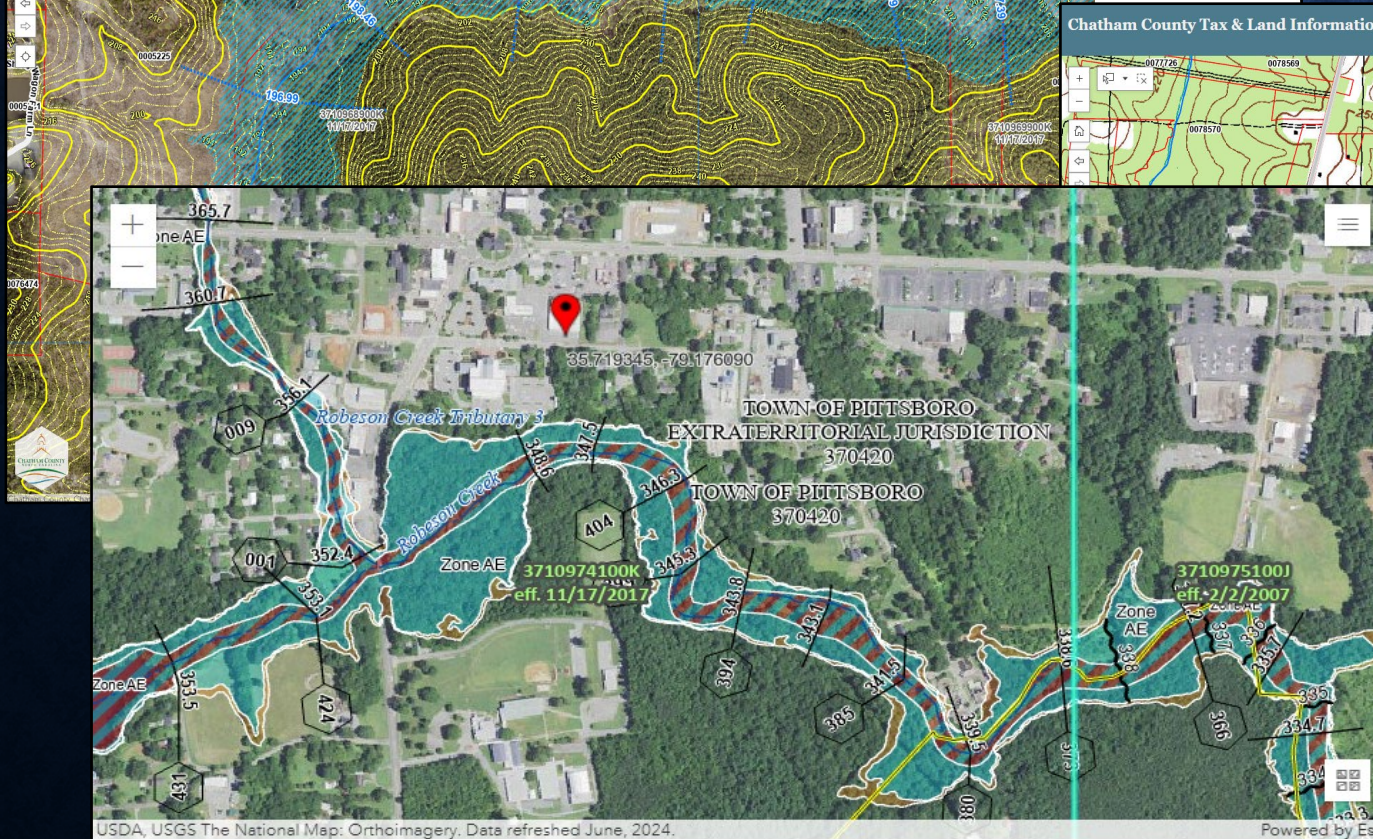
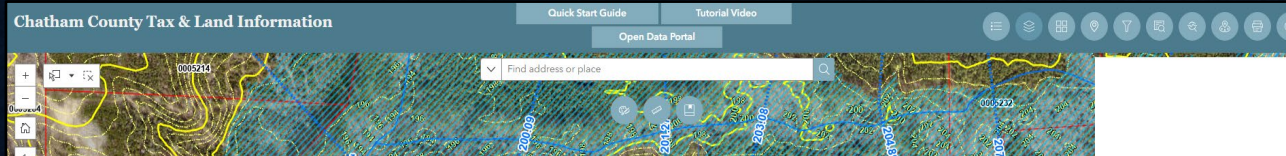
- Area & acreage to be stabilized with vegetation
 - Method of soil preparation
 - Seed type & rates (temporary & permanent)
 - Fertilizer type and rates
 - Mulch type and rates (include mulch anchoring methods)
- [NCGS 113A– 57, 15A NCAC 04b .0107]

8. Documentation

- Properly executed **Financial Responsibility/Ownership Form**
 - Certificate of assumed name, if the owner is a partnership
 - Name of Registered Agent (if applicable)
 - Copy of the most current Deed for the site. Please make sure the deed(s) and ownership information are consistent between the plan sheets, local records and this form.
 - Provide latitude & longitude (in decimal degrees) at the project entrance.
 - DWR Buffer Authorization, if required for project
 - Copies of any recorded easements and/or agreements with adjoining property owners for landlocked parcels
- [NCGS 113A– 57, 15A NCAC 04b .0107]



TOOLS



POINTERS

DO:

- Have hard copy/electronic files
- Allocate time appropriately
- Say what you mean and explain why
- Ask for more information
- Provide guidance where necessary
- Pick your battles
- Cite comments/Provide comment response
- Consider new measures or approaches



POINTERS

DON'T:

- Make large assumptions
- Skip reading ordinances or submittal requirements
- Forget to account for changes since last submittal
- Ever use measures in surface waters other than those that are allowed
- Get mad



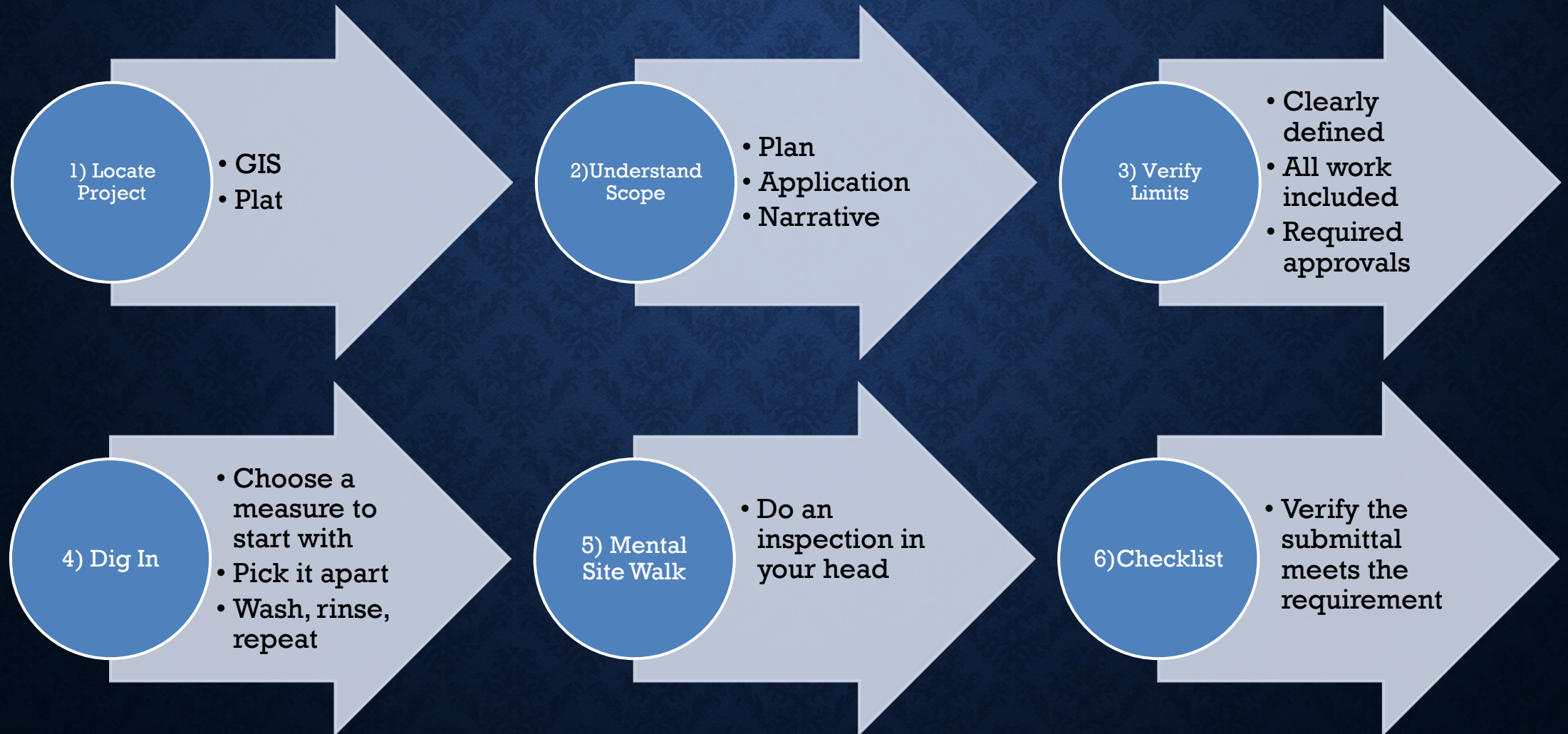
POINTERS

IN GENERAL:

- Find a process that works
- Catch the big stuff
- Focus your time on what's most important
- Your goal is to produce a plan that is compliant and sufficient



POINTERS




PART 2



APPLICATION

Check It

- All information present & correct
- NC Agent complete when required
- NC Secretary of State
- Authorized statement of financially responsibility & ownership
- Owner/FRP agreement

 **Land-Disturbing Permit Application**
Apply Online: <https://chathamcountync.viewpointcloud.com/categories/1082>

PROJECT NAME: _____ DATE: _____
ADDRESS OF PROJECT: _____
LATITUDE/LONGITUDE OF PROPERTY AT SITE ENTRANCE: _____
PARCEL #(S): _____
TOTAL DISTURBED ACRES or SQUARE FEET: _____
PURPOSE OF ACTIVITY: _____
FEE AMOUNT SUBMITTED: _____ RIVER BASIN: CAPE FEAR
ANTICIPATED START DATE: _____ ANTICIPATED END DATE: _____

***LANDOWNER(S) OF RECORD (attach page to list additional owners)**
Please provide a complete list of partners, managing members and registered agents if the responsible entity or landowner is a group of individuals, corporate organization or entity.
Name: _____ Phone: _____
Address: _____
E-Mail: _____ Signature: _____


***FINANCIALLY RESPONSIBLE PARTY (applicable only if different from property owner)**
Name: _____ Phone: _____
Address: _____
E-Mail: _____ Signature: _____

NORTH CAROLINA AGENT (applicable only if owner or financially responsible party does not reside in North Carolina)
Name: _____ Phone: _____
Address: _____
E-Mail: _____

ENGINEER/SURVEYOR	EROSION CONTROL
Company Name: _____	Person to contact should erosion & sediment control issues arise during land-disturbing activity:
Address: _____	Contact Person: _____
Contact Person: _____	Company Name: _____
Phone: _____	Phone: _____
E-Mail: _____	E-Mail: _____

*The mailing and street address of the principal place of business for the person/entity financially responsible and the landowner(s) must be provided. A P.O. box is NOT acceptable as an address.
*If the financially responsible party is different from the current landowner, an agreement signed by both parties must be provided allowing the financially responsible party to conduct the land-disturbing activity on the property. This agreement is provided on page 3 of this application and must be completed in its entirety.

Page 2 of 3

 **Soil Erosion and Sedimentation Control Financial Responsibility/Ownership Form**

PLEASE READ THE FOLLOWING INFORMATION:

1) This section must be signed in the presence of a Notary.
2) This form must be signed by the property owner if an individual. If owned by a company or corporation, this form must be signed by an officer, director, partner, attorney-in-fact, or other person with authority to execute instruments for the corporation and accompanied by a complete list of all partners, managing members and registered agents of the company or corporation.
3) This form must also be signed by the financially responsible party. Same provisions of # 2 above apply.
4) If the landowner and financially responsible party are different, the completion and signing of this page shall serve as documentation acknowledging the landowner consents to and authorizes the financially responsible party to undertake the proposed land-disturbing activity on the landowner's tract(s) of land identified on this application.
5) By signing this form, I agree to receive erosion control plan review letters electronically at the email address provided on the application.
6) The information provided on this form is true and correct to the best of my knowledge and belief and was provided by me while under oath.
7) All Land-Disturbing permits are valid for up to (2) years from the date of issuance. If circumstances warrant, the permit may be extended for (2) years per the conditions of the Chatham County Soil Erosion and Sedimentation Control Ordinance. Upon written notice, the Land-Disturbing permit may be revoked for failure to comply with the Ordinance. If the permit is revoked, all other permits and approvals are withheld until the property is once again in compliance with Chatham County regulations. Also, upon written notice, a civil penalty (fine) can be instigated against the property owner and/or additional financially responsible party (if any) for violations of the Chatham County Soil Erosion and Sedimentation Control Ordinance. This penalty is up to \$5000.00 per violation per day and is assessed daily for every day the property is in violation. Interfering with or hampering an inspection can result in a civil penalty without written notice.

OWNER OF PROPERTY:
Name and Title: _____
Company (if applicable): _____
Signature: _____

ADDITIONAL FINANCIALLY RESPONSIBLE PARTY (if any):
Name and Title: _____
Company: _____
Signature: _____

I, _____, a Notary Public of _____ County in the state of _____ do hereby certify that _____ personally appeared before me this day and under oath acknowledged reading the information above and acknowledged that the above form was executed by him or her.
Witness my hand and official seal, this the _____ day of _____, 20____

Notary Public
My commission expires: _____ (SEAL)

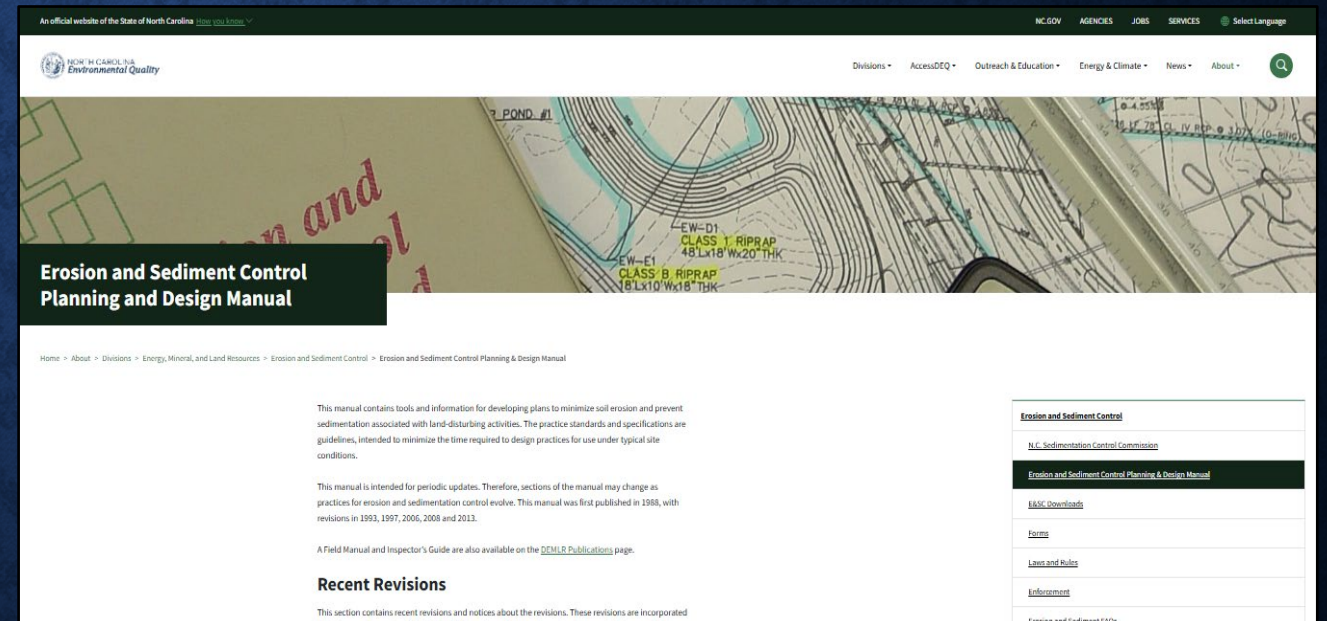
Page 3 of 3



DESIGN STANDARDS

Sources

- Local Ordinance
- NCDEQ Manual
- Other supported documents and practices of sound engineering



DESIGN STANDARDS

BASINS

Sediment Basin (Riser Basin)

Design Criteria	<u>Summary:</u>	<u>Temporary Sediment Basin:</u>
Primary Spillway:		Riser/Barrel Pipe
Maximum Drainage Area:		100 acres
Minimum Sediment Storage Volume:		1800 cubic feet per acre of disturbed area
Minimum Surface Area:		435 square feet per cfs of Q_{10} peak inflow
Minimum L/W Ratio:		2:1
Maximum L/W Ratio:		6:1
Minimum Depth:		2 feet
Dewatering Mechanism:		Skimmer(s) attached at bottom of riser pipe or flashboard riser
Minimum Dewatering Time:		48 hours
Baffles Required:		3 baffles*
(*Note: Basins less than 20 feet in length may use 2 baffles.)		



Sediment Skimmer Basin

Design Criteria	<u>Summary:</u>	<u>Skimmer Sediment Basin</u>
Primary Spillway:		Trapezoidal spillway with impermeable membrane
Maximum Drainage Area:		10 acres
Minimum Volume:		1800 cubic feet per acre of disturbed area
Minimum Surface Area:		325 square feet per cfs of Q_{10} peak inflow
Minimum L/W Ratio:		2:1
Maximum L/W Ratio:		6:1
Minimum Depth:		2 feet
Dewatering Mechanism:		Skimmer
Minimum Dewatering Time:		2 days
Baffles Required:		3 baffles*
(*Note: Basins less than 20 feet in length may use 2 baffles.)		

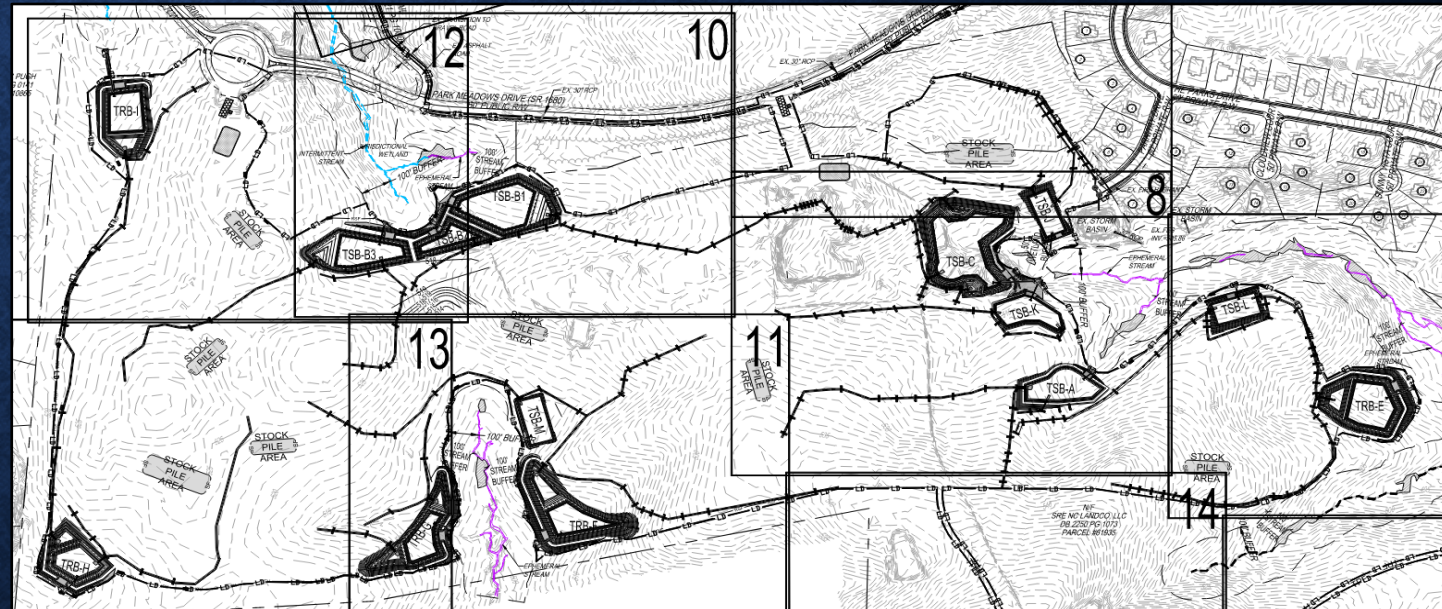


DESIGN STANDARDS

BASINS

Location

- Locate in low areas
- Capture majority of disturbed area
- Interfere minimally w/construction
- Allows access for sediment removal
- Never locate in surface waters



DESIGN STANDARDS

BASINS

➤ Surface Area: “Shape of the container”



➤ Volume: “What’s inside the container”





DESIGN STANDARDS

BASINS

Surface Area Required = $Q \times 435^*$

$Q = CIA$

Q = peak rate of runoff (cfs)

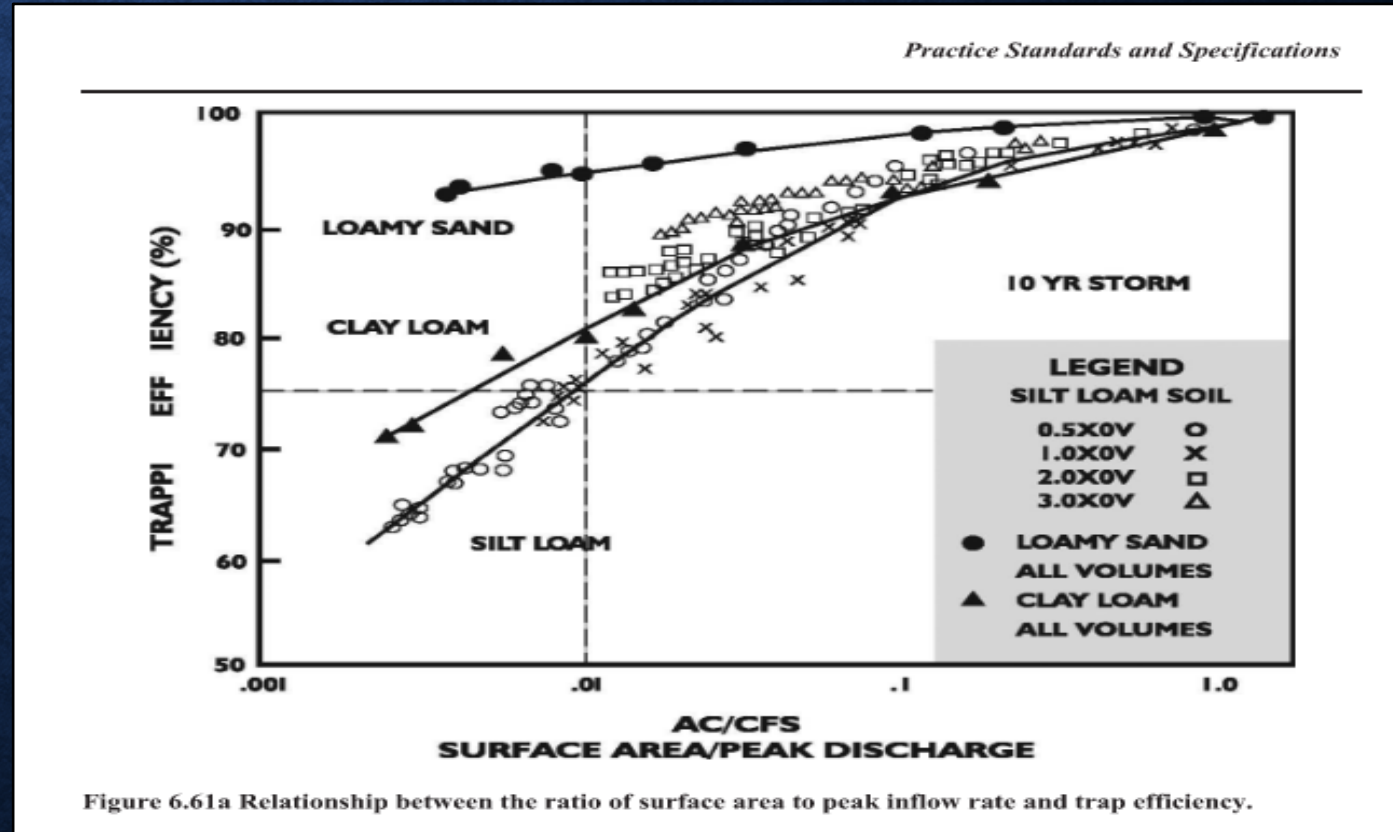
C = runoff coefficient

I = avg. intensity of rainfall (in/hr)

A = drainage area (acres)

*Skimmer Basin: Minimum 325 sq.ft./cfs of Q_{10} peak inflow

*Riser Basin = Minimum 435 sq.ft./cfs of Q_{10} peak inflow



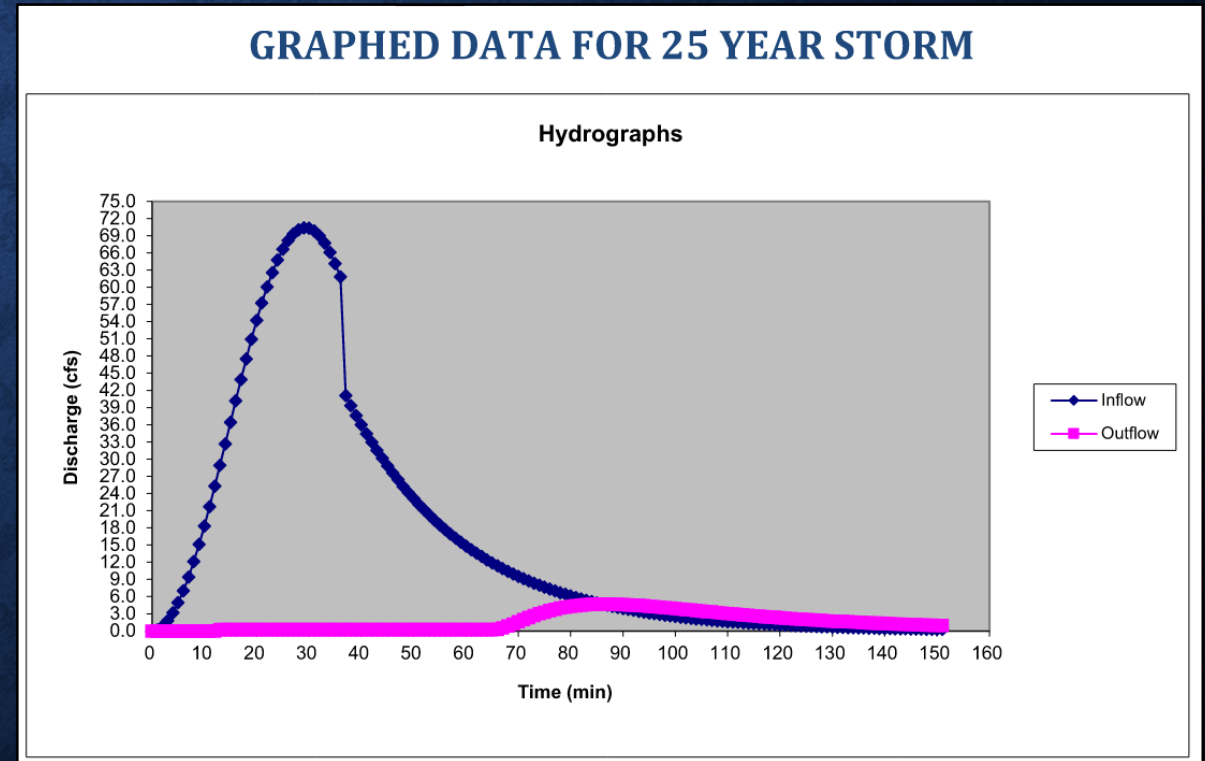


DESIGN STANDARDS

BASINS

Peak rate of runoff (Q)

- Maximum rate of water flow passing a specific point for the design storm
- Calculated using:
 - Rational method or
 - NRCS method
- Rational method is $Q = CIA$ and primarily used for drainage area < 50 acres
- NRCS more accurate for areas > 20 acres



DESIGN STANDARDS

BASINS

Runoff coefficient (C)

- Relationship between rainfall rate and runoff rate
- Determined by soil type/land cover in the drainage area
- Table of values in DEQ manual or ordinance
- Land use and soil cover homogenous in drainage area = use C value from table
- Land use and soil cover NOT homogenous, a weighted C value is calculated

8

Table 8.03b
Value of Runoff Coefficient
(C) for Rational Formula

Land Use	C	Land Use	C
Business:		Lawns:	
Downtown areas	0.70-0.95	Sandy soil, flat, 2%	0.05-0.10
Neighborhood areas	0.50-0.70	Sandy soil, ave., 2-7%	0.10-0.15
Residential:		Sandy soil, steep, 7%	0.15-0.20
Single-family areas	0.30-0.50	Heavy soil, flat, 2%	0.13-0.17
Multi units, detached	0.40-0.60	Heavy soil, ave., 2-7%	0.18-0.22
Multi units, Attached	0.60-0.75	Heavy soil, steep, 7%	0.25-0.35
Suburban	0.25-0.40	Agricultural land:	
Industrial:		Bare packed soil	
Light areas	0.50-0.80	Smooth	0.30-0.60
Heavy areas	0.60-0.90	Rough	0.20-0.50
Parks, cemeteries	0.10-0.25	Cultivated rows	
Playgrounds	0.20-0.35	Heavy soil no crop	0.30-0.60
Railroad yard areas	0.20-0.40	Heavy soil with crop	0.20-0.50
Unimproved areas	0.10-0.30	Sandy soil no crop	0.20-0.40
Streets:		Sandy soil with crop	0.10-0.25
Asphalt	0.70-0.95	Pasture	
Concrete	0.80-0.95	Heavy soil	0.15-0.45
Brick	0.70-0.85	Sandy soil	0.05-0.25
Drives and walks	0.75-0.85	Woodlands	0.05-0.25
Roofs	0.75-0.85		

NOTE: The designer must use judgement to select the appropriate C value within the range for the appropriate land use. Generally, larger areas with permeable soils, flat slopes, and dense vegetation should have lowest C values. Smaller areas with slowly permeable soils, steep slopes, and sparse vegetation should be assigned highest C values.

Source: American Society of Civil Engineers



DESIGN STANDARDS

BASINS

Rainfall Intensity (I)

- Minimum design storm = 10 year
- Average intensity (in/hr) for storm duration equal to time of concentration
- Time of concentration calculation should be provided
- Data source included with submittal

NOAA Atlas 14, Volume 2, Version 3
Location name: Chapel Hill, North Carolina, USA*
Latitude: 35.7617°, Longitude: -79.0596°
Elevation: 365 ft**

* source: ESRI Maps

** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps & aeriels](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)¹

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	4.98 (4.56-5.45)	5.86 (5.36-6.42)	6.76 (6.18-7.39)	7.45 (6.80-8.15)	8.20 (7.45-8.94)	8.72 (7.90-9.52)	9.19 (8.28-10.0)	9.60 (8.60-10.5)	10.0 (8.92-11.0)	10.4 (9.14-11.4)
10-min	3.98 (3.64-4.35)	4.69 (4.29-5.13)	5.41 (4.95-5.92)	5.96 (5.45-6.51)	6.53 (5.94-7.13)	6.95 (6.29-7.58)	7.31 (6.58-7.97)	7.61 (6.82-8.32)	7.94 (7.05-8.68)	8.18 (7.21-8.95)
15-min	3.31 (3.03-3.62)	3.92 (3.60-4.30)	4.56 (4.17-4.99)	5.02 (4.59-5.49)	5.52 (5.02-6.02)	5.86 (5.31-6.40)	6.16 (5.54-6.72)	6.40 (5.73-6.99)	6.67 (5.92-7.28)	6.85 (6.03-7.49)
30-min	2.27 (2.08-2.48)	2.71 (2.48-2.97)	3.24 (2.96-3.55)	3.64 (3.33-3.98)	4.09 (3.72-4.46)	4.41 (4.00-4.82)	4.71 (4.25-5.14)	4.98 (4.46-5.44)	5.30 (4.71-5.79)	5.55 (4.88-6.07)
60-min	1.42 (1.30-1.55)	1.70 (1.56-1.86)	2.08 (1.90-2.27)	2.37 (2.17-2.59)	2.72 (2.47-2.97)	2.99 (2.71-3.26)	3.25 (2.92-3.54)	3.49 (3.13-3.82)	3.80 (3.38-4.16)	4.05 (3.56-4.43)
2-hr	0.833 (0.759-0.919)	1.00 (0.916-1.11)	1.24 (1.13-1.37)	1.43 (1.29-1.57)	1.66 (1.50-1.83)	1.85 (1.66-2.03)	2.03 (1.81-2.23)	2.21 (1.96-2.42)	2.44 (2.14-2.68)	2.63 (2.29-2.90)
3-hr	0.589 (0.538-0.649)	0.710 (0.650-0.783)	0.880 (0.803-0.969)	1.02 (0.928-1.12)	1.20 (1.08-1.31)	1.34 (1.21-1.47)	1.49 (1.33-1.63)	1.64 (1.45-1.79)	1.83 (1.60-2.01)	2.00 (1.73-2.20)
6-hr	0.354 (0.325-0.388)	0.427 (0.392-0.468)	0.529 (0.484-0.580)	0.614 (0.560-0.672)	0.725 (0.657-0.792)	0.817 (0.736-0.892)	0.910 (0.812-0.992)	1.01 (0.889-1.10)	1.14 (0.990-1.24)	1.24 (1.07-1.36)
12-hr	0.208 (0.191-0.228)	0.250 (0.230-0.274)	0.312 (0.286-0.341)	0.364 (0.332-0.398)	0.434 (0.393-0.472)	0.493 (0.443-0.535)	0.553 (0.492-0.599)	0.616 (0.542-0.667)	0.704 (0.609-0.762)	0.779 (0.663-0.844)



DESIGN STANDARDS

BASINS

Volume Required	1800 cf/disturbed acre
Volume Recommended	1800 cf/drainage acre

Volume:

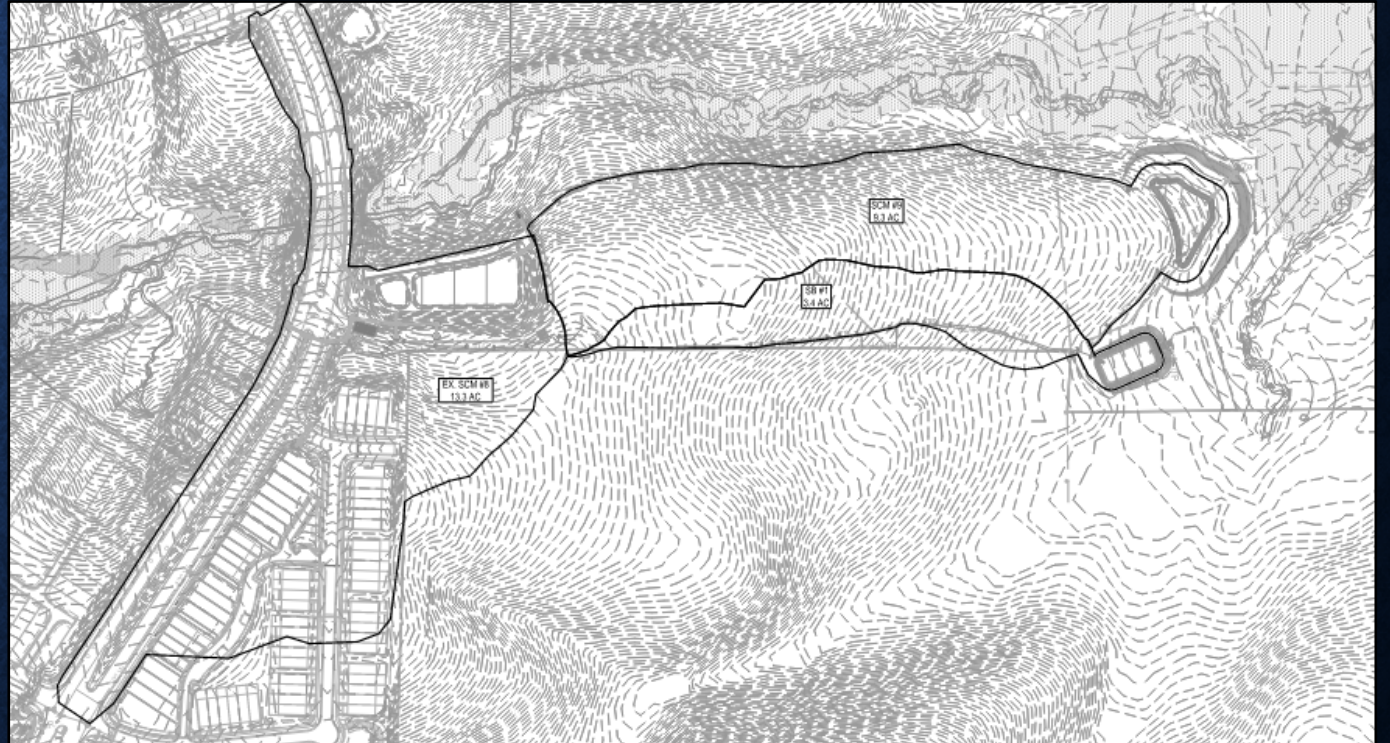
- Measured to the elevation of crest of principal spillway:
 - Riser basin = top of riser
 - Skimmer basin = emergency spillway

DESIGN STANDARDS

BASINS

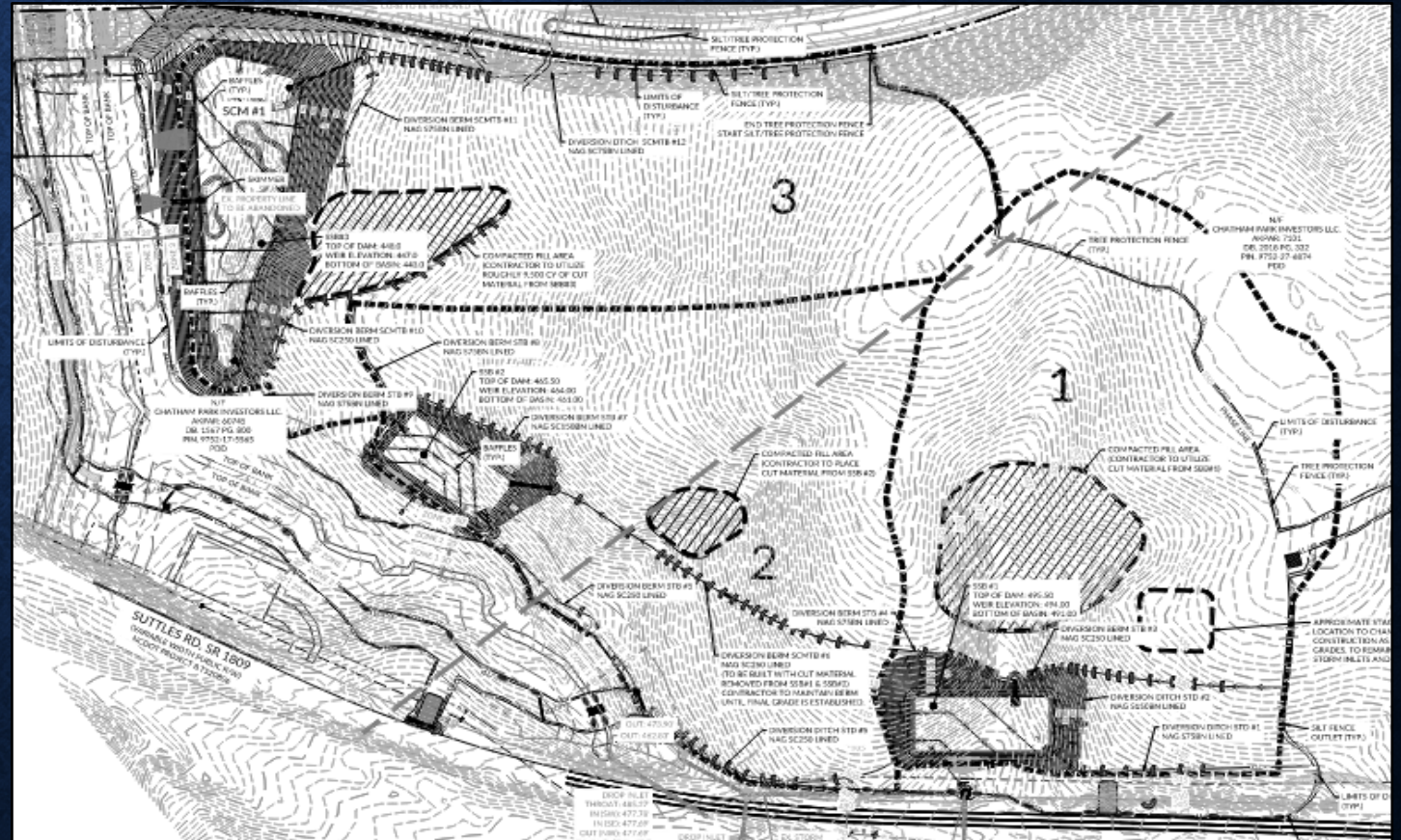
Drainage Area (A)

- Map included to confirm inputs
- Drainage area in acres
- Must include all drainage that would drain to basin



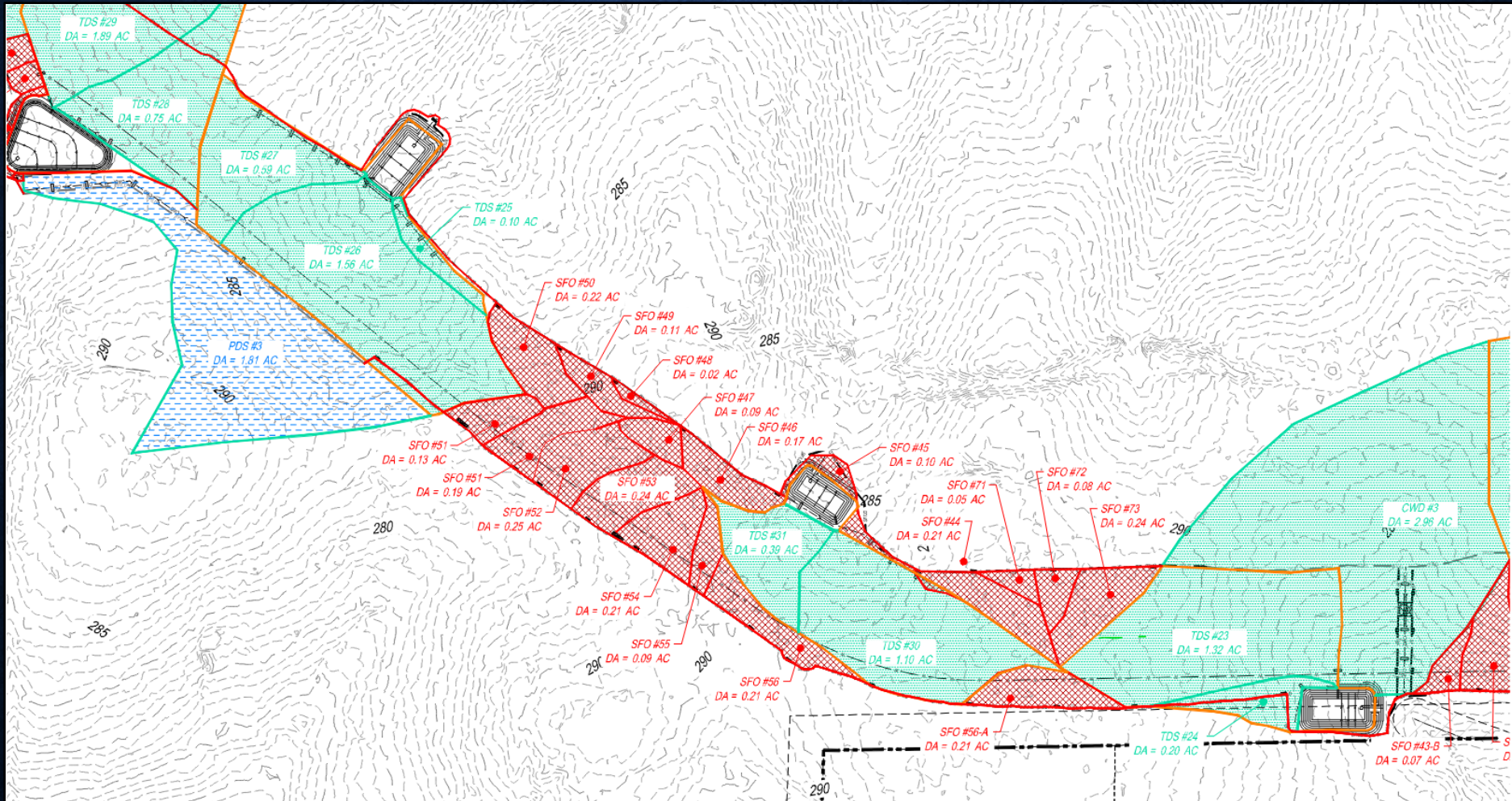


- Drainage values
- Contours
- Limits of disturbance
- Measures analyzed



DESIGN STANDARDS

BASINS



Drainage Maps for silt fence outlets to confirm all are < 0.25 acres / 100 LF silt fence outside basin drainage.



DESIGN STANDARDS

BASINS

TEMPORARY SKIMMER BASIN & ORIFICE DIAMETER CALCULATIONS

BASIN	TYPE	DRAINAGE AREA	DISTURBED AREA	C VALUE	PEAK FLOW	DEPTH	LENGTH	WIDTH	WEIR LENGTH	VOLUME REQUIRED	VOLUME PROVIDED	SURFACE AREA REQUIRED	SURFACE AREA PROVIDED	SKIMMER SIZE	ORIFICE SIZE	Dewatering Time
		(AC)	(AC)		(CFS)	(FT)	(FT)	(FT)	(FT)	(CF)	(CF)	(SF)	(SF)	(IN)	(IN)	(DAYS)
SSB #1	SKIMMER	9.99	9.99	0.50	41.26	4.5	SEE EC PLANS		38	35,964	42,517	17,948	19,014	4.0	3.25	3.02
SSB #2	SKIMMER	8.87	8.87	0.50	36.63	4.5	SEE EC PLANS		34	31,932	36,939	15,935	16,319	4.0	3	3.08
SSB #3	SKIMMER	9.97	9.97	0.50	41.18	4.0	SEE EC PLANS		38	35,892	97,108	17,912	51,871	5.0	4.5	3.22

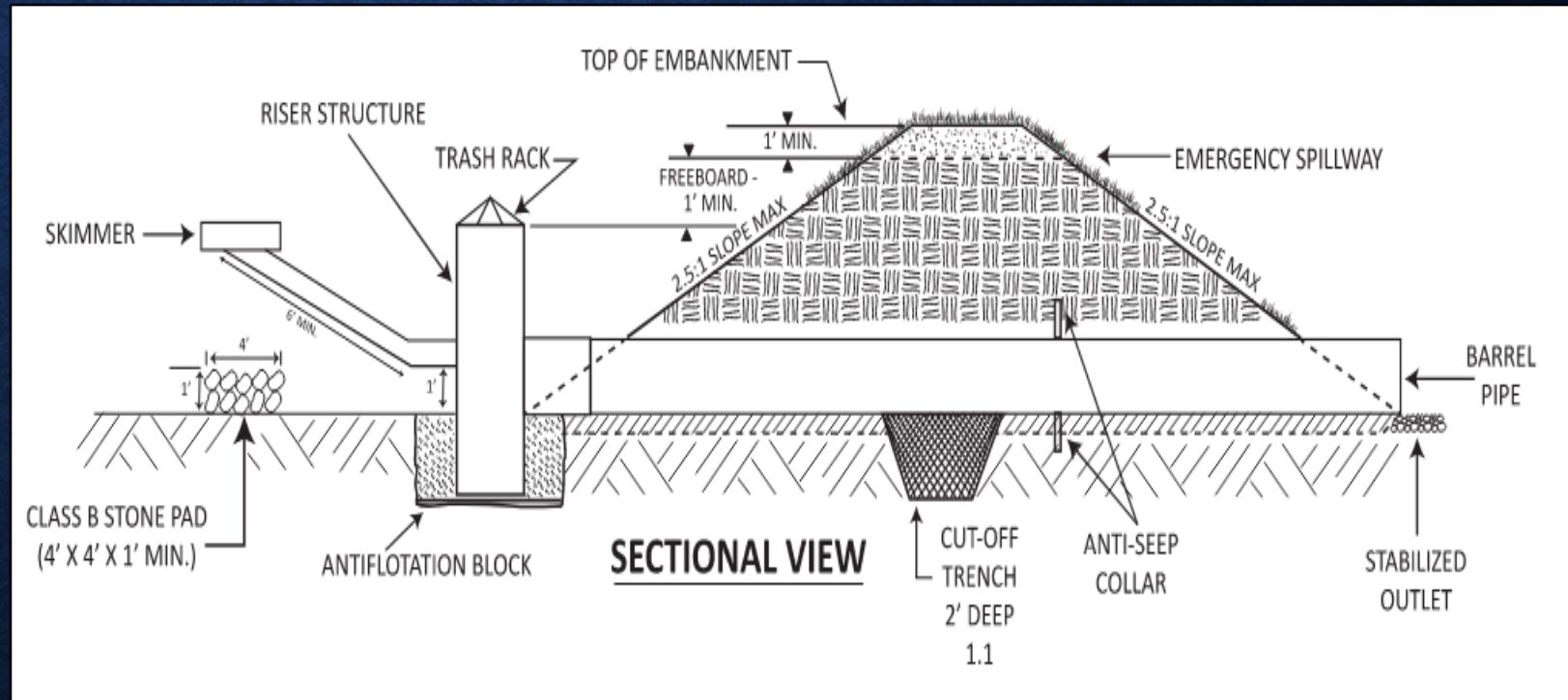
- The reviewer can ask for clear representation of design information
- This table should be in the plan or information otherwise called out

DESIGN STANDARDS

RISER BASINS

Risers:

- Design minimum 2-yr
- Riser profiles provided
- Protect against piping:
 - Watertight anti-seep collar
 - Filter diaphragm
- Protect against flotation
 - Anti-flotation block
- Cut-off trench

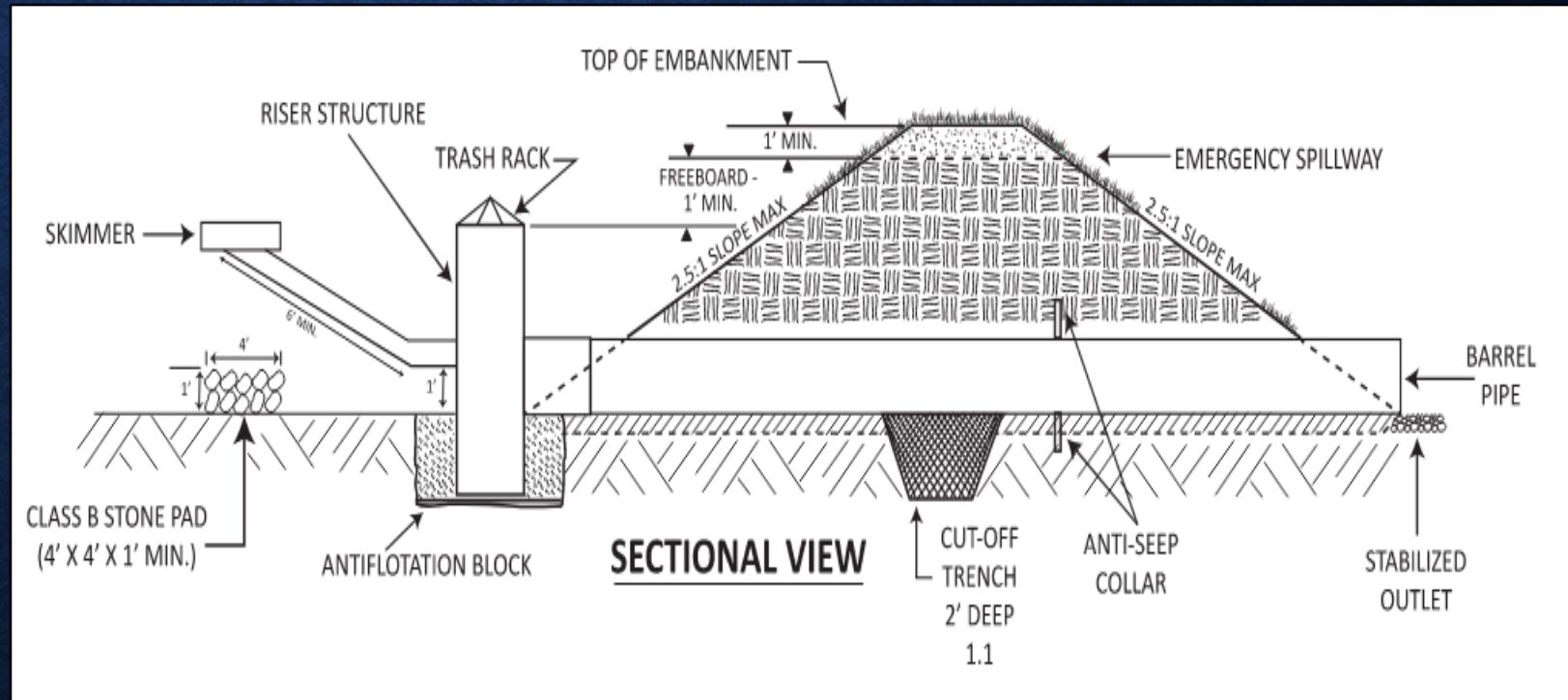


DESIGN STANDARDS

RISER BASINS

Embankment:

- Riser crest 1 ft. below emergency spillway crest
- Max Dam Height: 15 feet
- Top width:
 - Fill height < 10 ft = 8.0 ft
 - Fill height 10-15 ft = 10.0 ft
- Sideslopes 2.5:1 or flatter

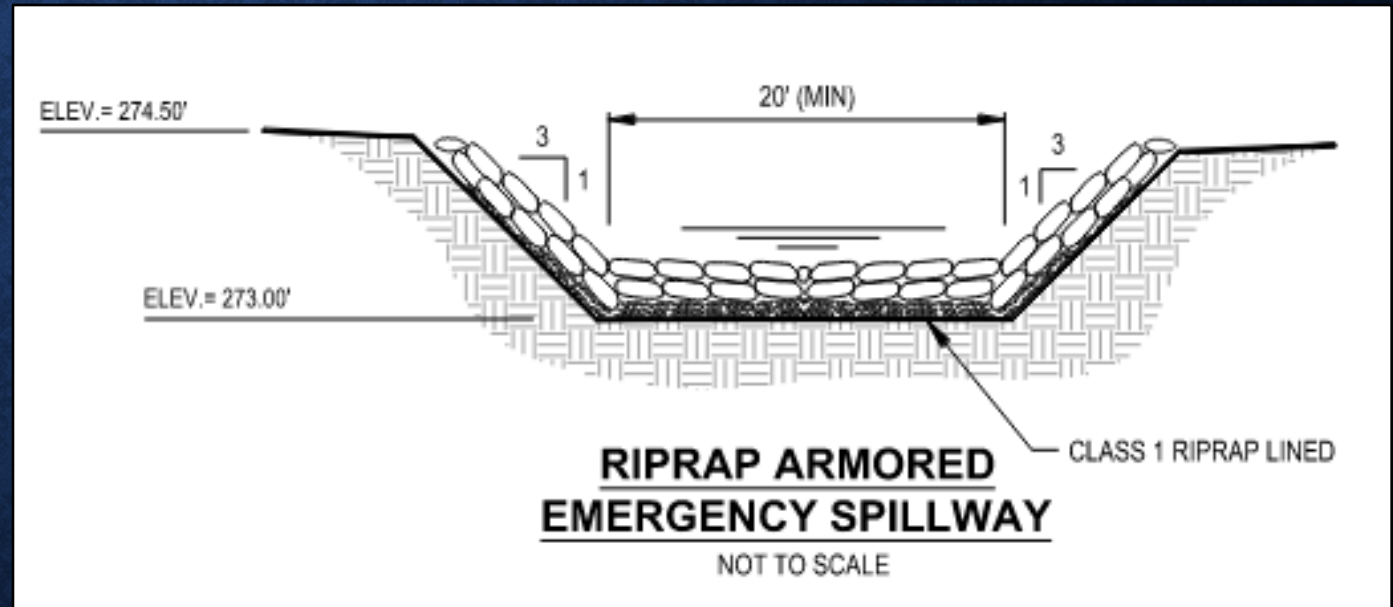


DESIGN STANDARDS

RISER BASINS

Emergency Spillway:

- Design minimum 10-yr
- Minimum 1 foot of freeboard
- Trapezoidal, sideslopes 3:1 or flatter
- Control section straight and at least 20 ft wide



DESIGN STANDARDS

RISER BASINS

Emergency Spillway:

- Stabilized appropriately
- Velocity of flow discharged non-erosive

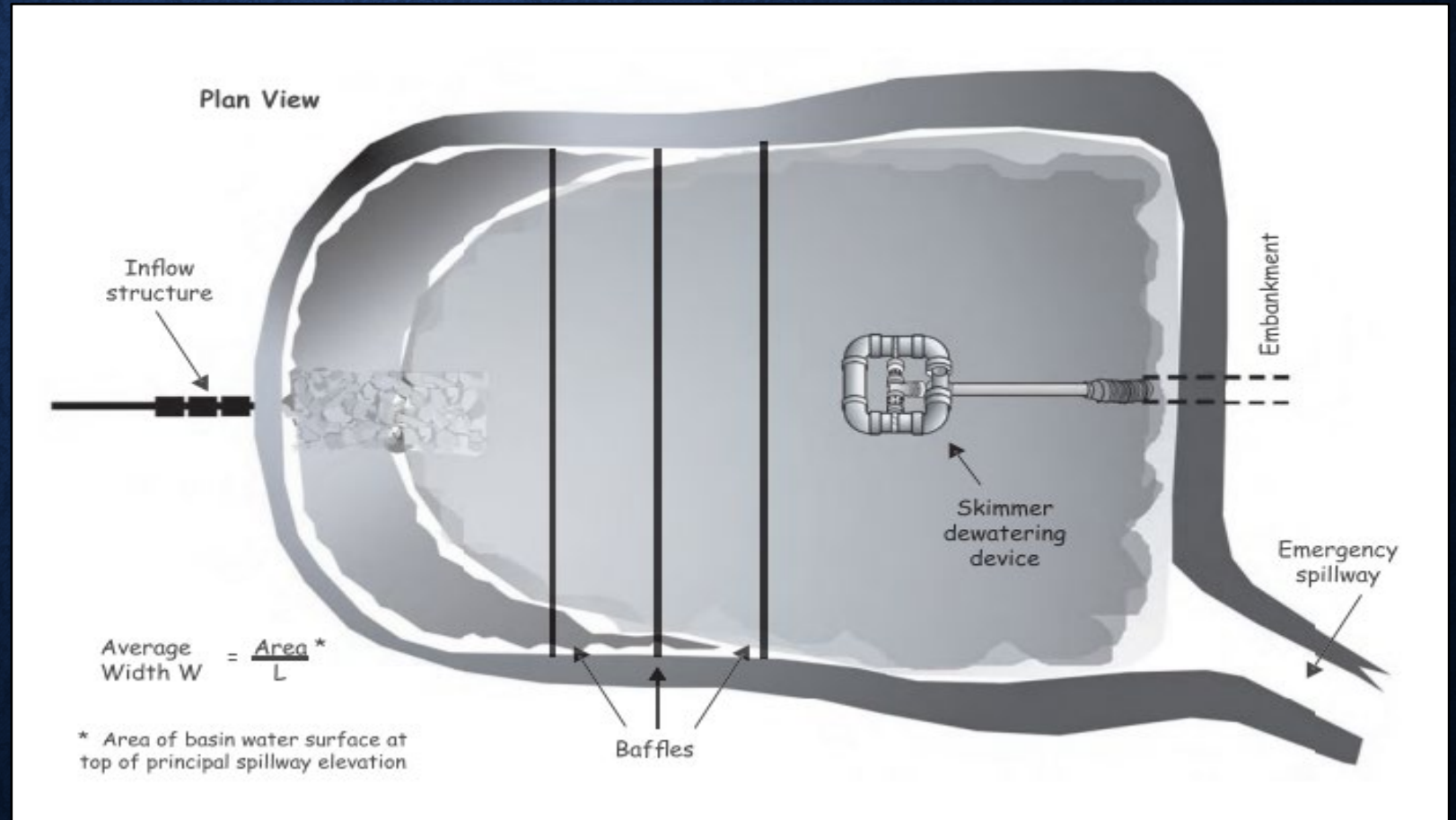


DESIGN STANDARDS

SKIMMER BASINS

Embankment:

- Max Dam Height: 5 feet
- Top width: 5 feet
- Sideslopes 2:1 or flatter

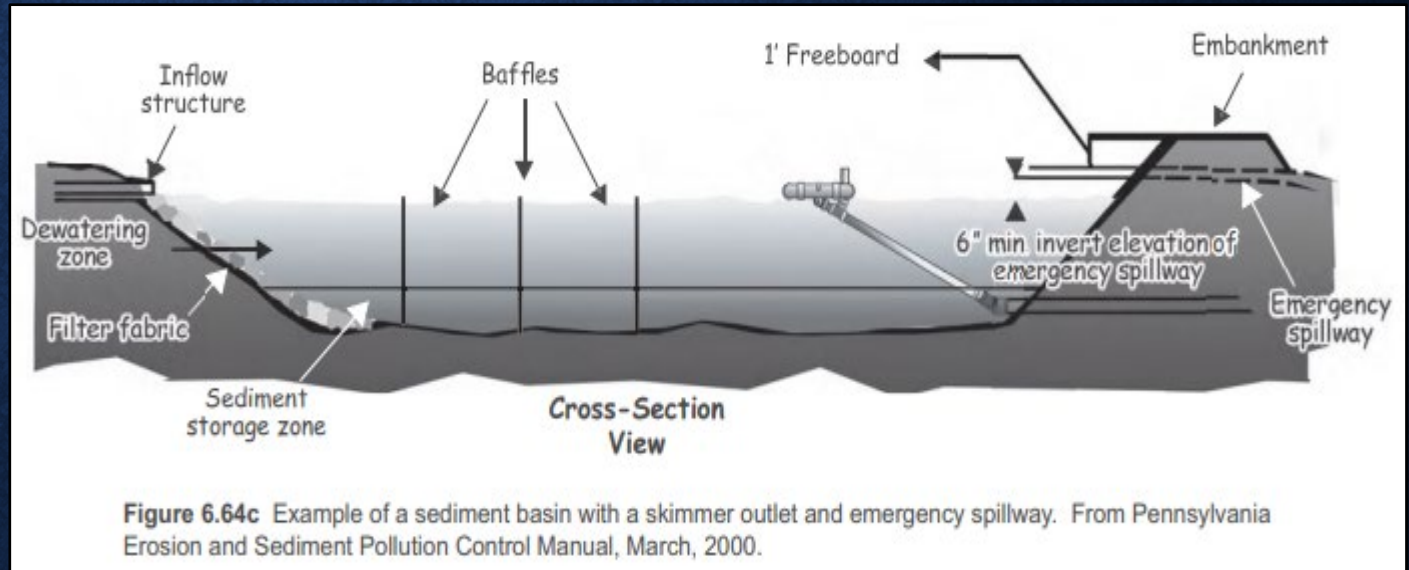


DESIGN STANDARDS

SKIMMER BASINS

Emergency Spillway:

- Same as riser basin
- Max depth of flow 6" from design storm elevation to spillway invert
- Designed length



DESIGN STANDARDS

ALL BASINS

Embankment:

➤ Stabilized:

- Temporary < 1 year
- Permanent ≥ 1 year



DESIGN STANDARDS

ALL BASINS

Skimmer:

- Attached to base of riser or through bottom of dam for skimmer basins
- Sized appropriately
- Dewater in 2-5 days

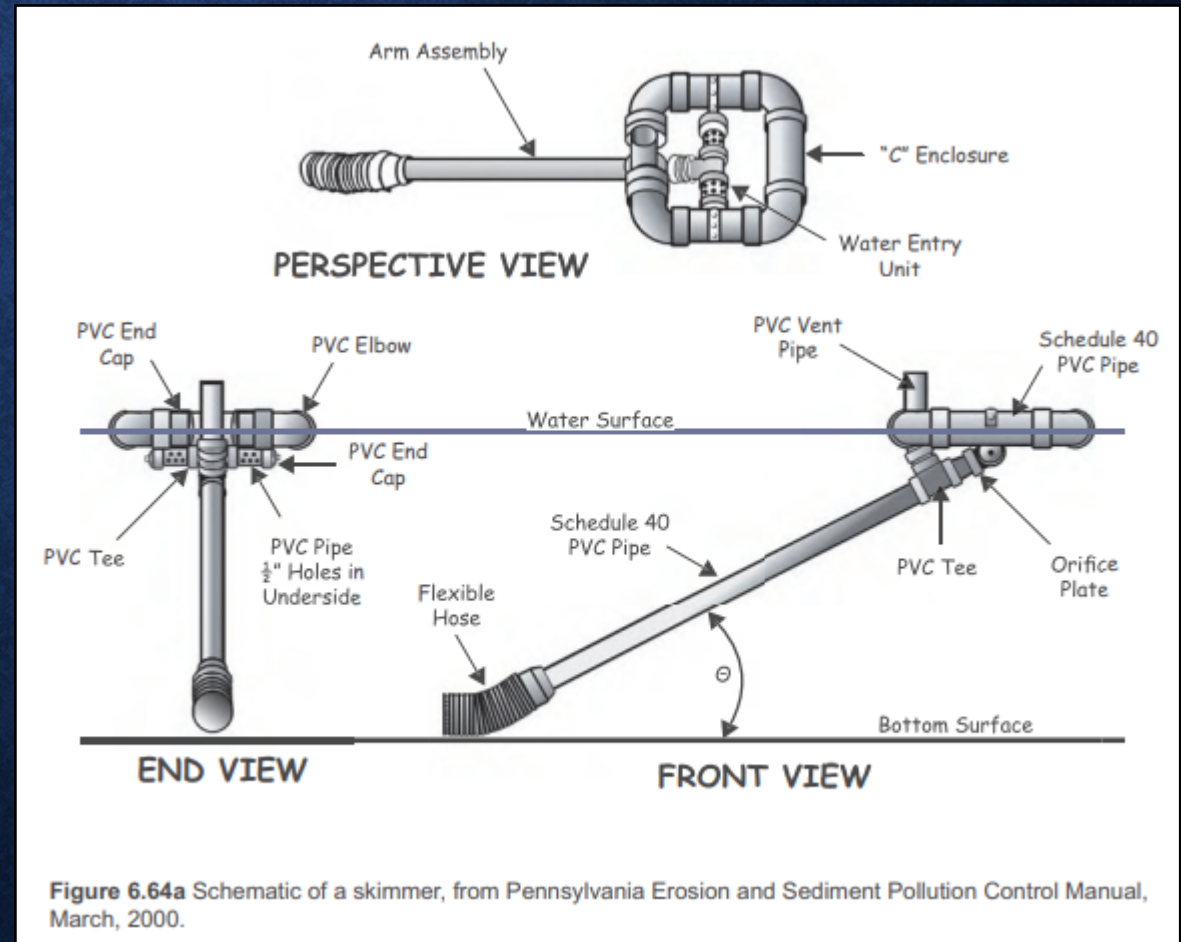


DESIGN STANDARDS

ALL BASINS

Skimmer:

- Schedule 40 PVC barrel
- Sit on rip rap pad to prevent sticking
- Retrieval rope



DESIGN STANDARDS

ALL BASINS

Baffles:

- Minimum (3) porous baffles unless basin < 20 ft
- Separate basin to even cells
- Purpose is to spread out flow and utilize surface area
- Should not be located directly at basin inlet

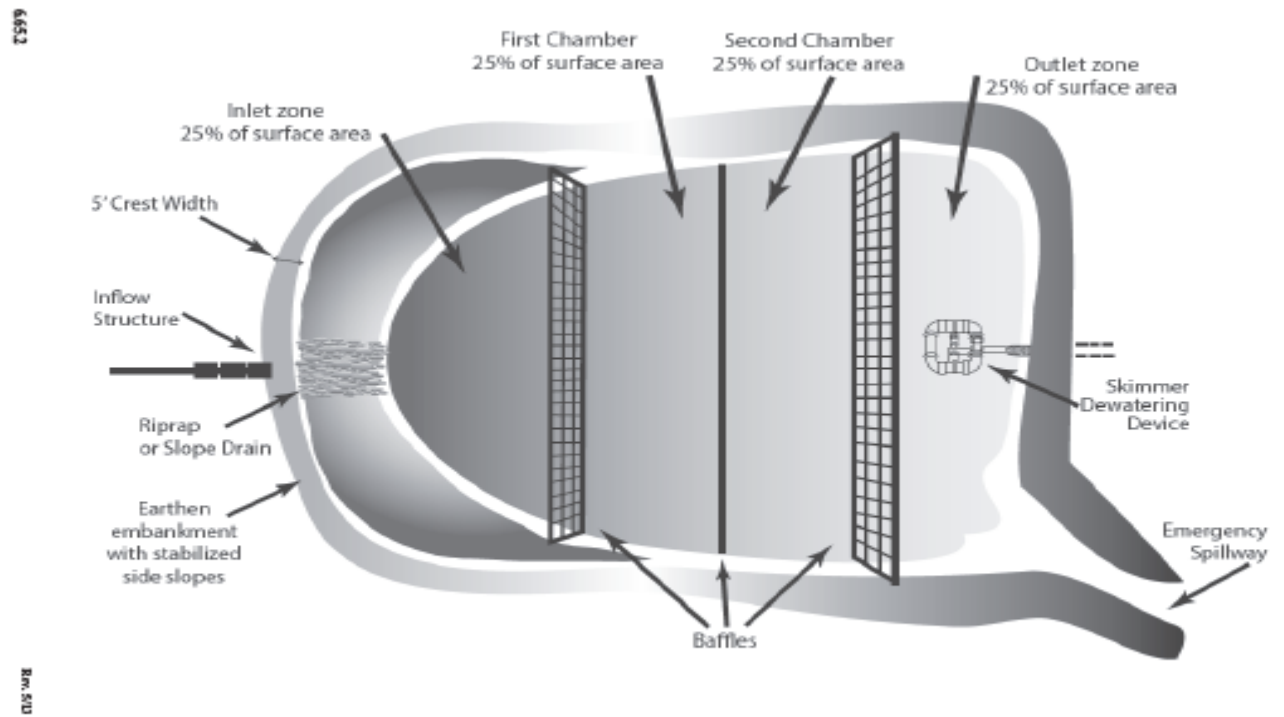


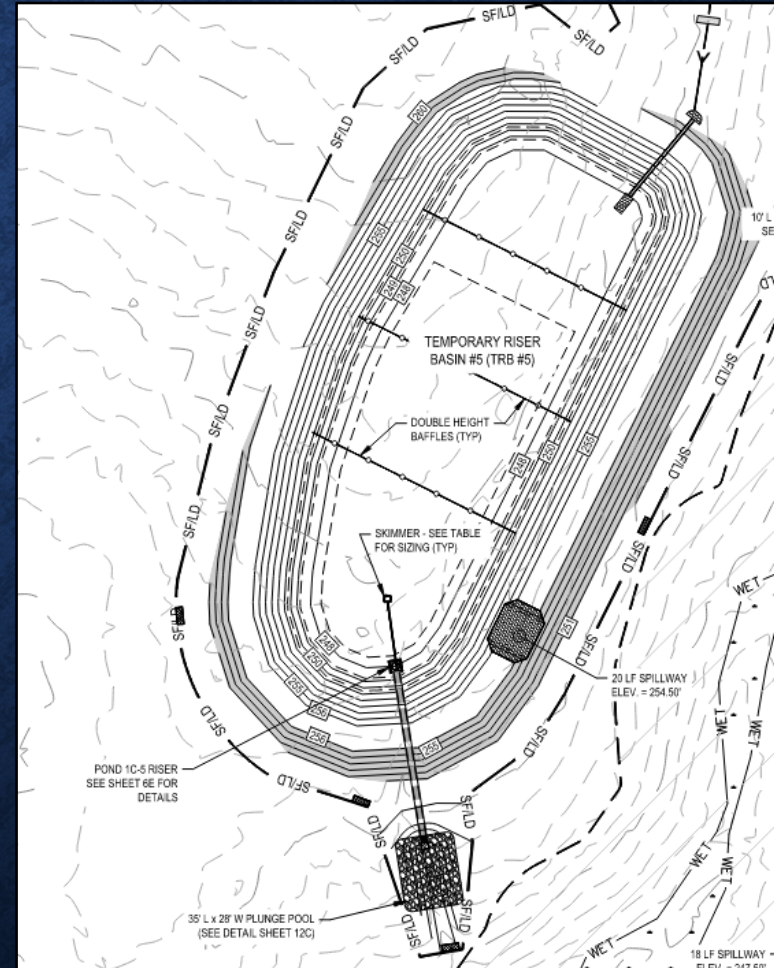
Figure 6.65a Porous baffles in a sediment basin. The flow is distributed evenly across the basin to reduce flow rates and turbulence, resulting in greater sediment retention.

DESIGN STANDARDS

ALL BASINS

Baffles:

- Top of baffle fabric 6" higher than invert of spillway
- Top of baffle 2" lower than top of dam
- If temporary basin will be converted to permanent stormwater basin of greater depth, baffle height based on pool depth during use as temporary basin
- Increase in baffle height may be necessary



DESIGN STANDARDS

ALL BASINS



Single height baffles

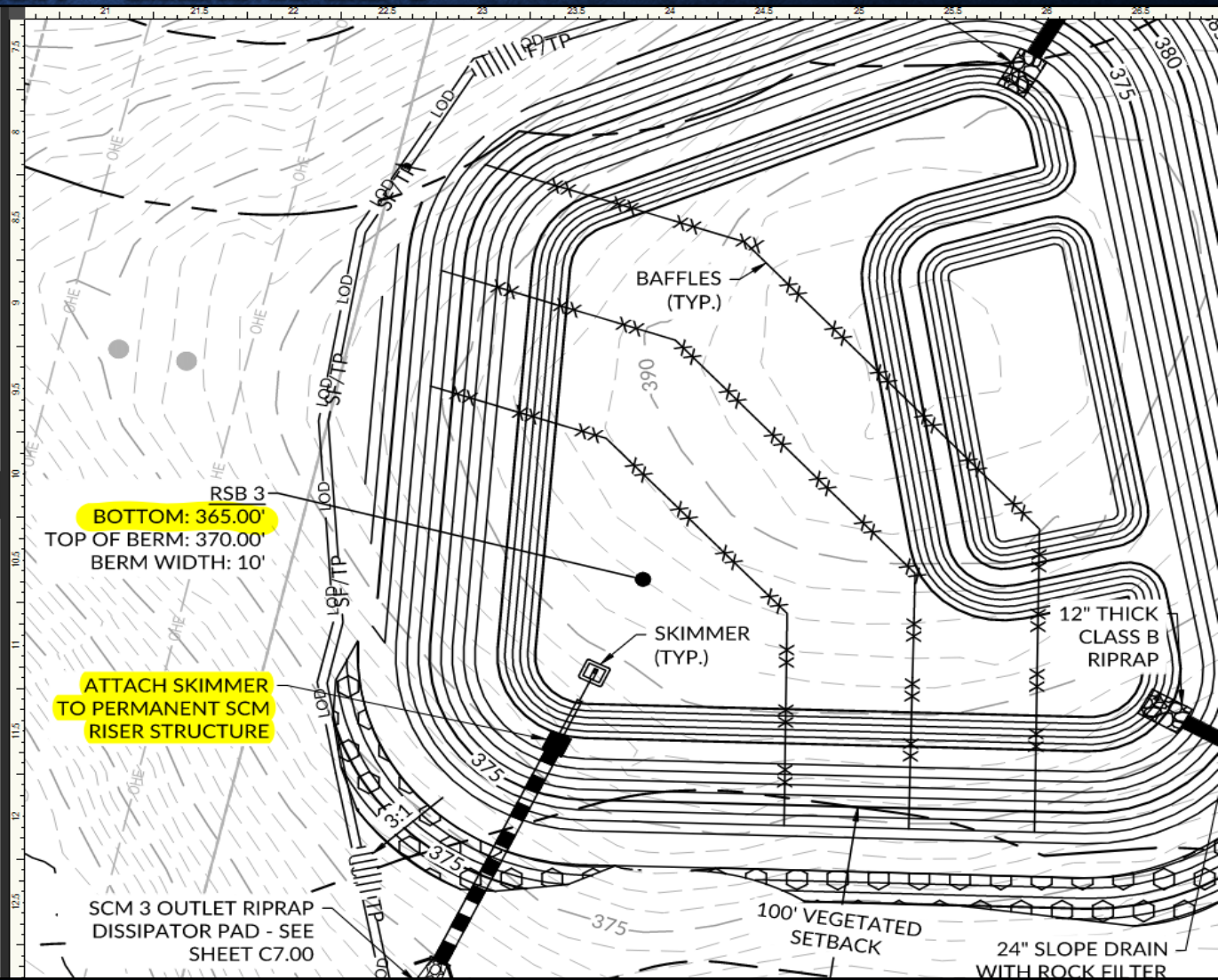
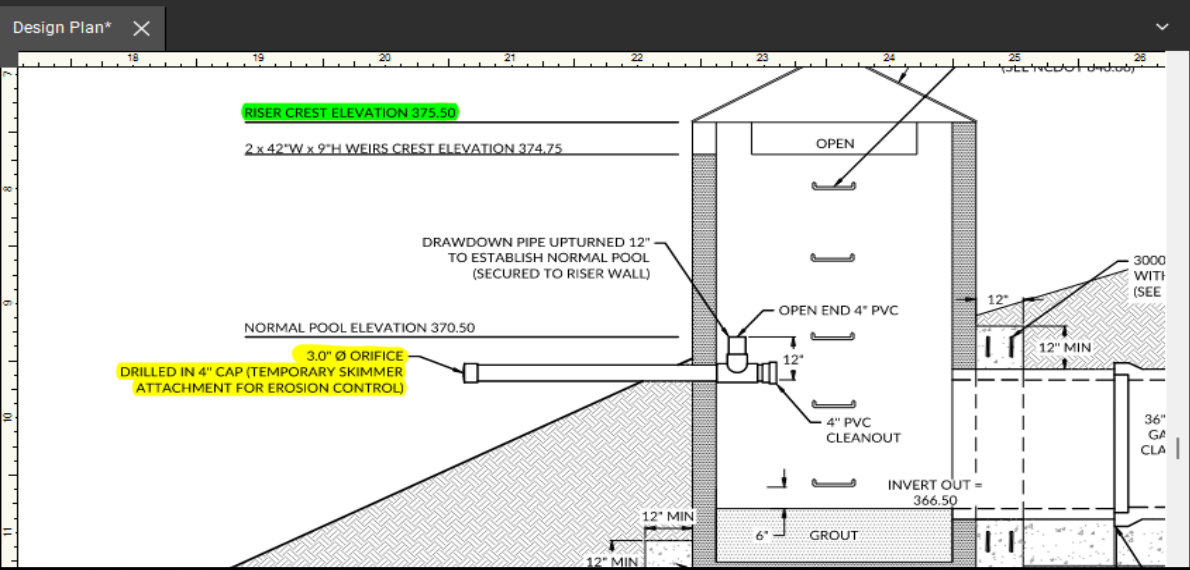
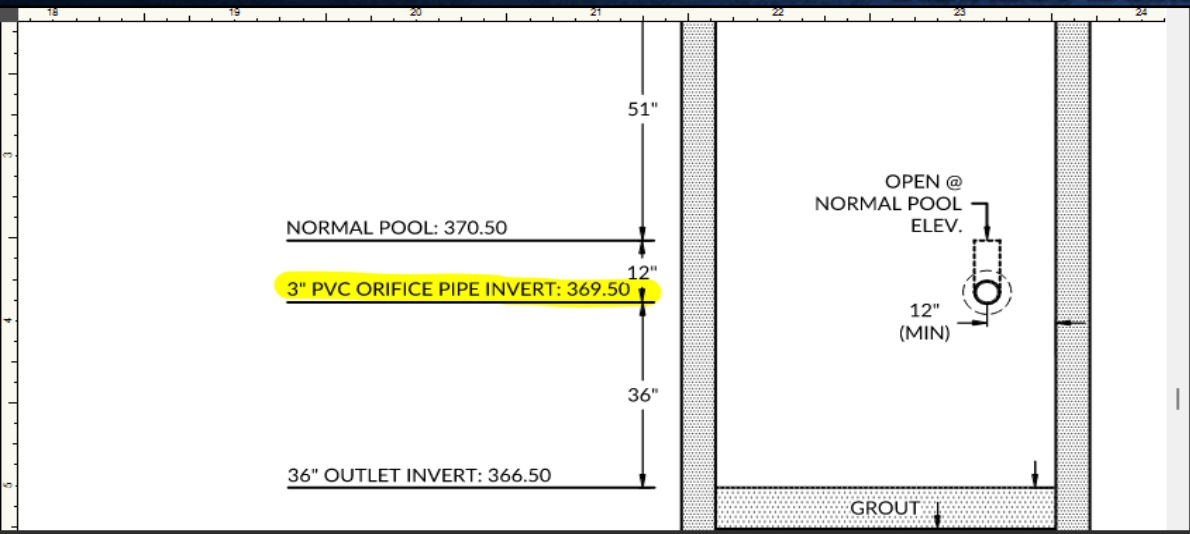


Single height baffles



DESIGN STANDARDS

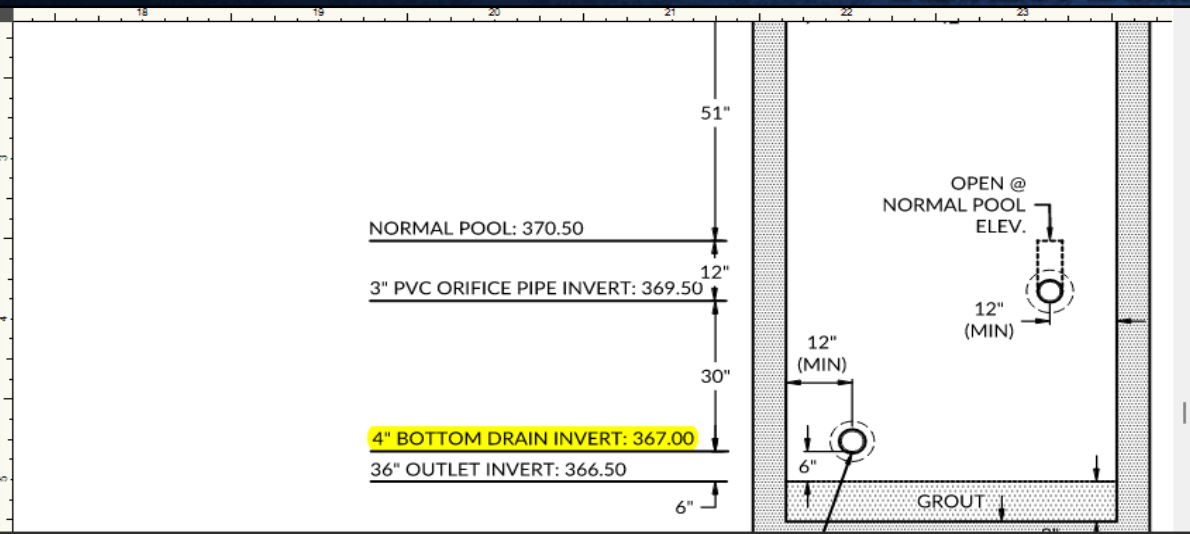
RISER BASIN – EXAMPLE 1



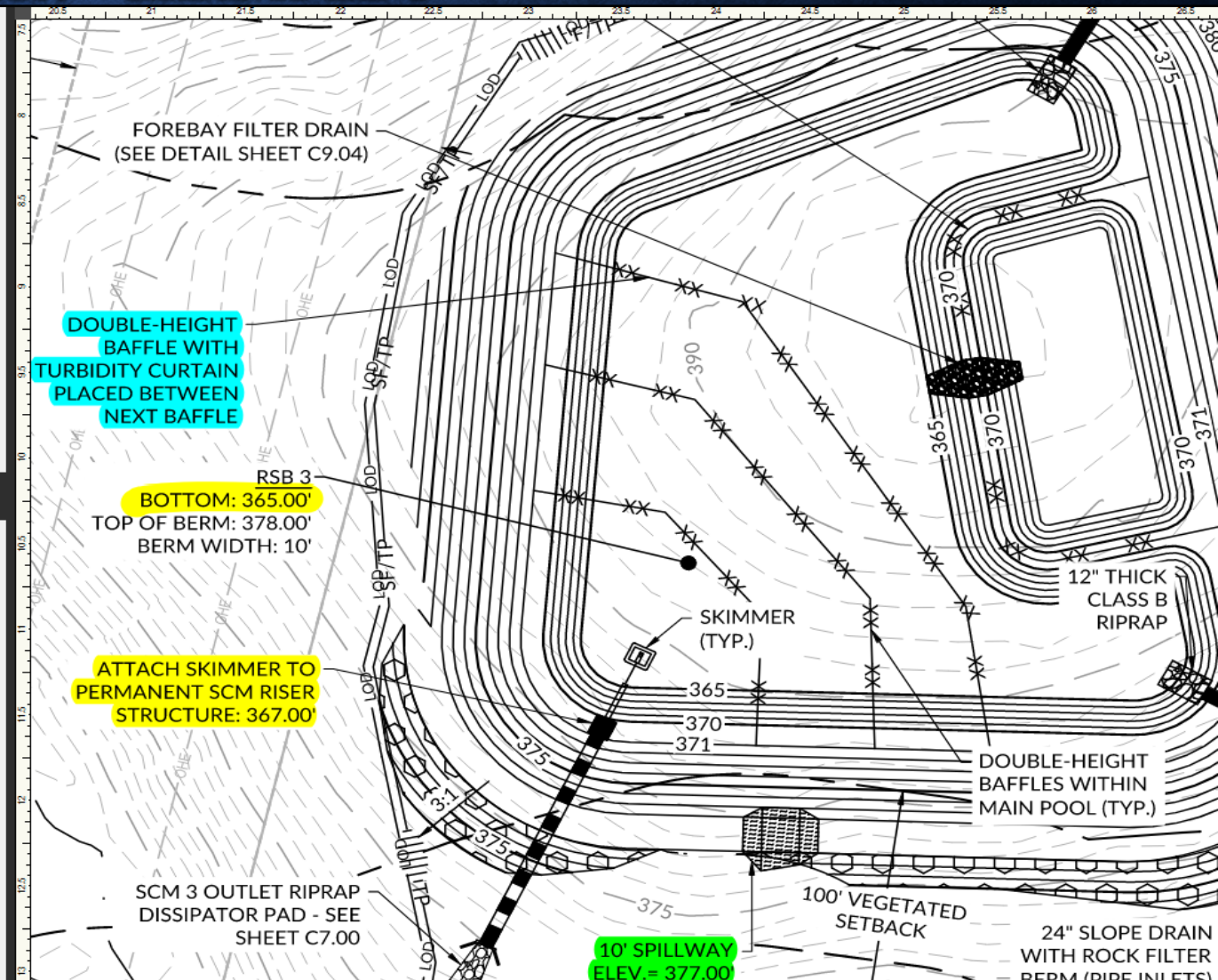
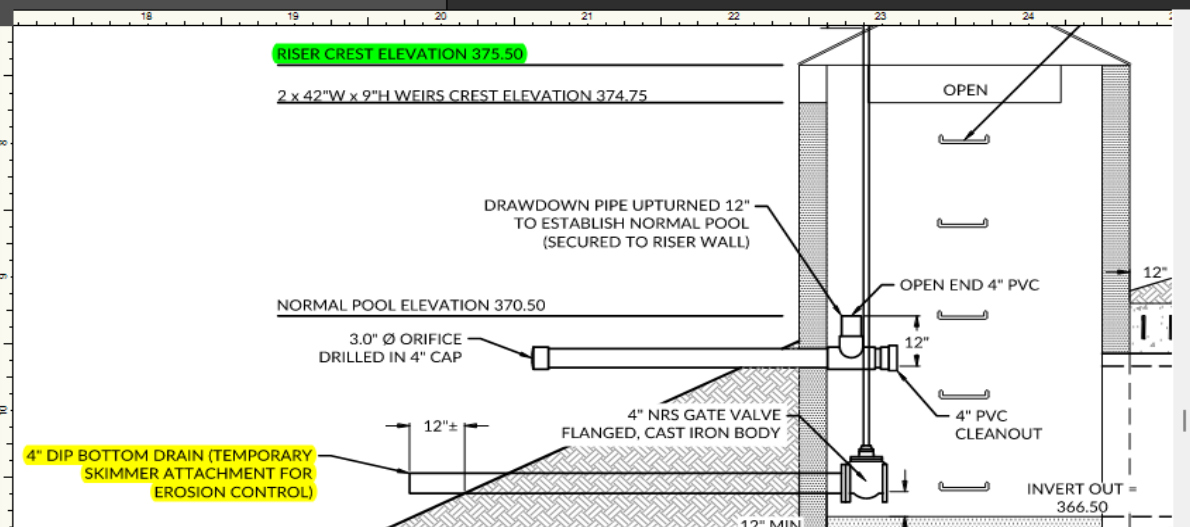


DESIGN STANDARDS

RISER BASIN – EXAMPLE 1

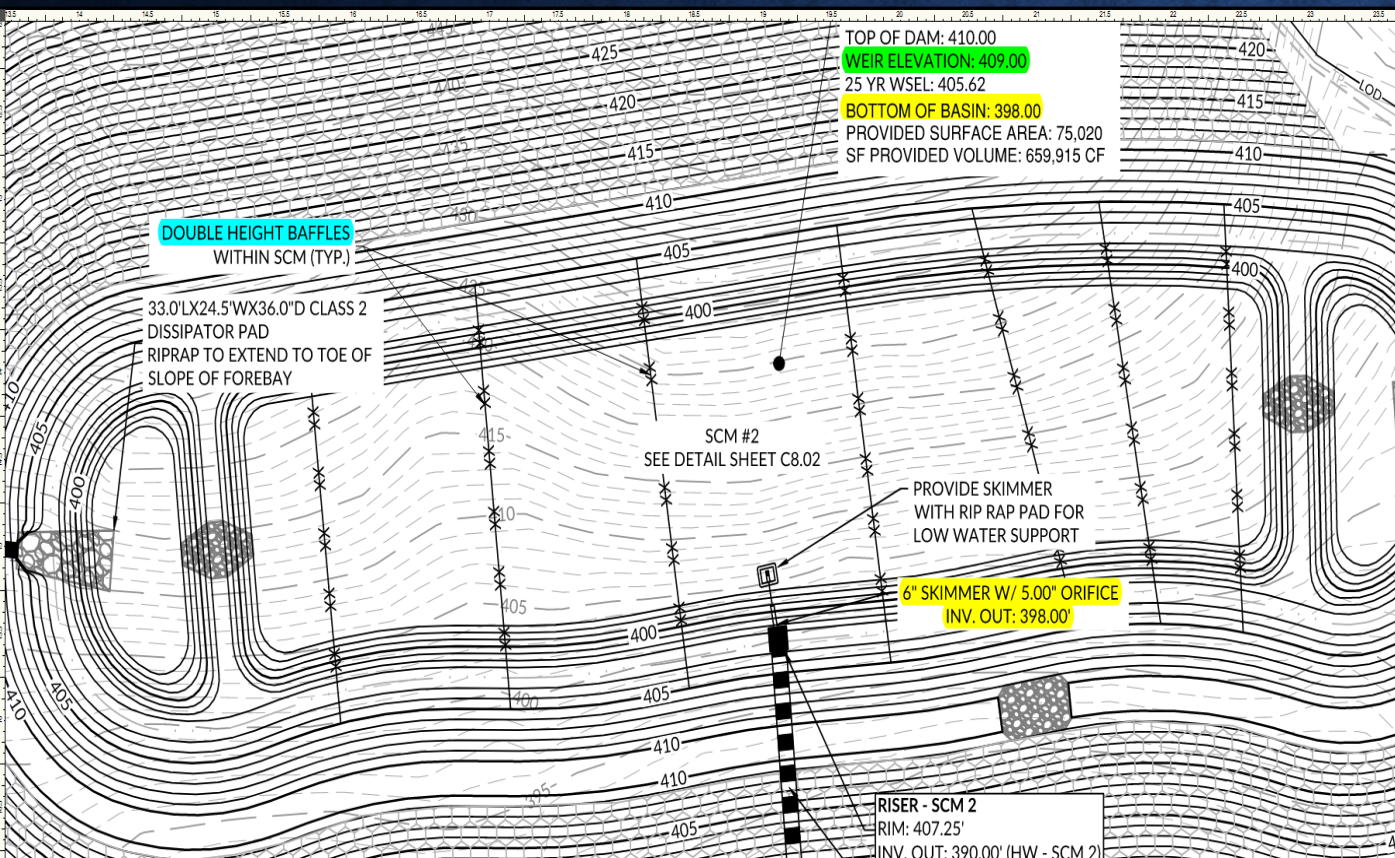


Approved Erosion Control Plan REVISED 10-24-25*



DESIGN STANDARDS

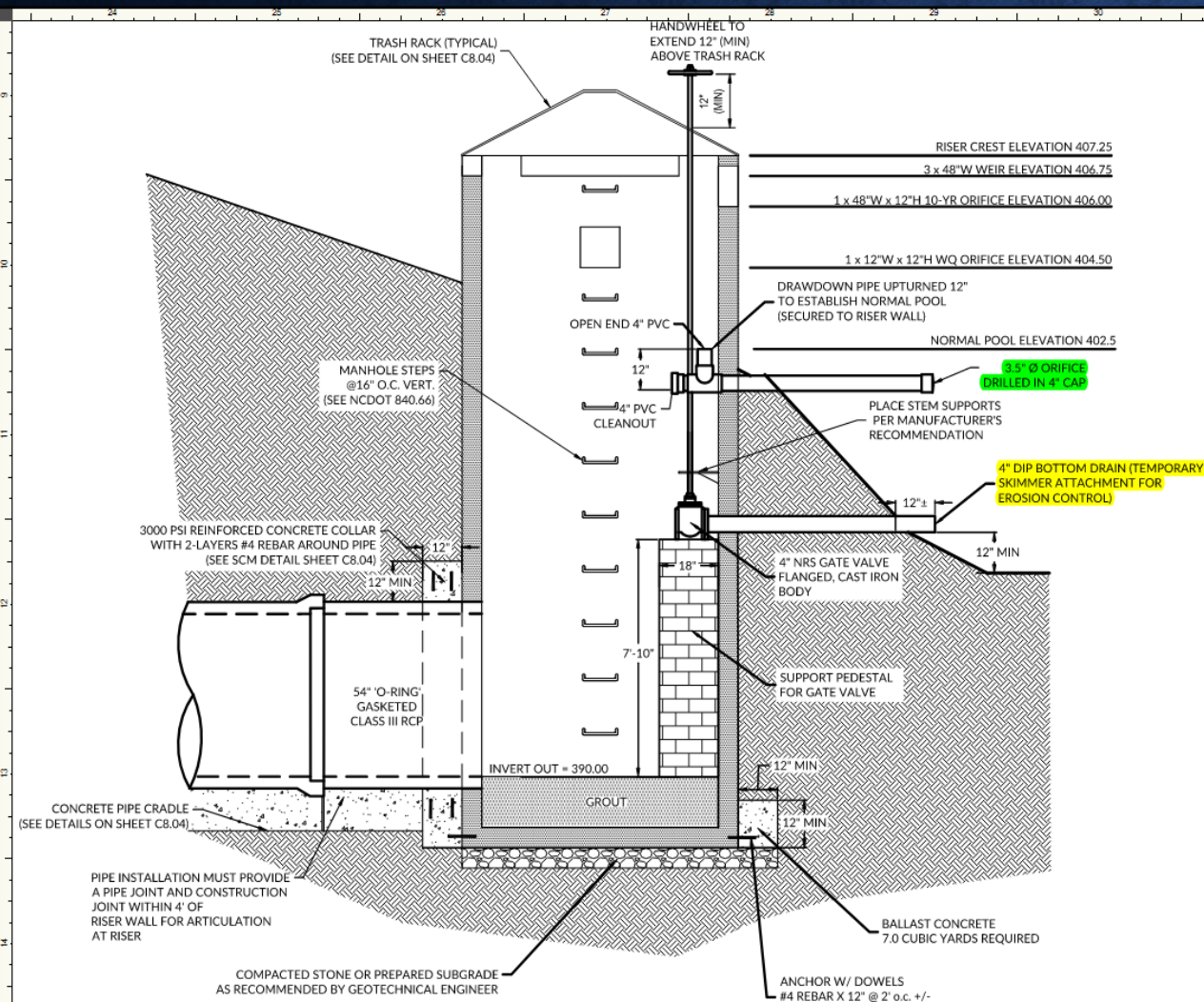
RISER BASIN – EXAMPLE 2



Sediment basin to permanent depth

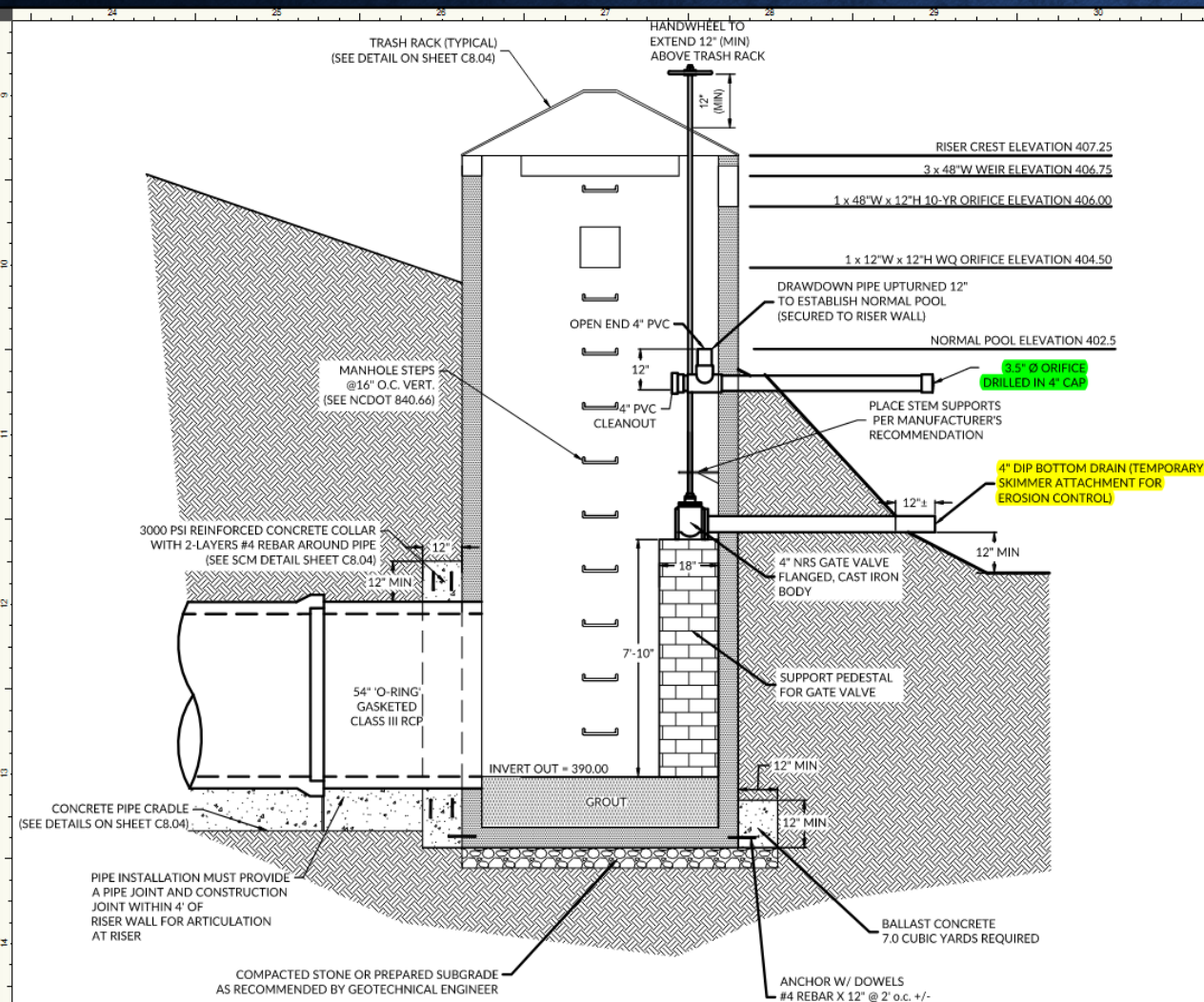
DESIGN STANDARDS

RISER BASIN – EXAMPLE 2



DESIGN STANDARDS

RISER BASIN – EXAMPLE 2





DESIGN STANDARDS

RISER BASIN - EXAMPLE 2



DESIGN STANDARDS

RISER BASIN – EXAMPLE 2





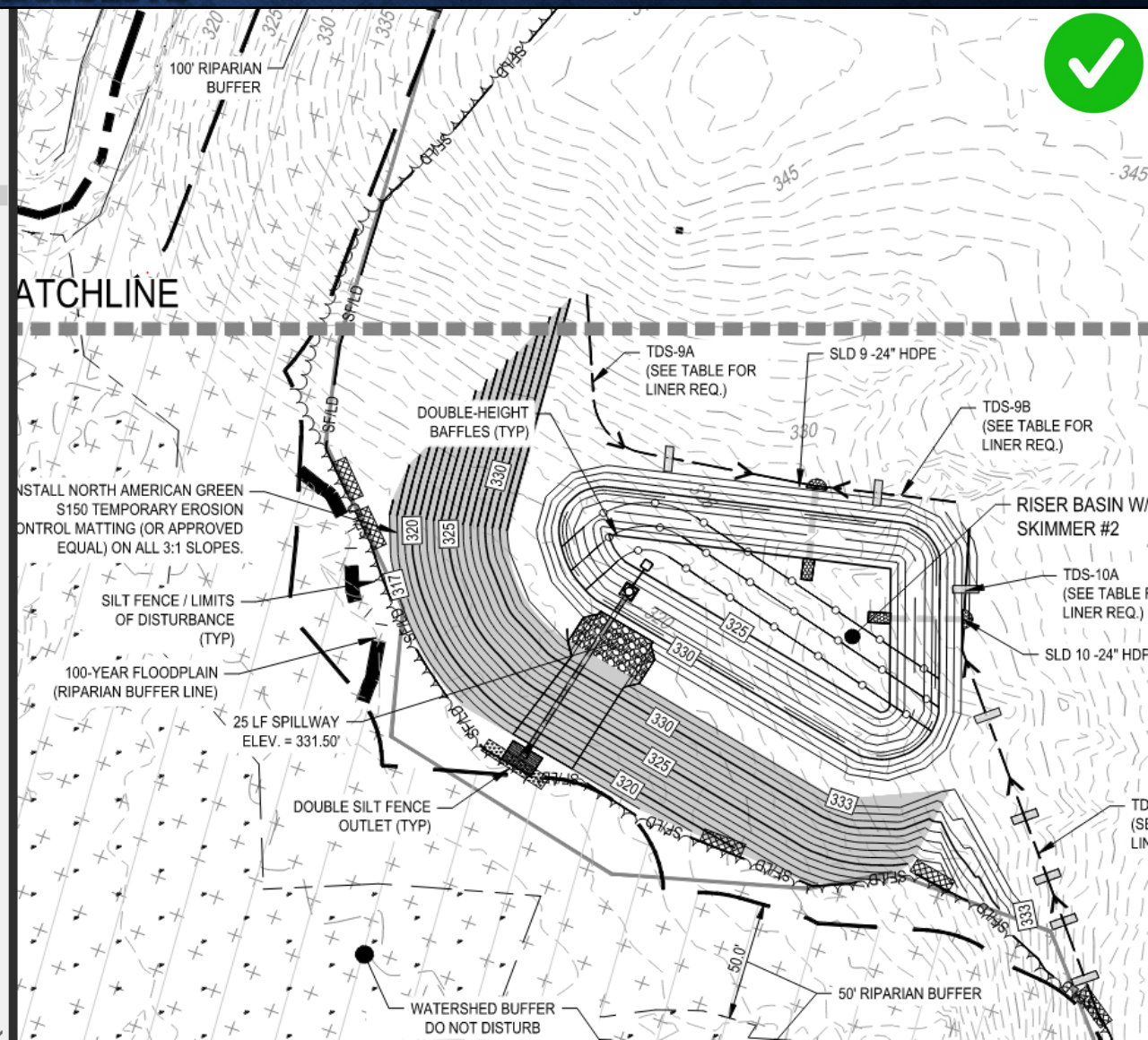
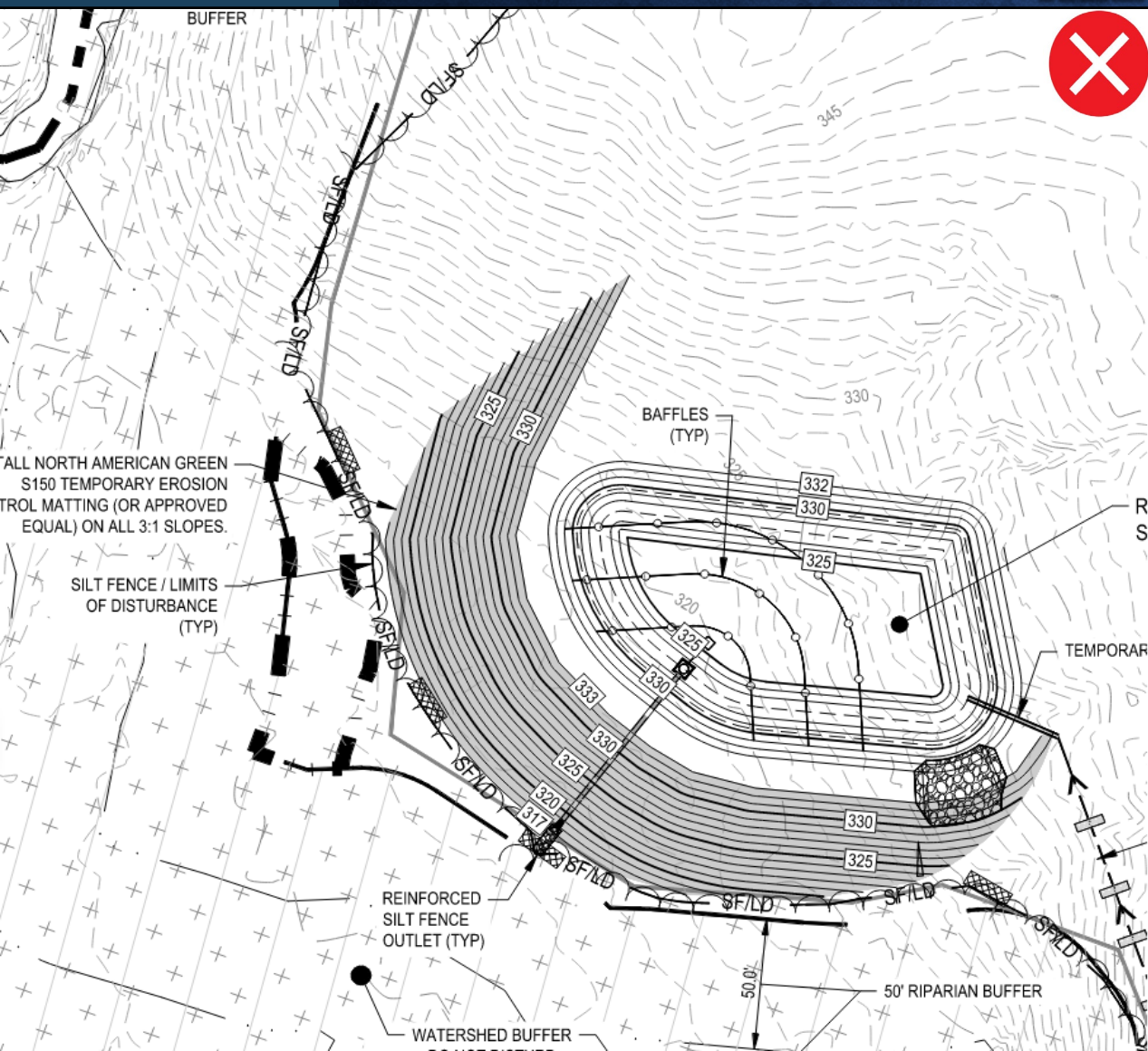
- Basin shape = 2:1 to 6:1
- Longest flow path
- No sheet flow
- Stable inlets
 - Slope drain
 - Rip rap lined

SLOPE DRAIN SIZING										
Slope Drain ID	Diversion Ditch ID	Drainage Area (Ac)	C	I (Q ₂₅ , in/hr)	Q ₂₅ (cfs)	Ditch Depth (ft)	Slope Drain Size (in)	Slope Drain Area (SF)	h Value (ft)	Slope Drain Capacity (cfs)
1	DD-1	0.06	0.60	8.30	0.30	1.0	15	1.23	0.375	3.62
2	DD-2	0.61	0.60	8.30	3.04	1.5	15	1.23	0.875	5.53
3	DD-3	0.24	0.60	8.30	1.20	1.0	15	1.23	0.375	3.62
4	DD-4 & 7	2.43	0.60	8.30	12.10	2.0	24	3.14	1.000	15.13
5	DD-5	1.00	0.60	8.30	4.98	1.5	15	1.23	0.875	5.53
6	DD-6	1.48	0.60	8.30	7.37	1.5	18	1.77	0.750	7.37

** Basin side slope to be armored with 12" NCDOT Class B Rip Rap (d50 = 6") with geotextile fabric base.

DESIGN STANDARDS

ALL BASINS



DESIGN STANDARDS

ALL BASINS

Other:

- Stabilize all areas disturbed by construction



- Outlet protection

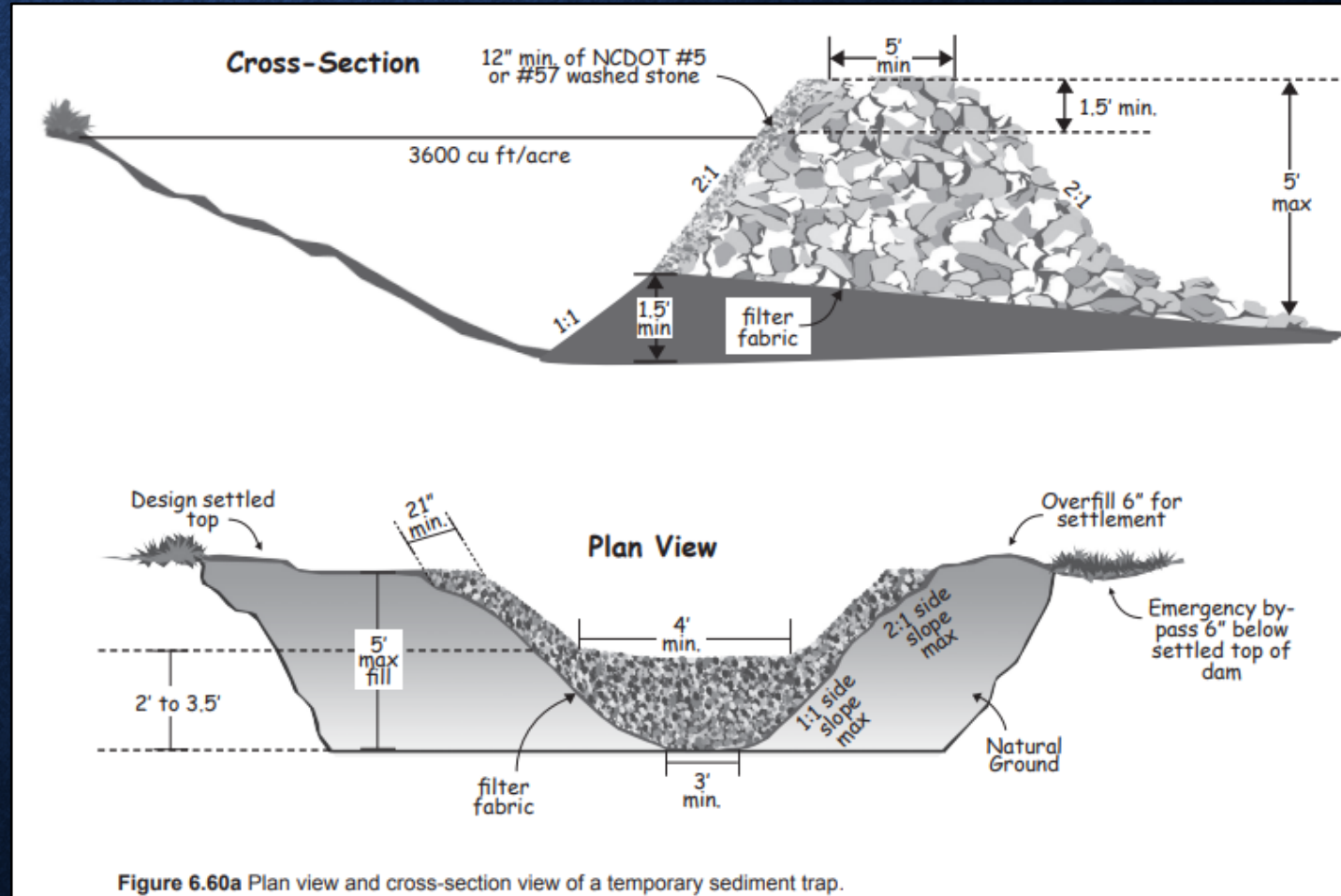
DESIGN STANDARDS

TEMPORARY SEDIMENT TRAPS

Criteria:

- Used for smaller drainage areas
- Miniature basin with no skimmer
- Capacity, stone outlet, and stabilization very important

Design Criteria	Summary:	Temporary Sediment Trap
	Primary Spillway:	Stone Spillway
	Maximum Drainage Area:	5 acres
	Minimum Volume:	3600 cubic feet per acre of disturbed area
	Minimum Surface Area:	435 square feet per cfs of Q_{10} peak inflow
	Minimum L/W Ratio:	2:1
	Minimum Depth:	3.5 feet, 1.5 feet excavated below grade
	Maximum Height:	Weir elevation 3.5 feet above grade
	Dewatering Mechanism:	Stone Spillway
	Minimum Dewatering Time:	N/A
	Baffles Required:	3



DESIGN STANDARDS

DITCH/SWALE

Parabolic



Trapezoidal



Triangular



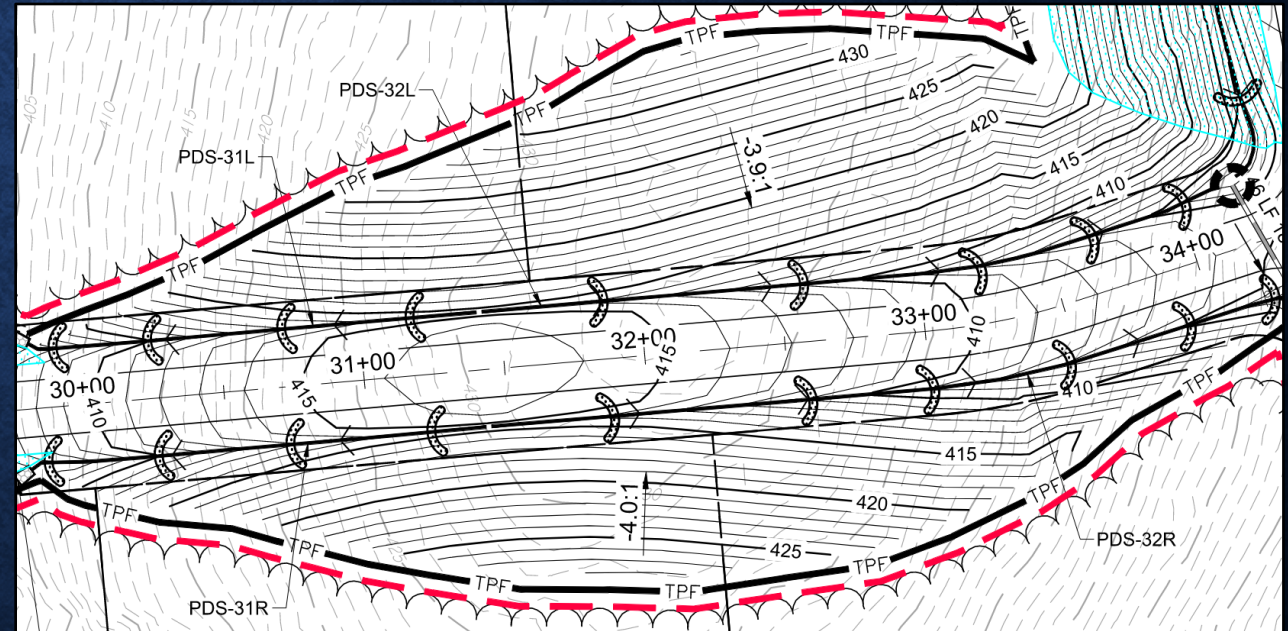
Design and site conditions dictate.

DESIGN STANDARDS

DITCH/SWALE

Capacity

- 10-year storm w/o eroding
- Increased capacity for flood hazards
- Check dams/wattles every 2' in grade change

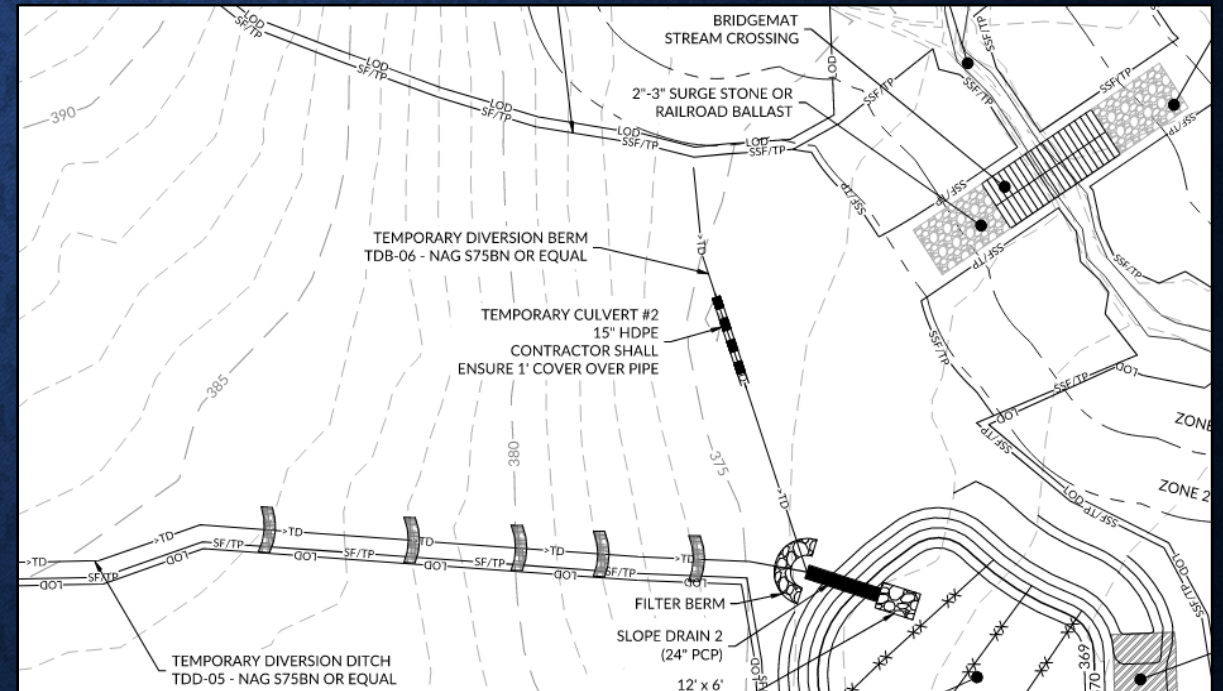


DESIGN STANDARDS

DITCH/SWALE

Grade

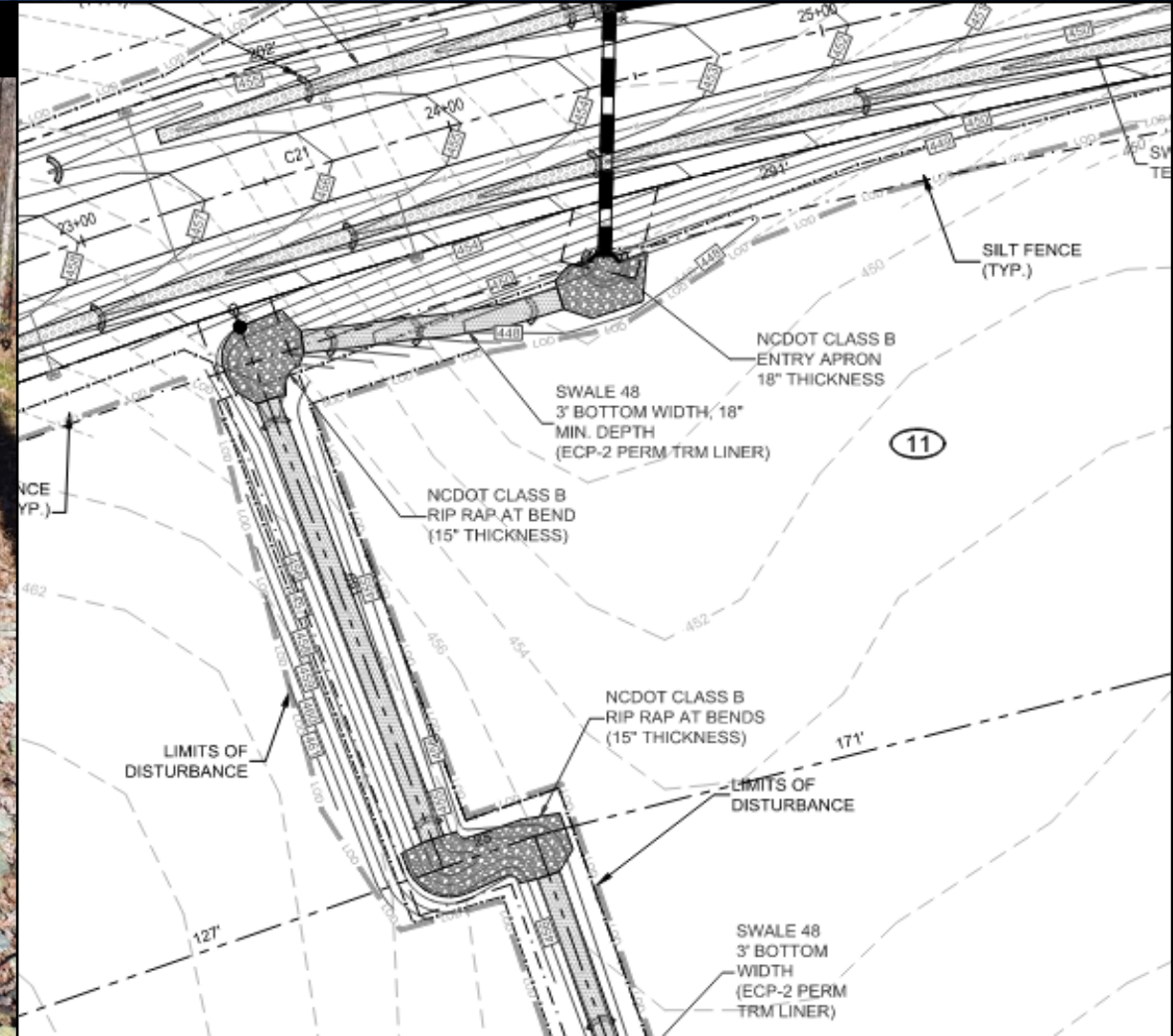
- Positive drainage
- Uniform or gradually increasing
- 3:1 slopes or flatter
- Ditch should be uninterrupted



DESIGN STANDARDS

DITCH/SWALE

Riprap for excessive grade/sharp turns



DESIGN STANDARDS

DITCH/SWALE

Drainage Area

- Drainage areas should match calcs
- Most onsite drainage should be accounted for
- Basin drainage > sum of ditches



DESIGN STANDARDS

DITCH/SWALE

Velocity

- Allowable design determined by soil conditions, type of vegetation, method of establishment
- Maximum permissible velocities table
- Preferred < 5.0 ft/sec up to 6.0 ft/sec
- Velocity > 2.0 ft/sec requires liner
 - Temporary liner: 2-year storm
 - Permanent liner: 10-year storm

8

Table 8.05a
Maximum Allowable Design Velocities¹
for Vegetated Channels

Typical Channel Slope Application	Soil Characteristics ²	Grass Lining	Permissible Velocity ³ for Established Grass Lining (ft/sec)
0-5%	Easily Erodible Non-plastic (Sands & Silts)	Bermudagrass	5.0
		Tall fescue	4.5
		Bahiagrass	4.5
		Kentucky bluegrass	4.5
		Grass-legume mixture	3.5
	Erosion Resistant Plastic (Clay mixes)	Bermudagrass	6.0
		Tall fescue	5.5
		Bahiagrass	5.5
		Kentucky bluegrass	5.5
		Grass-legume mixture	4.5
5-10%	Easily Erodible Non-plastic (Sands & Silts)	Bermudagrass	4.5
		Tall fescue	4.0
		Bahiagrass	4.0
		Kentucky bluegrass	4.0
		Grass-legume mixture	3.0
	Erosion Resistant Plastic (Clay mixes)	Bermudagrass	5.5
		Tall fescue	5.0
		Bahiagrass	5.0
		Kentucky bluegrass	5.0
		Grass-legume mixture	3.5
>10%	Easily Erodible Non-plastic (Sands & Silts)	Bermudagrass	3.5
		Tall fescue	2.5
		Bahiagrass	2.5
		Kentucky bluegrass	2.5
		Grass-legume mixture	2.5
	Erosion Resistant Plastic (Clay mixes)	Bermudagrass	4.5
		Tall fescue	3.5
		Bahiagrass	3.5
		Kentucky bluegrass	3.5
		Grass-legume mixture	3.5

Source: USDA-SCS Modified

NOTE: ¹Permissible Velocity based on 10-year storm peak runoff

²Soil erodibility based on resistance to soil movement from concentrated flowing water.

³Before grass is established, permissible velocity is determined by the type of temporary liner used.

DESIGN STANDARDS

DITCH/SWALE

Shear Stress

- Is the point at which soil will erode
- Determines what liner is required
- Liner: Appendix 8 of NCDEQ Manual
- Calculated shear stress < permissible shear stress of liner

Table 8.05g
Permissible Shear Stresses
for Riprap and Temporary
Liners


		Permissible Unit Shear Stress, T_d
Lining Category	Lining Type	(lb/ft ²)
Temporary	Woven Paper Net	0.15
	Jute Net	0.45
	Fiberglass Roving:	
	Single	0.60
	Double	0.85
	Straw with Net	1.45
	Curled Wood mat	1.55
	Synthetic Mat	2.00
d_{50} Stone Size (inches)		
Gravel Riprap	1	0.33
	2	0.67
Rock Riprap	6	2.00
	9	3.00
	12	4.00
	15	5.00
	18	6.00
	21	7.80
	24	8.00

Adapted From: FHWA, HEC-15, April 1983, pgs. 17 & 37.

DESIGN STANDARDS

DITCH/SWALE

Material Content		
Matrix	100% Coconut Fiber	0.5 lbs/sq yd (0.27 kg/sm)
	Leno Woven 100% biodegradable jute	9.3 lbs/1000 sq ft (4.5 kg/100 sm)
Netting	100% Biodegradable jute	7.7 lb/1000 sq ft (3.76 kg/100 sm)
	Thread	Biodegradable



Specification Sheet

BioNet® C125BN™ Erosion Control Blanket

DESCRIPTION

The long-term double net erosion control blanket shall be a machine-produced mat of 100% coconut fiber with a functional longevity of up to 36 months. (NOTE: functional longevity may vary depending upon climatic conditions, soil, geographical location, and elevation). The blanket shall be of consistent thickness with the coconut evenly distributed over the entire area of the mat. The blanket shall be covered on the top and bottom sides with 100% biodegradable woven natural organic fiber netting. The netting shall consist of machine directional strands formed from two intertwined yarns with cross directional strands interwoven through the the twisted machine strands (commonly referred to as Leno weave) to form an approximate 0.50 x 1.0 in (1.27 x 2.54 cm) mesh. The blanket shall be manufactured with a colored thread stitched along both outer edges (approximately 2-5 inches [5-12.5 cm] from the edge) as an overlap guide for adjacent mats.

The C125BN shall meet Type 4 specification requirements established by the Erosion Control Technology Council (ECTC) and Federal Highway Administration's (FHWA) FP-03 Section 713.17.

Material Content		
Matrix	100% Coconut Fiber	0.5 lbs/sq yd (0.27 kg/sm)
	Leno Woven 100% biodegradable jute	9.3 lbs/1000 sq ft (4.5 kg/100 sm)
Netting	100% Biodegradable jute	7.7 lb/1000 sq ft (3.76 kg/100 sm)
	Thread	Biodegradable



Index Property	Test Method	Typical
Thickness	ASTM D6525	0.23 in. (5.84 mm)
Resiliency	ECTC Guidelines	80%
Water Absorbency	ASTM D1117	260%
Mass/Unit Area	ASTM G475	9.79 oz/sq yd (323 g/sm)
Swell	ECTC Guidelines	40%
Snapper Resistance	ECTC Guidelines	Yes
Stiffness	ASTM D1388	0.11 sq-in
Light Penetration	ASTM D6567	16.2%
Tensile Strength - MD	ASTM D6818	206.4 lbs/ft (3.06 kN/m)
Elongation - MD	ASTM D6818	15.3%
Tensile Strength - TD	ASTM D6818	145.2 lbs/ft (2.15 kN/m)
Elongation - TD	ASTM D6818	12.9%
Biomass Improvement	ASTM 7322	473%

Design Permissible Shear Stress		
Unvegetated Shear Stress	2.35 psf (112 Pa)	
Unvegetated Velocity	10.0 fps (3.05 m/s)	

Slope Design Data: C Factors			
Slope Gradients (S)			
Slope Length (L)	≤ 3:1	3:1 - 2:1	≥ 2:1
≤ 20 ft (6 m)	0.0001	0.018	0.050
20-50 ft	0.003	0.040	0.060
≥ 50 ft (15.2 m)	0.007	0.070	0.070

Roughness Coefficients - Unveg.		
Flow Depth	Manning's n	
≤ 0.50 ft (0.15 m)	0.022	
0.50 - 2.0 ft	0.022-0.014	
≥ 2.0 ft (0.60 m)	0.014	

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Design Permissible Shear Stress

Unvegetated Shear Stress	2.35 psf (112 Pa)	
Unvegetated Velocity	10.0 fps (3.05 m/s)	

Slope Design Data: C Factors

Slope Gradients (S)			
Slope Length (L)	≤ 3:1	3:1 - 2:1	≥ 2:1
≤ 20 ft (6 m)	0.0001	0.018	0.050
20-50 ft	0.003	0.040	0.060
≥ 50 ft (15.2 m)	0.007	0.070	0.070

Roughness Coefficients - Unveg.

Flow Depth	Manning's n	
≤ 0.50 ft (0.15 m)	0.022	
0.50 - 2.0 ft	0.022-0.014	
≥ 2.0 ft (0.60 m)	0.014	

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Design Permissible Shear Stress

Unvegetated Shear Stress	2.35 psf (112 Pa)	
Unvegetated Velocity	10.0 fps (3.05 m/s)	

Slope Design Data: C Factors

Slope Gradients (S)			
Slope Length (L)	≤ 3:1	3:1 - 2:1	≥ 2:1
≤ 20 ft (6 m)	0.0001	0.018	0.050
20-50 ft	0.003	0.040	0.060
≥ 50 ft (15.2 m)	0.007	0.070	0.070

Roughness Coefficients - Unveg.

Flow Depth	Manning's n	
≤ 0.50 ft (0.15 m)	0.022	
0.50 - 2.0 ft	0.022-0.014	
≥ 2.0 ft (0.60 m)	0.014	

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Design Permissible Shear Stress

Unvegetated Shear Stress	2.35 psf (112 Pa)	
Unvegetated Velocity	10.0 fps (3.05 m/s)	

Slope Design Data: C Factors

Slope Gradients (S)			
Slope Length (L)	≤ 3:1	3:1 - 2:1	≥ 2:1
≤ 20 ft (6 m)	0.0001	0.018	0.050
20-50 ft	0.003	0.040	0.060
≥ 50 ft (15.2 m)	0.007	0.070	0.070

Roughness Coefficients - Unveg.

Flow Depth	Manning's n	
≤ 0.50 ft (0.15 m)	0.022	
0.50 - 2.0 ft	0.022-0.014	
≥ 2.0 ft (0.60 m)	0.014	

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Design Permissible Shear Stress

Unvegetated Shear Stress	2.35 psf (112 Pa)	
Unvegetated Velocity	10.0 fps (3.05 m/s)	

Slope Design Data: C Factors

Slope Gradients (S)			
Slope Length (L)	≤ 3:1	3:1 - 2:1	≥ 2:1
≤ 20 ft (6 m)	0.0001	0.018	0.050
20-50 ft	0.003	0.040	0.060
≥ 50 ft (15.2 m)	0.007	0.070	0.070

Roughness Coefficients - Unveg.

Flow Depth	Manning's n	
≤ 0.50 ft (0.15 m)	0.022	
0.50 - 2.0 ft	0.022-0.014	
≥ 2.0 ft (0.60 m)	0.014	

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Design Permissible Shear Stress

Unvegetated Shear Stress	2.35 psf (112 Pa)	
Unvegetated Velocity	10.0 fps (3.05 m/s)	

Slope Design Data: C Factors

Slope Gradients (S)			
Slope Length (L)	≤ 3:1	3:1 - 2:1	≥ 2:1
≤ 20 ft (6 m)	0.0001	0.018	0.050
20-50 ft	0.003	0.040	0.060
≥ 50 ft (15.2 m)	0.007	0.070	0.070

Roughness Coefficients - Unveg.

Flow Depth	Manning's n	
≤ 0.50 ft (0.15 m)	0.022	
0.50 - 2.0 ft	0.022-0.014	
≥ 2.0 ft (0.60 m)	0.014	

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Design Permissible Shear Stress

Unvegetated Shear Stress	2.35 psf (112 Pa)	
Unvegetated Velocity	10.0 fps (3.05 m/s)	

Slope Design Data: C Factors

Slope Gradients (S)			
Slope Length (L)	≤ 3:1	3:1 - 2:1	≥ 2:1
≤ 20 ft (6 m)	0.0001	0.018	0.050
20-50 ft	0.003	0.040	0.060
≥ 50 ft (15.2 m)	0.007	0.070	0.070

Roughness Coefficients - Unveg.

Flow Depth	Manning's n	
≤ 0.50 ft (0.15 m)	0.022	
0.50 - 2.0 ft	0.022-0.014	
≥ 2.0 ft (0.60 m)	0.014	

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Design Permissible Shear Stress

Unvegetated Shear Stress	2.35 psf (112 Pa)	
Unvegetated Velocity	10.0 fps (3.05 m/s)	

Slope Design Data: C Factors

Slope Gradients (S)			
Slope Length (L)	≤ 3:1	3:1 - 2:1	≥ 2:1
≤ 20 ft (6 m)	0.0001	0.018	0.050
20-50 ft	0.003	0.040	0.060
≥ 50 ft (15.2 m)	0.007	0.070	0.070

Roughness Coefficients - Unveg.

Flow Depth	Manning's n	
≤ 0.50 ft (0.15 m)	0.022	
0.50 - 2.0 ft	0.022-0.014	
≥ 2.0 ft (0.60 m)	0.014	

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Design Permissible Shear Stress

Unvegetated Shear Stress	2.35 psf (112 Pa)	
Unvegetated Velocity	10.0 fps (3.05 m/s)	

Slope Design Data: C Factors

Slope Gradients (S)			
Slope Length (L)	≤ 3:1	3:1 - 2:1	≥ 2:1
≤ 20 ft (6 m)	0.0001	0.018	0.050
20-50 ft	0.003	0.040	0.060
≥ 50 ft (15.2 m)	0.007	0.070	0.070

Roughness Coefficients - Unveg.

Flow Depth	Manning's n	
≤ 0.50 ft (0.15 m)	0.022	
0.50 - 2.0 ft	0.022-0.014	
≥ 2.0 ft (0.60 m)	0.014	

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Standard Roll Sizes

Width	6.67 (2.03 m)	8.0 ft (2.4 m)
Length	108 ft (32.92 m)	110 ft (34.14 m)
Weight ± 10%	52.22 lbs (23.69 kg)	66.25 lbs (29.61 kg)
Area	80 sq yd (64.9 sm)	100 sq yd (80.61 sm)
	Leno weave top only	Leno weave top and bottom

Design Permissible Shear Stress

Unvegetated Shear Stress	2.35 psf (112 Pa)	
Unvegetated Velocity	10.0 fps (3.05 m/s)	

Slope Design Data: C Factors

Slope Gradients (S)			
Slope Length (L)	≤ 3:1	3:1 - 2:1	≥ 2:1
≤ 20 ft (6 m)	0.0001	0.018	0.050
20-50 ft	0.003	0.040	0.060
≥ 50 ft (15.2 m)	0.007	0.070	0.070

Roughness Coefficients - Unveg.

Flow Depth	Manning's n	
≤ 0.50 ft (0.15 m)	0.022	
0.50 - 2.0 ft	0.022-0.014	



DESIGN STANDARDS

DITCH/SWALE

Physical Characteristics of Drainage Swale							
Chan Bot Width (ft)	Side Slope	Side Slope Length (ft)	Design Depth (ft)	Channel Area (sf)	Wetted Perim., Pw (ft)	Hydraulic Radius (ft)	Channel Slope (ft/ft)
2	2	4.5	2	12.0	10.94	1.10	0.04

Rational Equation for 2 year storm				Addition flow from upstream Swales			Total Flow (cfs)
C Factor	Intensity Inch/hr	Drainage Area (Acres)	Q2 Discharge (Cfs)	Upstream swale#	Upstream swale#	Upstream swale#	
0.50	5.88	1.13	3.32	-	-	-	3.32
				Flow (cfs)	Flow (cfs)	Flow (cfs)	
				0.00	0.00	0.00	

Rational Equation for 10 year storm				Addition flow from upstream Swales			Total Flow (cfs)
C Factor	Intensity Inch/hr	Drainage Area (Acres)	Q10 Discharge (Cfs)	Upstream swale#	Upstream swale#	Upstream swale#	
0.50	7.48	1.13	4.23	-	-	-	4.23
				Flow (cfs)	Flow (cfs)	Flow (cfs)	
				0.00	0.00	0.00	

Bare Soil				
Material	Recommended n Values for Flow Depth			Permissible velocity (fps)
	0-0.5 ft	0.5-2.0 ft	> 2.0 ft	
Bare Soil	0.023	0.02	0.02	2.00

Temporary Liners				
*Material	Recommended n Values for Flow Depth			Allowable Shear Stress (lb/sf)
	0-0.5 ft	0.5-2.0 ft	> 2.0 ft	
Unvegetated				
S75	0.055	0.028	0.021	1.55
S150	0.055	0.028	0.021	1.65
SC150	0.050	0.025	0.018	1.80
C125	0.022	0.014	0.014	2.25
C350	0.040	0.025	0.02	2.25
P300	0.034	0.024	0.02	2.00

Rip Rap Liners				
Material	Recommended n Values for Flow Depth			Allowable Shear Stress (lb/sf)
	0-0.5 ft	0.5-2.0 ft	> 2.0 ft	
Class A	0.044	0.033	0.03	1.65
Class B	0.066	0.041	0.034	3.00
Class I	0.104	0.069	0.035	4.50
Class II	-	0.078	0.04	6.00

Notes: Spreadsheet works for trapezoidal channel only
Side slope = horiz./vert.
Needs input
Output taken from AutoCad's Trapezoidal Ditch Calculator
* The Material lists refers to North American Green products. Contractor to use referenced material or approved equal.

Shear Stress, $T = \gamma(d/12)^*s$
 T = shear stress in lb/sq. ft.
 γ = unit weight of water, 62.4 lb/cu. ft.
 d = flow depth in in.
 s = channel slope in ft./ft.

Bare Soil calculations (Q2)				
Mannings "n"	Q Allow.	AutoCad Depth (in)	AutoCad Velocity (fps)	Shear Stress (lb/sf)
0.020	190.12	3.1	5.17	0.65

Temporary liner calculations (Q2)				
Mannings "n"	Q Allow.	AutoCad Depth (in)	AutoCad Velocity (fps)	Shear Stress (lb/sf)
0.028	135.80	3.7	4.09	0.77

Permanent liner calculations (Q10)				
Mannings "n"	Q Allow.	AutoCad Depth (in)	AutoCad Velocity (fps)	Shear Stress (lb/sf)
0.030	126.75	4.4	4.17	0.92

Permanent Liners				
*Material	Recommended n Values for Flow Depth			Allowable Shear Stress (lb/ft ²)
	0-0.5 ft	0.5-2.0 ft	> 2.0 ft	
Vegetated				
C350 Phase 2	0.044	0.044	0.044	4.50
C300 Phase 2	0.044	0.044	0.044	4.00
C350 Phase 3	0.049	0.049	0.049	8.00
P300 Phase 3	0.049	0.049	0.049	8.00

Permanent Liners		
Material	Recommended n Values	Permissible velocity (fps)
Grass Mixture	0.030	5.00

Selected Temporary Liner: S75

Selected Permanent Liner: Grass Mixture

Channel Report

Hydralflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

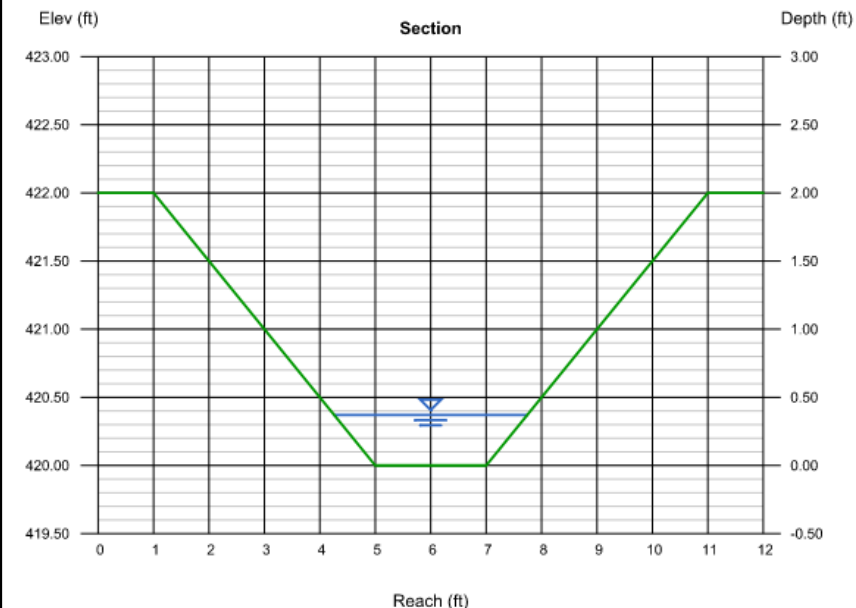
Wednesday, Aug 21 2024

ROADSIDE DITCH #1 10 YR STORM

Trapezoidal
Bottom Width (ft) = 2.00
Side Slopes (z:1) = 2.00, 2.00
Total Depth (ft) = 2.00
Invert Elev (ft) = 420.00
Slope (%) = 4.00
N-Value = 0.030

Highlighted
Depth (ft) = 0.37
Q (cfs) = 4.230
Area (sqft) = 1.01
Velocity (ft/s) = 4.17
Wetted Perim (ft) = 3.65
Crit Depth, Yc (ft) = 0.45
Top Width (ft) = 3.48
EGL (ft) = 0.64

Calculations
Compute by: Known Q
Known Q (cfs) = 4.23





DESIGN STANDARDS

DITCH/SWALE

Design Storm	25-YR
Rainfall Intensity	8.26

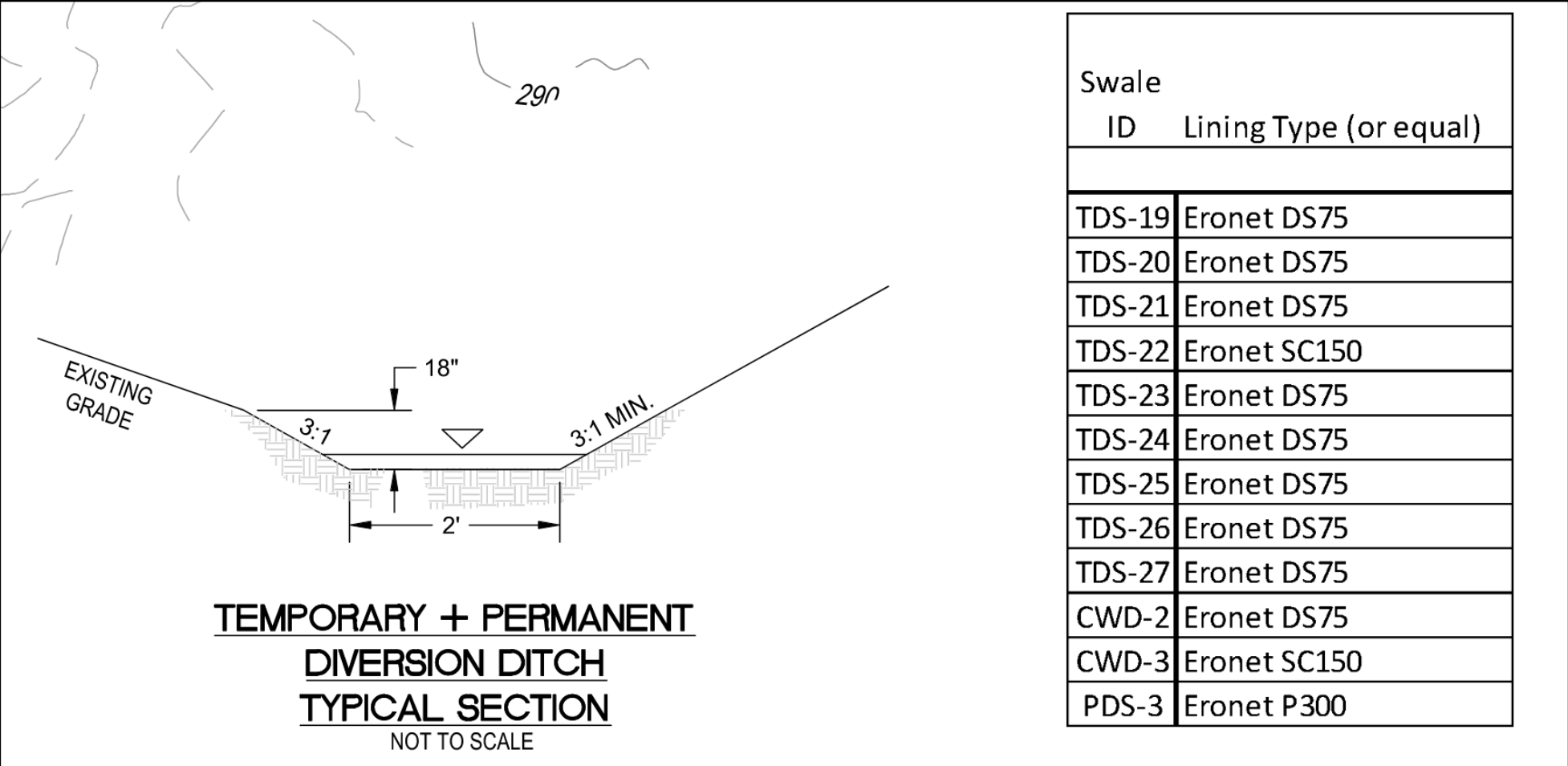
CHANNEL DESIGN INFORMATION													
CHANNEL I.D.	DRAINAGE AREA (AC)	WEIGHTED 'C' COEFFICIENT	CHANNEL FLOW (CFS)	CHANNEL SLOPE (%)	CHANNEL FLOW DEPTH (FT)	CHANNEL MINIMUM DEPTH (FT)	CHANNEL BOTTOM WIDTH (FT)	CHANNEL SIDE SLOPES	CHANNEL VELOCITY (FPS)	ALLOWABLE VELOCITY (FPS)	CHANNEL SHEAR STRESS (PSF)	ALLOWABLE SHEAR STRESS (PSF)	DITCH LINING
CWTD #1	8.25	0.15	10.23	3.00	0.72	1.50	1.00	3:1	4.51	5.00	1.34	1.60	NAG S75BN OR EQUAL
CWTD #2	6.02	0.15	7.46	2.80	0.72	1.50	1.00	3:1	3.29	5.00	1.25	1.60	NAG S75BN OR EQUAL
SCMTD #1	1.65	0.50	6.83	7.00	0.52	1.50	1.00	3:1	5.20	10.00	2.25	2.25	NAG C125BN OR EQUAL
SCMTD #2	0.22	0.50	0.91	5.00	0.65	1.50	1.00	3:1	0.47	8.00	2.03	2.10	NAG SC150BN OR EQUAL
SCMTD #3	1.29	0.50	5.33	3.00	0.67	1.50	1.00	3:1	2.65	5.00	1.25	1.60	NAG S75BN OR EQUAL
SCMTD #4	0.93	0.50	3.83	4.00	0.49	1.50	1.00	3:1	3.11	5.00	1.23	1.60	NAG S75BN OR EQUAL
SCMTB #5	4.03	0.50	16.66	3.90	0.77	1.50	1.00	3:1	6.52	8.00	1.87	2.10	NAG SC150BN OR EQUAL
SCMTD #6	0.91	0.50	3.74	1.60	0.65	1.50	1.00	3:1	1.94	5.00	0.65	1.60	NAG S75BN OR EQUAL
SCMTD #7	1.41	0.15	1.74	3.00	0.77	1.50	1.00	3:1	0.68	5.00	1.44	1.60	NAG S75BN OR EQUAL
SCMTD #8	2.94	0.41	9.92	1.20	0.77	1.50	1.00	3:1	3.88	5.00	0.57	1.60	NAG S75BN OR EQUAL
SCMTB #9	4.66	0.50	19.24	1.00	0.65	1.50	1.00	3:1	9.99	10.00	0.40	2.25	NAG C125BN OR EQUAL
SCMTD #10	1.46	0.50	6.02	2.20	0.77	1.50	1.00	3:1	2.35	5.00	1.05	1.60	NAG S75BN OR EQUAL

Ditch depth, bottom width, sideslopes, and stabilization liner should be in plan.



DESIGN STANDARDS

DITCH/SWALE



Ditch depth, bottom width, sideslopes, and stabilization liner should be in plan.

DESIGN STANDARDS

TEMPORARY DIVERSIONS

Criteria

- Route construction runoff
- Divert offsite drainage around site
- Similar design to ditches
- Drainage area < 5 acres
- Design to 10-year storm

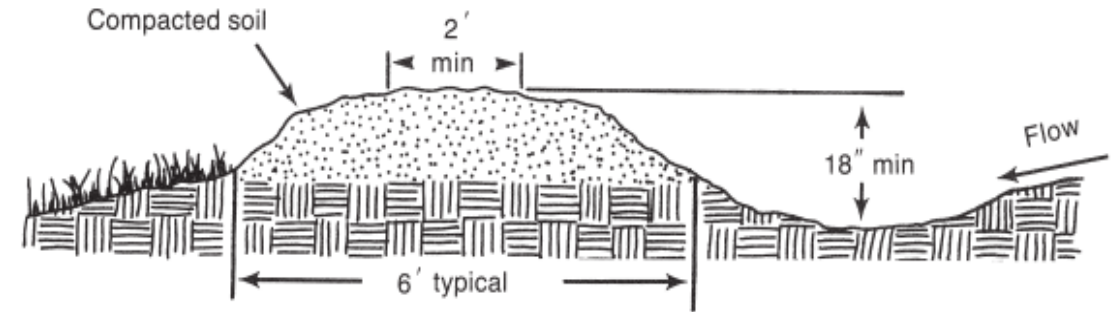


Figure 6.20a Temporary earthen diversion dike.

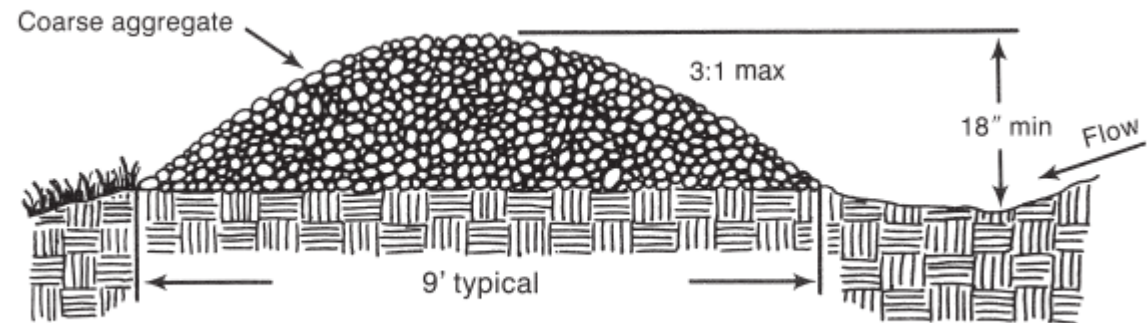


Figure 6.20b Temporary gravel diversion dike for vehicle crossing (modified from Va SWCC).

DESIGN STANDARDS

WATER BARS

Criteria

- Used: dirt roadways, utility corridors, and approaches to surface waters
- Similar design to diversions
- Spacing based on slope

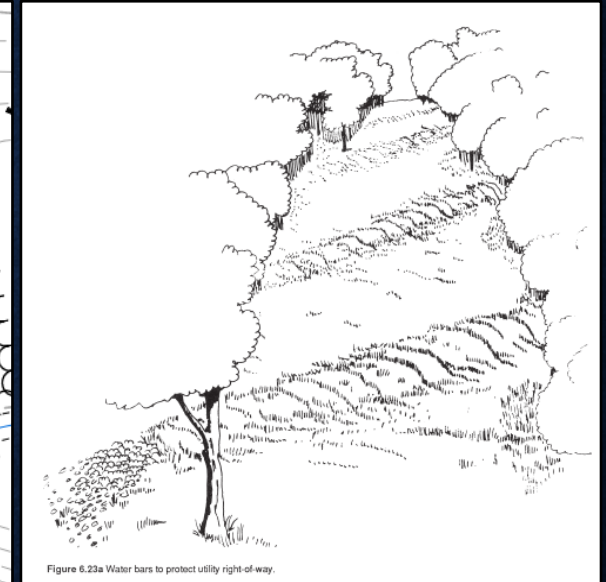
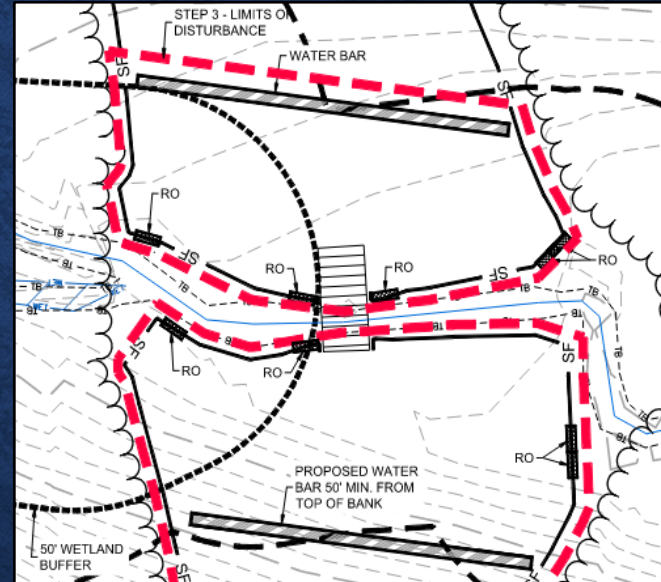
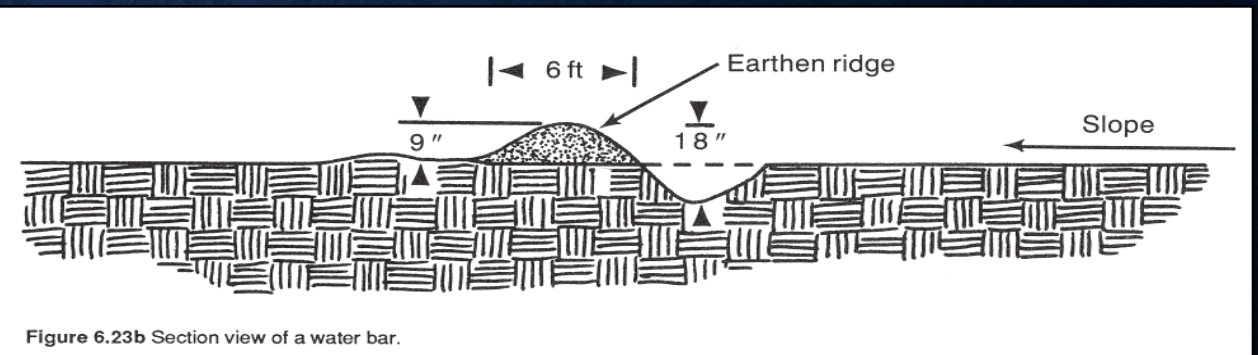


Table 6.23a
Spacing of Water Bars on
Right-of-Way Less than
100 ft Wide

Slope (%)	Spacing (Ft)
<5	125
5 to 10	100
10 to 20	75
20 to 35	50
>35	25

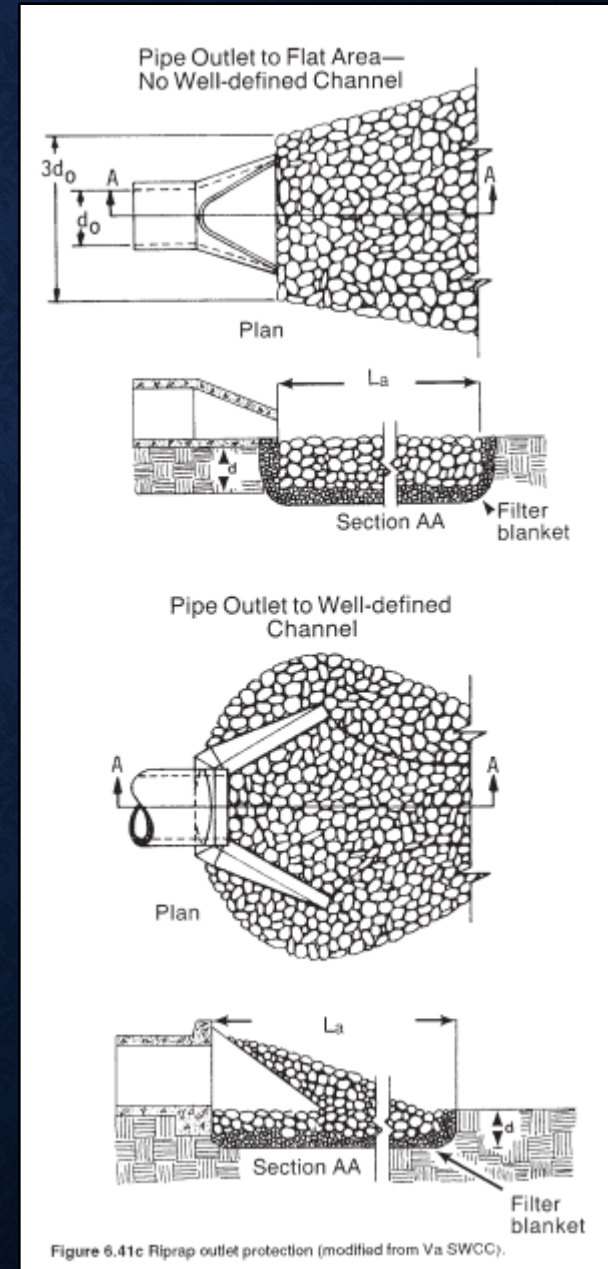
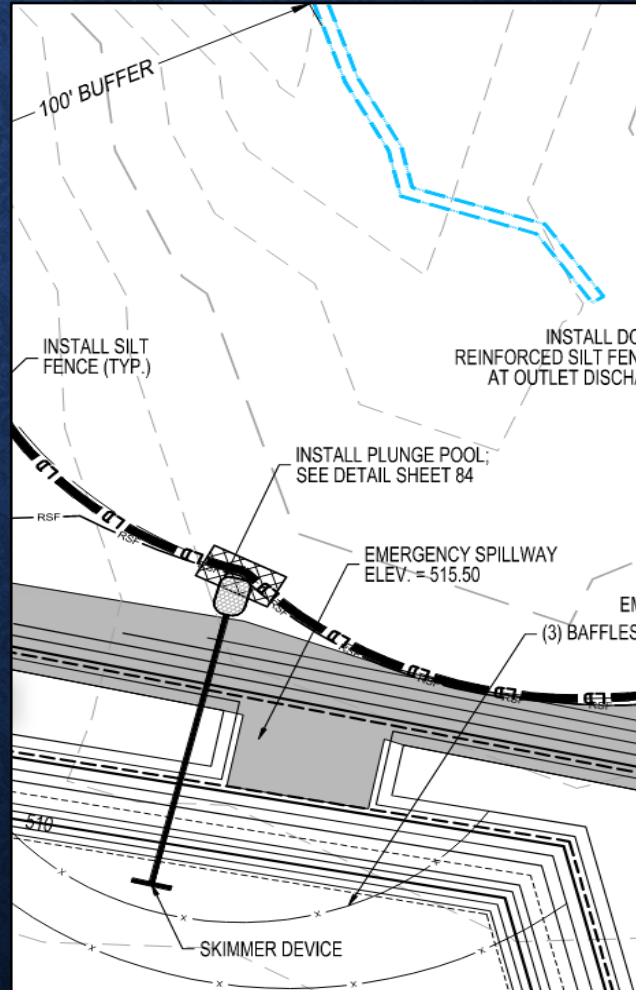


DESIGN STANDARDS

OUTLET PROTECTION

Criteria

- Channel, diversion, and conduit outlets must be stable
- Exit velocity for required design must be non-erosive
- Required design = conveyance
- Filter fabric underliner

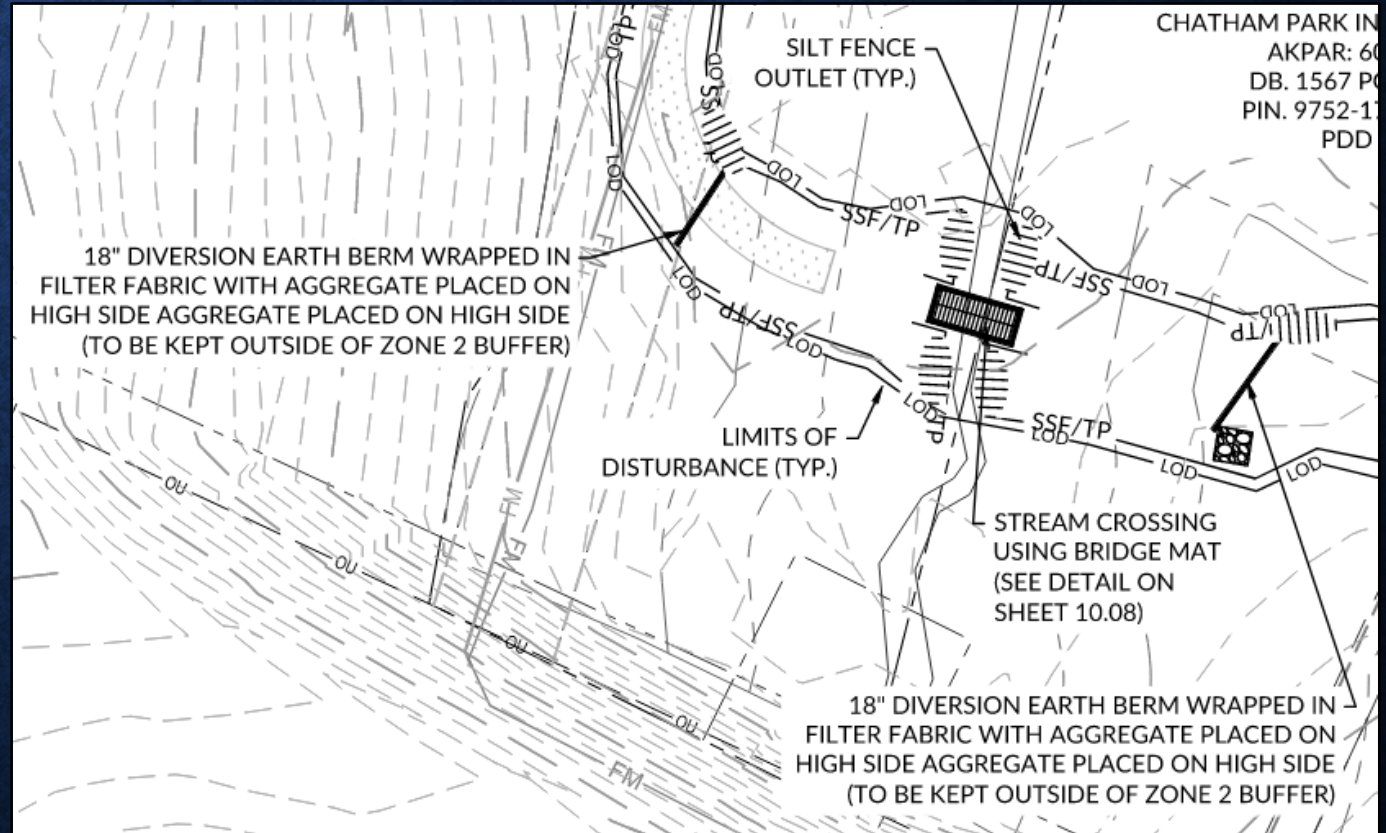


DESIGN STANDARDS

TEMPORARY STREAM CROSSING

Planning Considerations

- Try to avoid crossing streams
- If possible, complete development separately on each side
- Installed perpendicular to stream
- Often most sensitive area on site

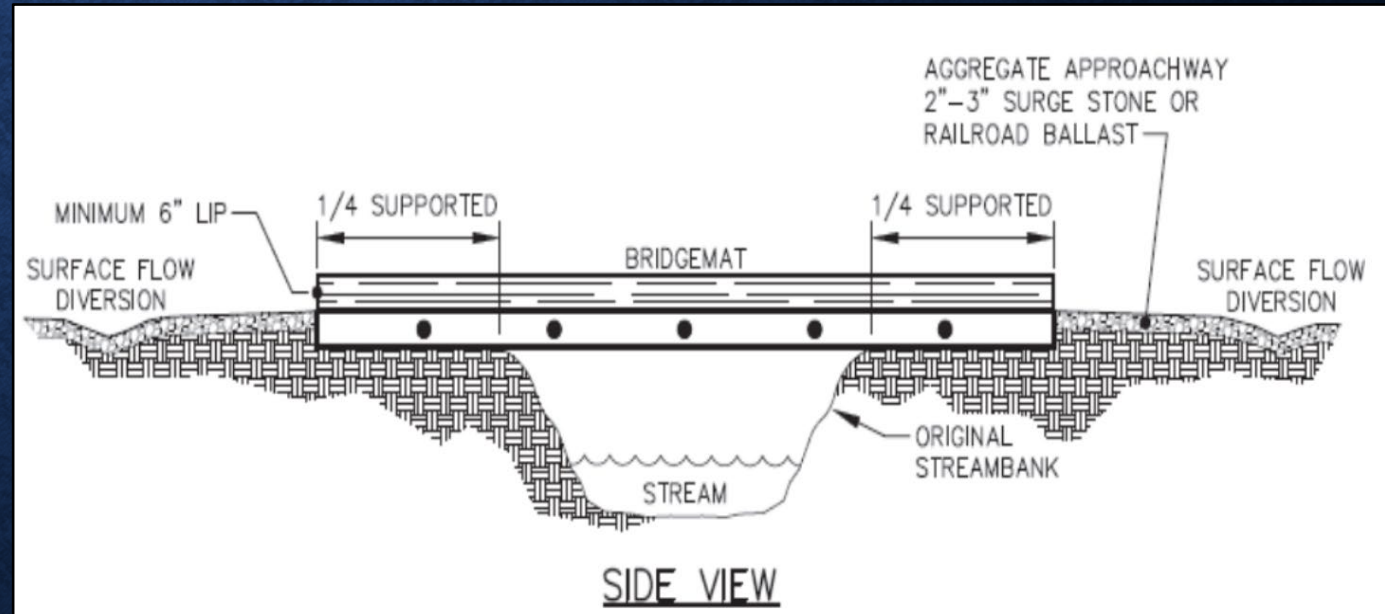


DESIGN STANDARDS

TEMPORARY STREAM CROSSING - BRIDGEMAT

Planning Considerations

- Preferred Method:
 - Least disturbance to stream bed, banks, and surrounding area
 - Often no stream impacts
 - Least obstruction to flow & fish migration
- Must support expected loads

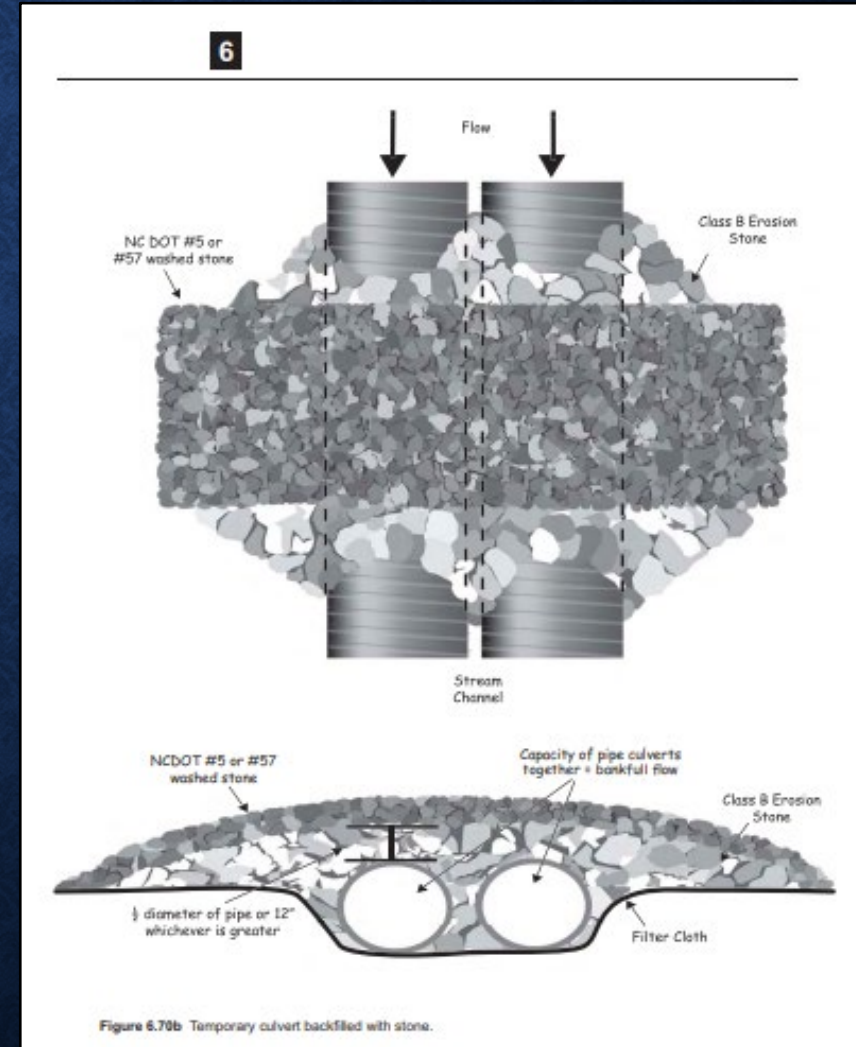


DESIGN STANDARDS

TEMPORARY STREAM CROSSING - PIPED

Planning Considerations

- Adaptable to most site situations
- Safely support heavy loads
- Installation and removal causes considerable disturbance to stream and surrounding area
- Greatest obstruction to flood flows
- Subject to blockage and washout

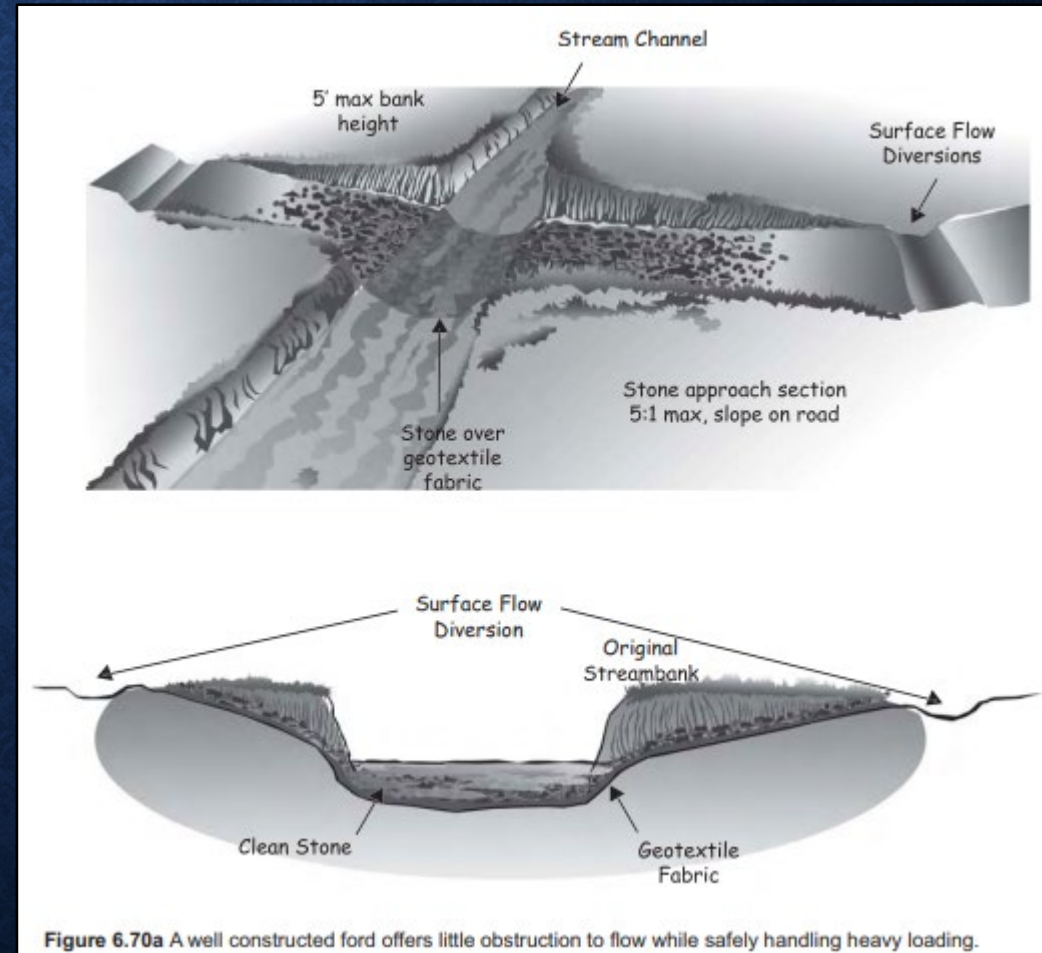


DESIGN STANDARDS

TEMPORARY STREAM CROSSING - FORD

Planning Considerations

- Used:
 - For steep areas subject to flash flooding
 - Normal flows < 3 inches deep
 - Intermittent streams
 - Only when crossings are infrequent
 - Wide, shallow watercourses
- Safely handle heavy loads
- Little to no obstruction of flow
- Could involve major disturbance in-stream
- Approach area subject to erosion

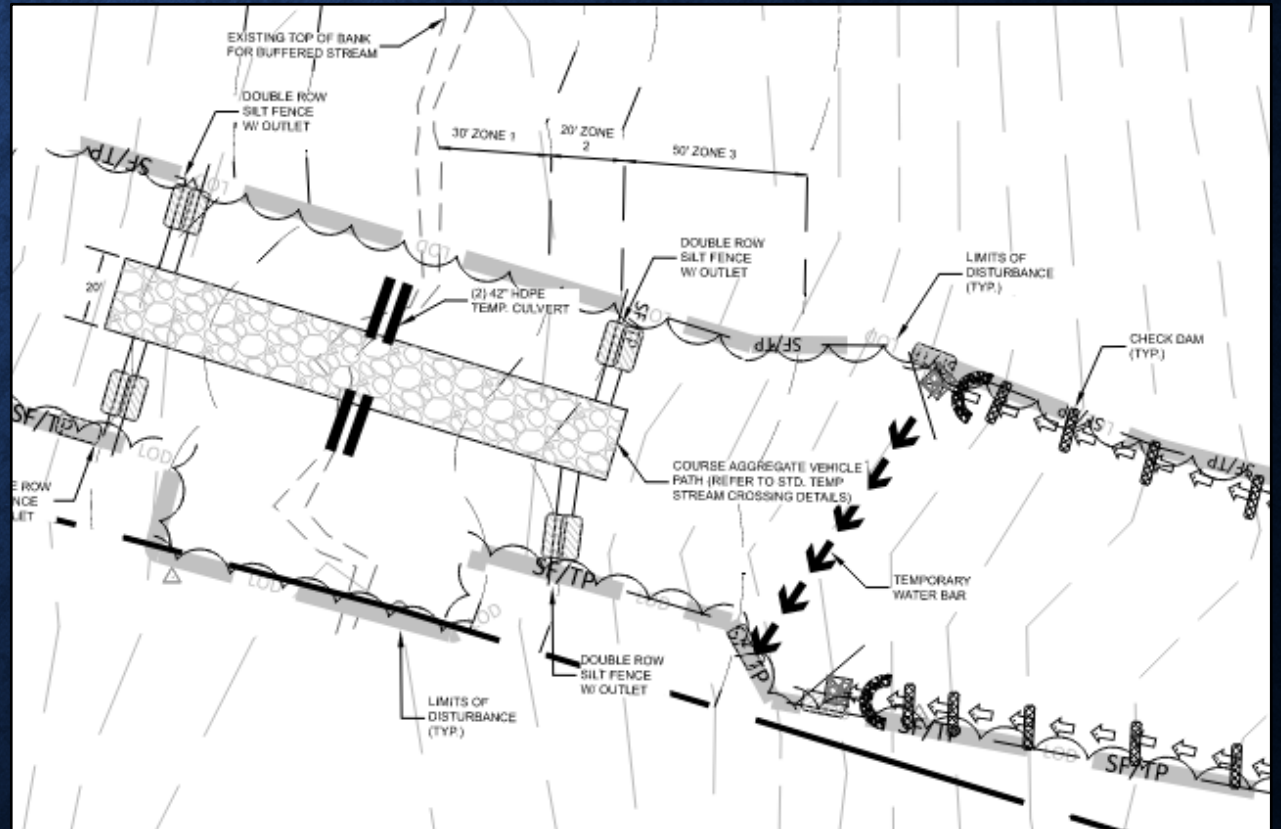


DESIGN STANDARDS

TEMPORARY STREAM CROSSING

Criteria

- Anticipated life = 1 year
- Minimum design: Pass bankfull flow or peak flow from 2-yr storm (10-25 yr. recommended)
- Consider overflow areas for larger storms
- Design flow velocity non-erosive for receiving stream
- Design other EC measures associated with stream crossing to 10-year peak storm runoff

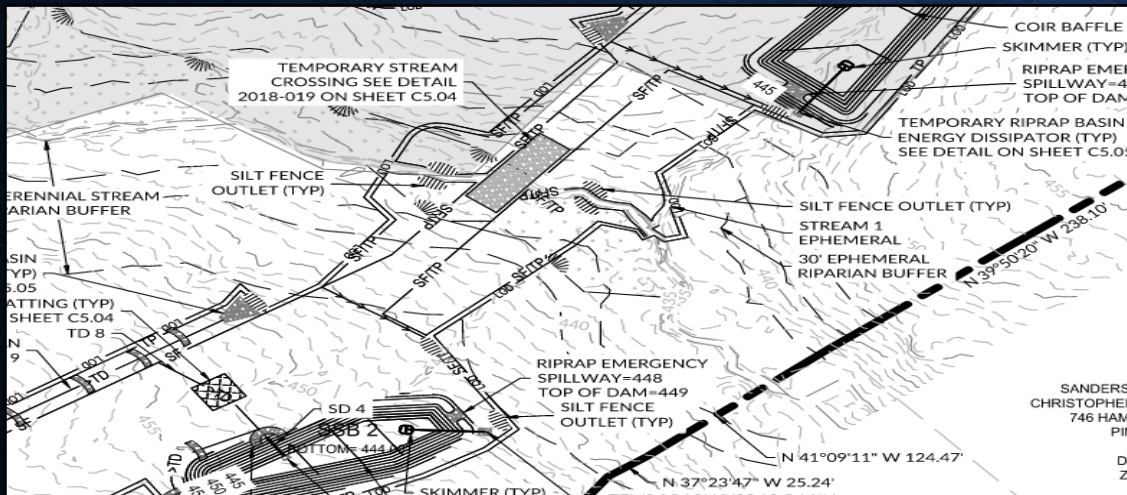
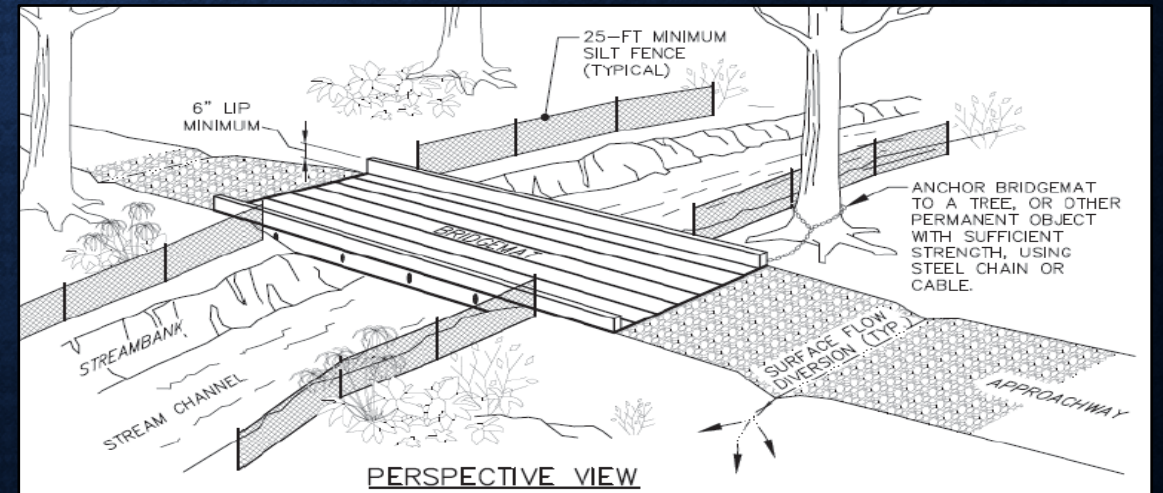
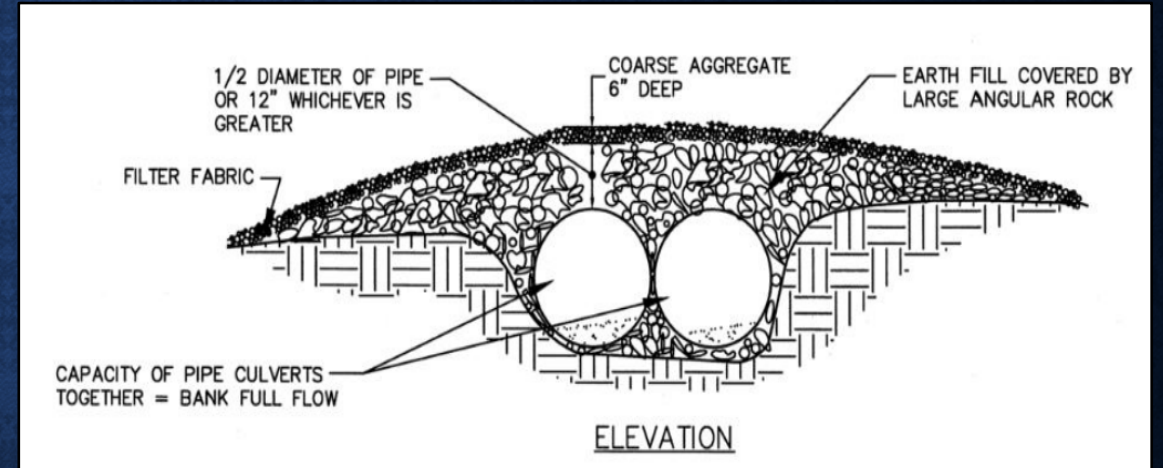


DESIGN STANDARDS

TEMPORARY STREAM CROSSING

Criteria

- Silt fence at four corners
- Coarse aggregate approachways
- Lip to prevent sedimentation to stream
- Divert surface water



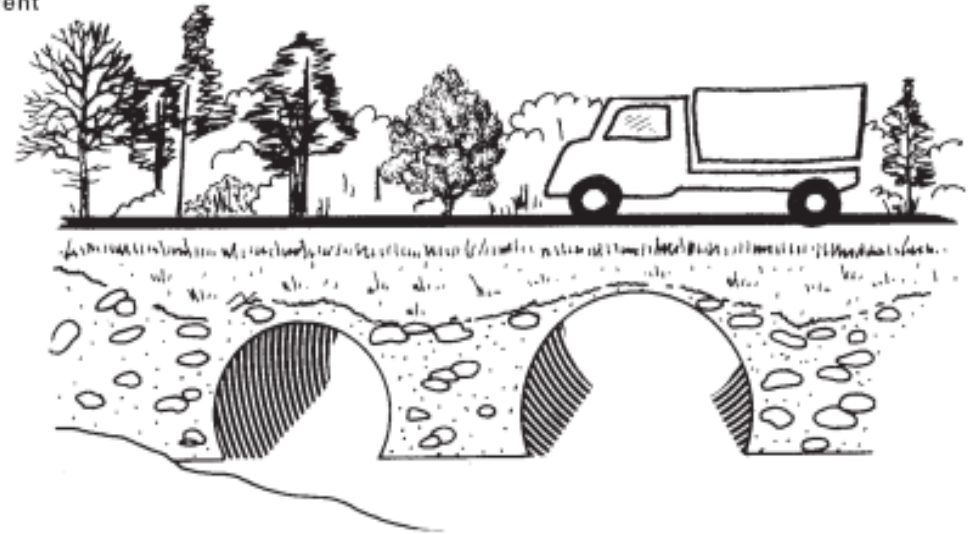
DESIGN STANDARDS

PERMANENT STREAM CROSSING

Planning Considerations

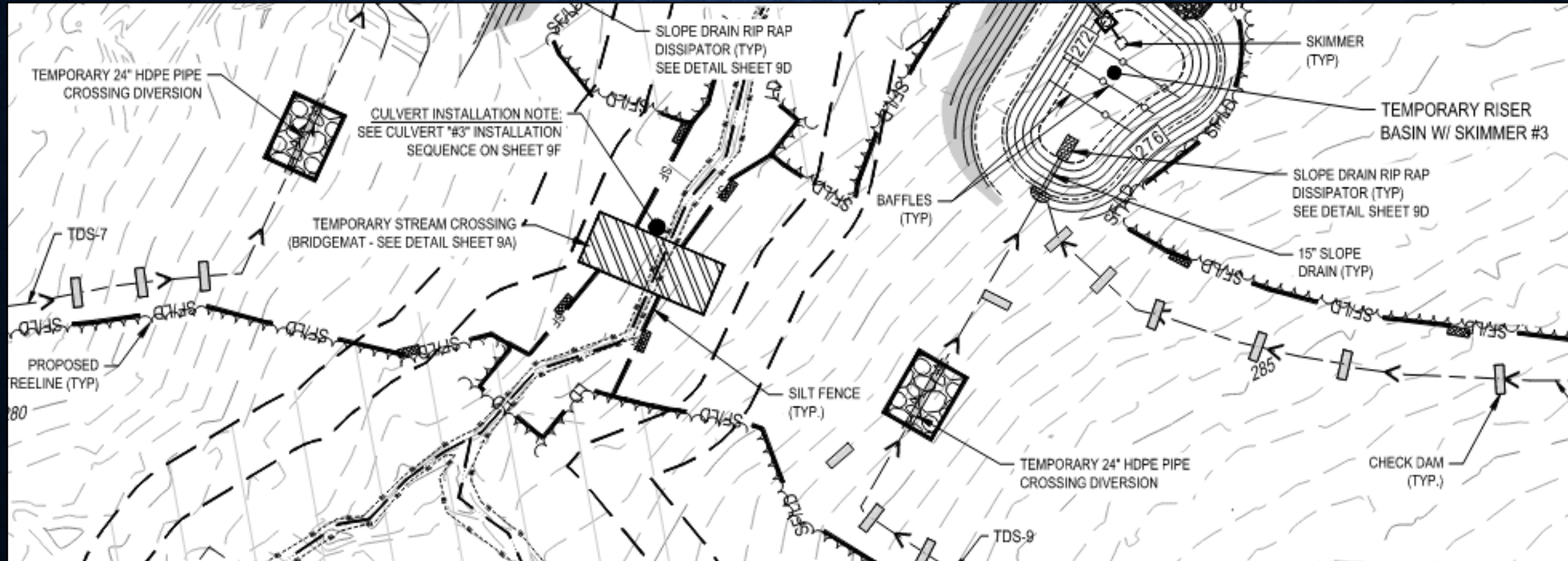
- Locate in higher, better drained sections of stream to minimize flooding
- Permanent protection measures where road water enters stream to prevent erosion
- Stream section at crossing protected from erosion due to flood flows

Figure 6.71a Permanent stream crossing with culverts designed to prevent overtopping.



DESIGN STANDARDS

STREAM CROSSINGS – EXAMPLE 1 (PUMP AROUND)





DESIGN STANDARDS

STREAM CROSSINGS – EXAMPLE 1 (PUMP AROUND)

CHANNEL IMPACT SEQUENCE

GENERAL: WITH STAGE 1 CONSTRUCTION, CROSS STREAM WITH BRIDGEMAT TO ACCESS REMAINDER OF SITE AND INSTALL INITIAL TEMPORARY EROSION CONTROL MEASURES. SILT FENCE TO BE INSTALLED OUTSIDE OF TOP OF BANKS OF STREAM WITH REINFORCED SILT FENCE OUTLETS AS SHOWN.

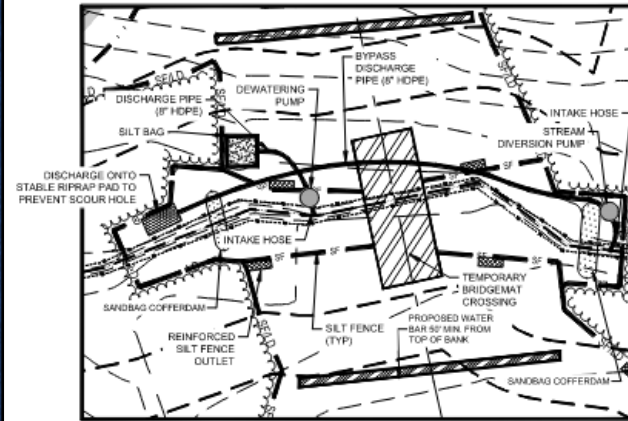
THE CONTRACTOR SHALL NOT IMPACT THE STREAM UNTIL THE CULVERT AND HEADWALLS HAVE BEEN DELIVERED ON-SITE AND READY FOR INSTALLATION. WORK WITHIN THE STREAM SHOULD ONLY BE UNDERTAKEN WITH THE EXPECTATION OF 4-5 DAYS OF DRY WEATHER AS FORECASTED BY NWS.

STEP 1: AFTER UPSTREAM DIVERSIONS ARE FUNCTIONING; INSTALL WATER BARS (DIVERSION) AS SHOWN. NO ACTIVITY ALLOWED WITHIN STREAM CHANNEL. LAY 8" FUSED HDPE BYPASS DISCHARGE PIPE ON TOP OF GROUND AND POSITION STREAM DIVERSION PUMP UPSTREAM OF COFFER DAM. INSTALL SILT BAG AT DOWNSTREAM END OF 8" HDPE BYPASS DISCHARGE PIPE (OUTSIDE TOP OF BANK) INSTALL UPSTREAM AND DOWNSTREAM SANDBAG COFFER DAMS. CLEARING ONLY AS NECESSARY WITHIN THE STREAM AND NORTHERN TOP OF BANK TO INSTALL COFFER DAM AND DOWNSTREAM CLASS II RIP RAP DISSIPATOR. ACTIVATE BYPASS PUMPING SYSTEM.

STEP 2: WHILE MAINTAINING BYPASS PUMPING SYSTEM, EXCAVATE CHANNEL FOR CULVERT PIPE STARTING FROM DOWNSTREAM END. PLACE ON A 4,000 PSI CONCRETE MUD SILL AS DIRECTED BY GEOTECHNICAL ENGINEER. EXCAVATE GRADE 4' BEYOND OUTSIDE EDGE OF RCP ON EACH SIDE TO ALLOW ADEQUATE WIDTH FOR RECOMPACTION. INSTALL RCP STARTING FROM DOWNSTREAM END THEN HEADWALLS. UTILIZE EARTHEN MATERIAL STOCKPILED AS SELECT BACKFILL TO PIPE SPRINGLINE (APPROVED BY GEOTECHNICAL ENGINEER). INSTALL RIP RAP DISSIPATOR. IMPACT NORTHERN CHANNEL TOP OF BANK AND BEYOND ONLY AS NECESSARY TO INSTALL PIPE AND HEADWALL.

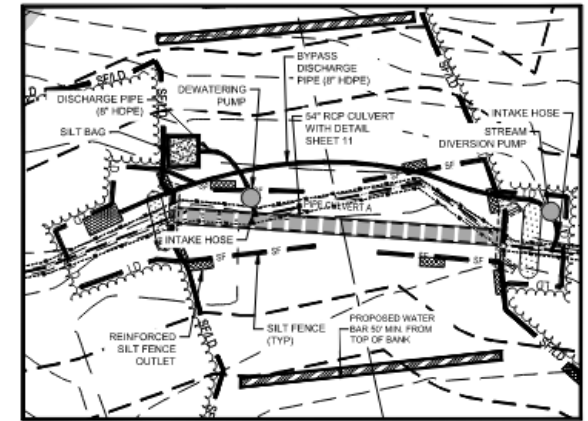
STEP 3: AFTER PIPE AND HEADWALLS ARE COMPLETE AND BACKFILLED; INSTALL SILT FENCING ALONG HEADWALL AND ON NORTH SIDE OF EXISTING CHANNEL BANK AS SHOWN WITH REINFORCED SILT FENCE OUTLETS. REMOVE UPPER COFFER DAM AND REPLACE WITH CLASS II RIP RAP FILTER WITH WASH STONE FACE (ROCK CHECK DAM WITH CLASS II RIP RAP - SEE DETAIL SHEET 9C).

STEP 4: AFTER TEMPORARY GROUND STABILIZATION IS ACHIEVED; REMOVE UPSTREAM RIP RAP FILTERS AND MAINTAIN SILT FENCE AT TOE OF SLOPES AND ALONG HEADWALLS.



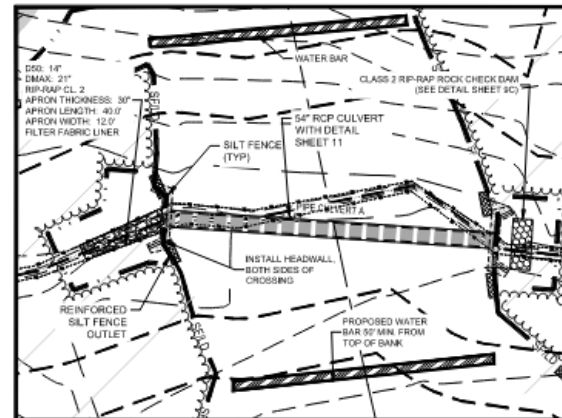
IMPACT - STEP 1

SCALE: 1" = 30'-0"



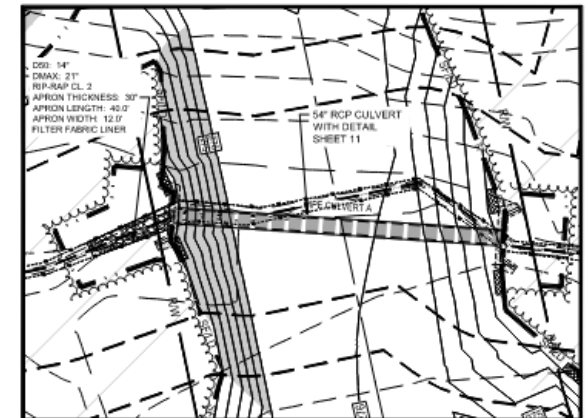
IMPACT - STEP 2

SCALE: 1" = 30'-0"



IMPACT - STEP 3

SCALE: 1" = 30'-0"



IMPACT - STEP 4

SCALE: 1" = 30'-0"



DESIGN STANDARDS

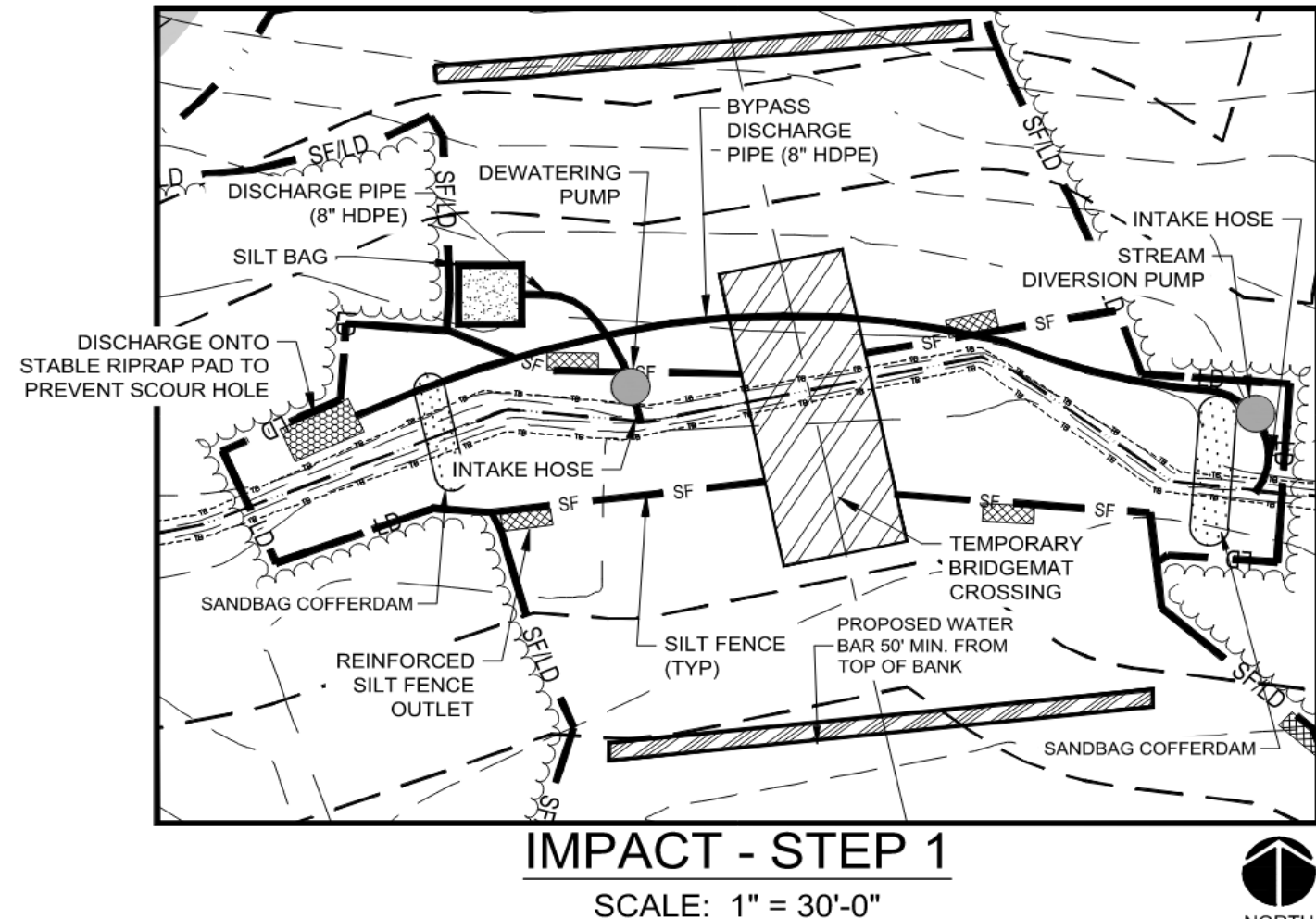
STREAM CROSSINGS – EXAMPLE 1 (PUMP AROUND)

CHANNEL IMPACT SEQUENCE

GENERAL: WITH STAGE 1 CONSTRUCTION, CROSS STREAM WITH BRIDGEMAT TO ACCESS REMAINDER OF SITE AND INSTALL INITIAL TEMPORARY EROSION CONTROL MEASURES. SILT FENCE TO BE INSTALLED OUTSIDE OF TOP OF BANKS OF STREAM WITH REINFORCED SILT FENCE OUTLETS AS SHOWN.

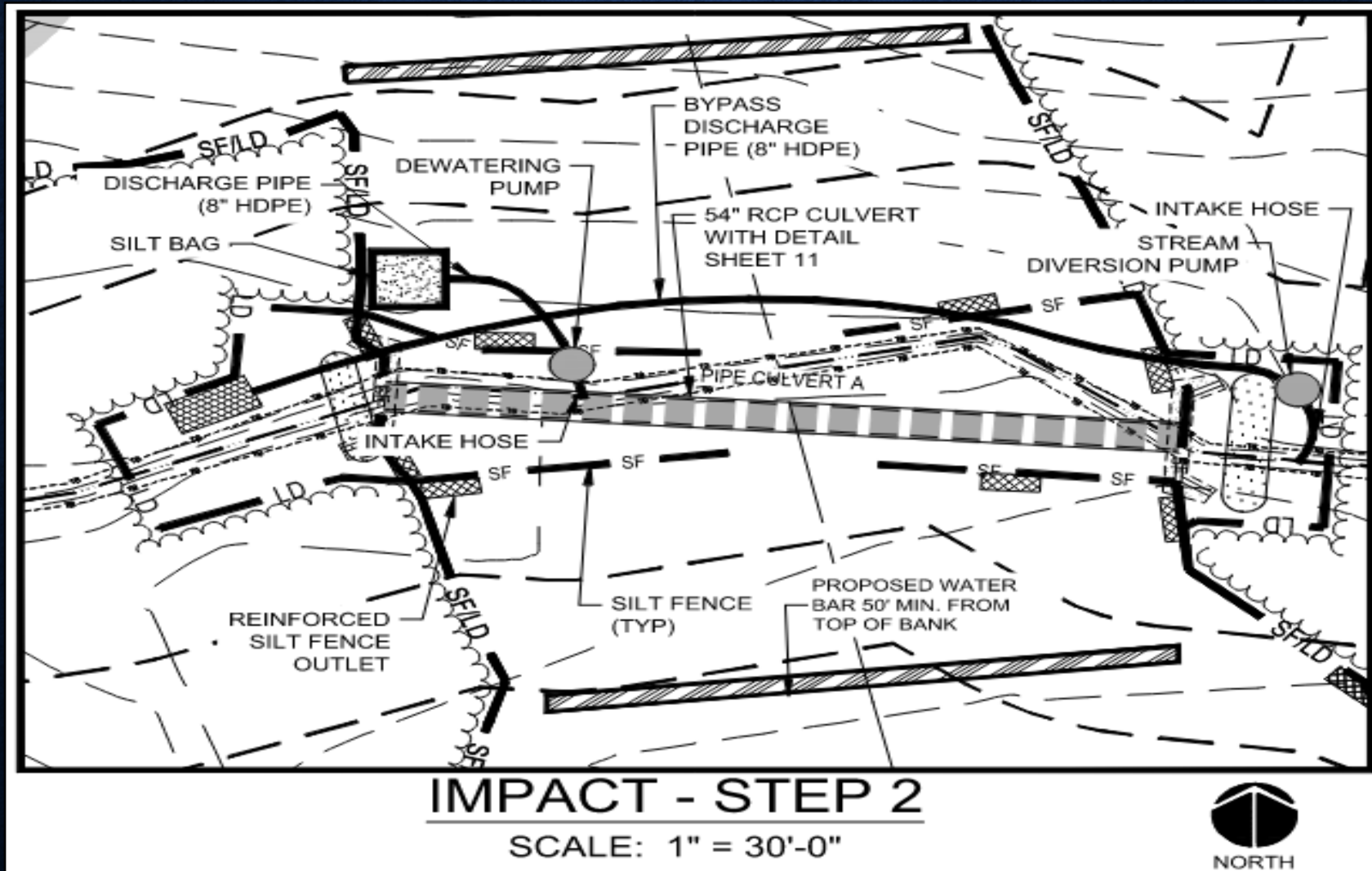
THE CONTRACTOR SHALL NOT IMPACT THE STREAM UNTIL THE CULVERT AND HEADWALLS HAVE BEEN DELIVERED ON-SITE AND READY FOR INSTALLATION. WORK WITHIN THE STREAM SHOULD ONLY BE UNDERTAKEN WITH THE EXPECTATION OF 4-5 DAYS OF DRY WEATHER AS FORECASTED BY NWS.

STEP 1: AFTER UPSTREAM DIVERSIONS ARE FUNCTIONING; INSTALL WATER BARS (DIVERSION) AS SHOWN. NO ACTIVITY ALLOWED WITHIN STREAM CHANNEL. LAY 8" FUSED HDPE BYPASS DISCHARGE PIPE ON TOP OF GROUND AND POSITION STREAM DIVERSION PUMP UPSTREAM OF COFFER DAM. INSTALL SILT BAG AT DOWNSTREAM END OF 8" HDPE BYPASS DISCHARGE PIPE (OUTSIDE TOP OF BANK) INSTALL UPSTREAM AND DOWNSTREAM SANDBAG COFFER DAMS. CLEARING ONLY AS NECESSARY WITHIN THE STREAM AND NORTHERN TOP OF BANK TO INSTALL COFFER DAM AND DOWNSTREAM CLASS II RIP RAP DISSIPATOR. ACTIVATE BYPASS PUMPING SYSTEM.



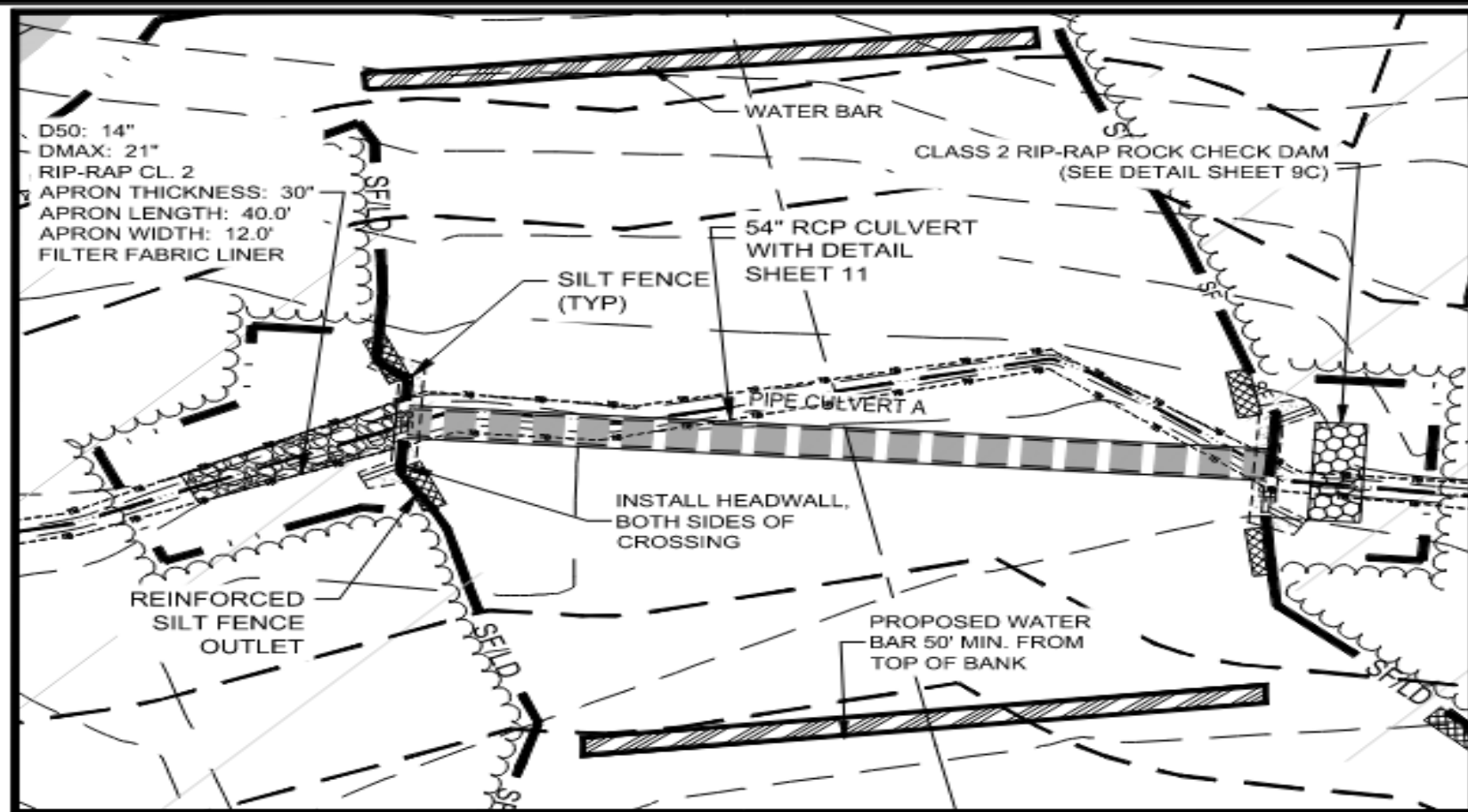
DESIGN STANDARDS

STREAM CROSSINGS – PUMP AROUND



DESIGN STANDARDS

STREAM CROSSINGS – PUMP AROUND



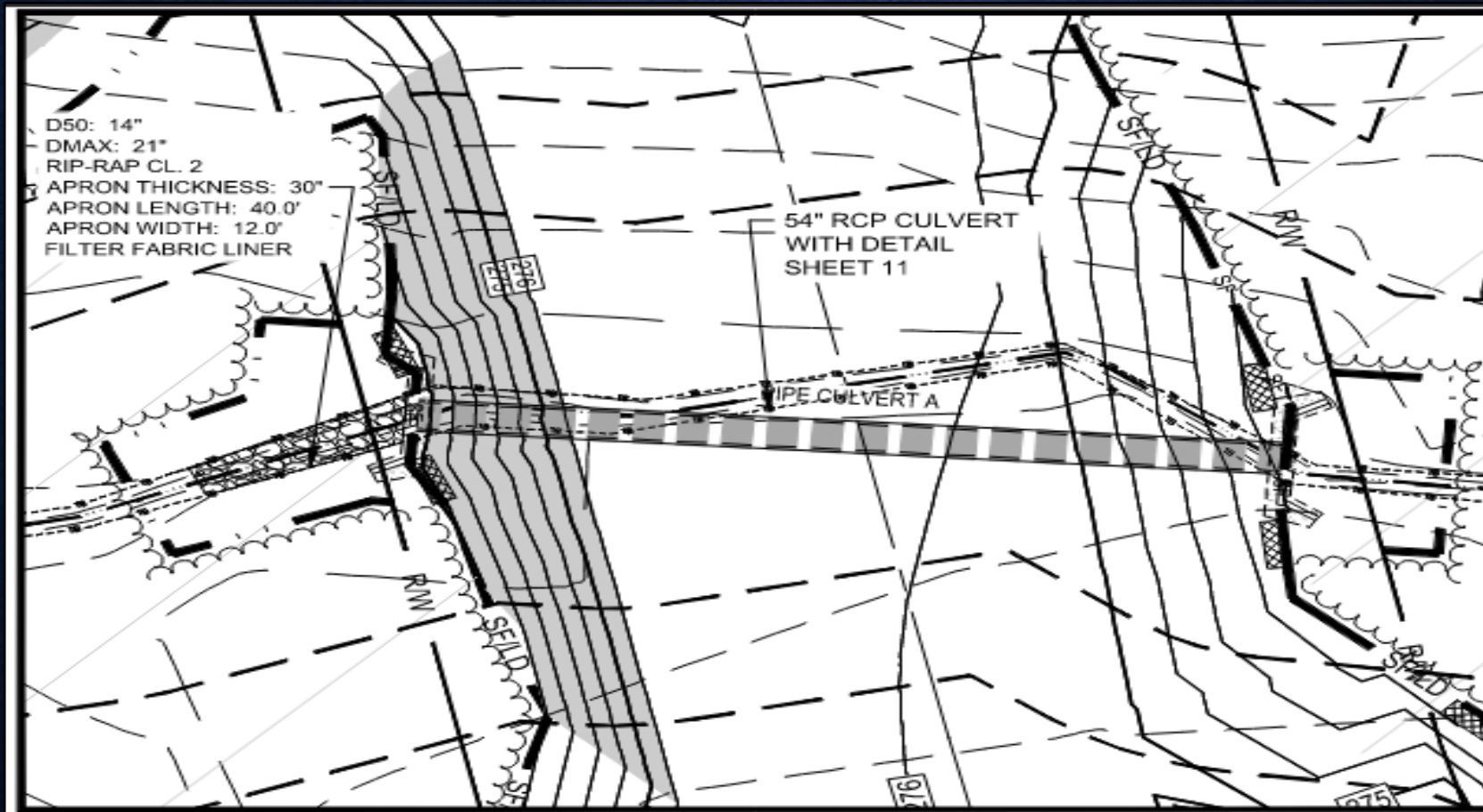
IMPACT - STEP 3

SCALE: 1" = 30'-0"



DESIGN STANDARDS

STREAM CROSSINGS – PUMP AROUND



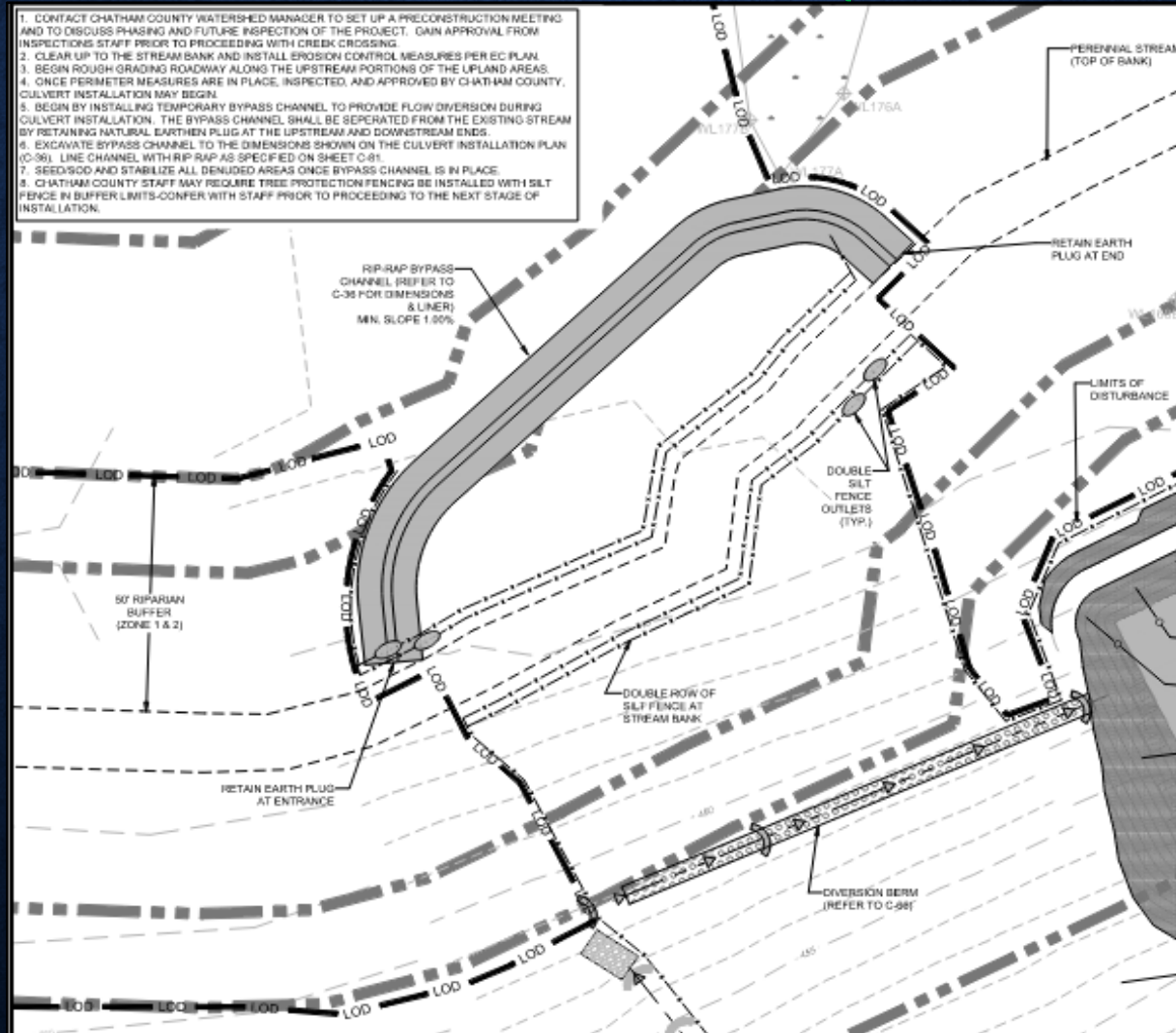
IMPACT - STEP 4

SCALE: 1" = 30'-0"

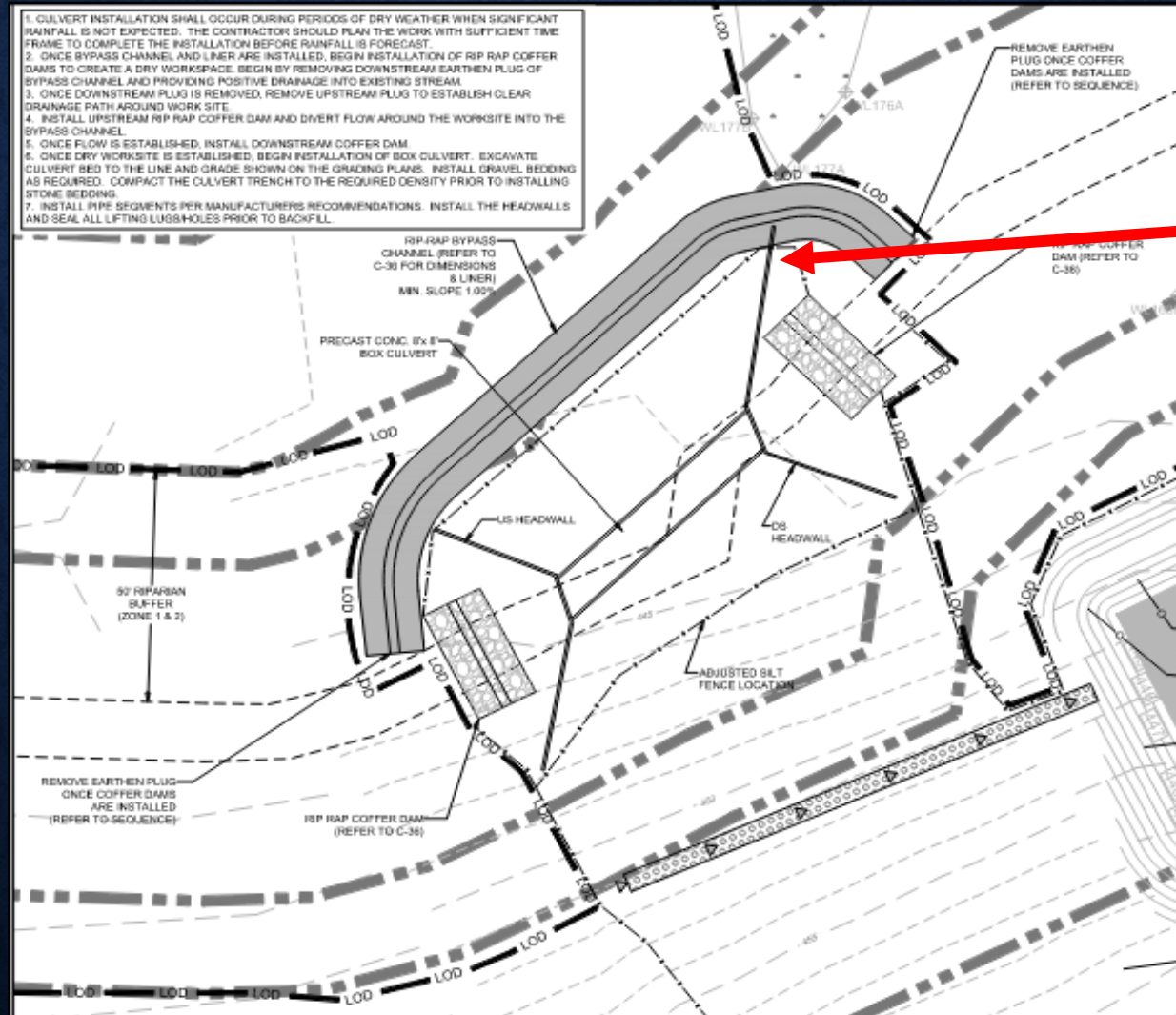


DESIGN STANDARDS

STREAM CROSSINGS – EXAMPLE 2 (BYPASS CHANNEL)



STREAM CROSSINGS – EXAMPLE 2 (BYPASS CHANNEL)





DESIGN STANDARDS

CONSTRUCTION SEQUENCE



Detailed

- Instructs what to do and when
- Presented in stages
- Lists required inspections
- Instructs to clear only as necessary to install initial measures

EROSION CONTROL SEQUENCE - STAGE 1

1. OBTAIN GRADING PERMIT / FINAL APPROVAL FROM CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT.
2. CONTACT THE CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT TO SET UP A PRE-CONSTRUCTION MEETING PRIOR TO ANY LAND DISTURBANCE WORK PERFORMED.
3. THE CONTRACTOR SHALL KEEP A COPY OF THE APPROVED EROSION CONTROL PLANS ON SITE AT ALL TIMES.
4. DELINEATE THE PROPOSED TREELINE/DISTURBED LIMIT LINE AND INSTALL ALL PROPOSED SILT FENCE AND TREE PROTECTION FENCE THROUGHOUT THE ENTIRE PROJECT AREA ALONG WITH TEMPORARY CONSTRUCTION ENTRANCE. INSTALL CLEAN WATER DIVERSIONS / BERMS.
5. INSTALL TEMPORARY SILT FENCE OUTLETS IN ALL LOCATIONS AS SHOWN ON THE PLANS. FIELD LOCATE SILT FENCE OUTLETS AT APPROPRIATE LOW SPOTS AT THE SHOWN LOCATIONS.
6. ONCE THE PREVIOUS MEASURES ARE INSTALLED, CLEAR ONLY AS REQUIRED TO INSTALL TEMPORARY SKIMMER BASINS, TEMPORARY DIVERSION BERMS/ DITCHES, AND OTHER TEMPORARY MEASURES AS SHOWN ON THE APPROVED PLAN.
7. INSTALL CHECK DAMS AND TEMPORARY MATTING IN TEMPORARY DIVERSION DITCHES.
8. THE CONTRACTOR SHALL STABILIZE ALL DIVERSIONS, AND RISER/SKIMMER BASINS IMMEDIATELY UPON THEIR CONSTRUCTION.
9. ALL MEASURES SHALL BE INSTALLED AND INSPECTED FOR COMPLIANCE BY CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT PRIOR TO COMMENCEMENT OF ANY PROPOSED R.O.W. CLEARING/GRUBBING AND EXCAVATION. CALL FOR AN INSPECTION FOR COMPLIANCE PRIOR TO ANY ADDITIONAL CLEARING. IF SITE IS APPROVED, PROCEED TO STAGE 2.

*CONTRACTOR TO REESTABLISH TEMPORARY DIVERSION SWALES AT THE END OF EACH DAY TO ENSURE DRAINAGE TO THE APPROPRIATE STRUCTURE.

DESIGN STANDARDS

CONSTRUCTION SEQUENCE



Thoughtful

- Explains transition between stages
- Includes total scope of work
- Clarifies any gray areas
- Define how long sediment basins remain
- Clearly state when conversion can occur
- Steps to close plan/permit

CONSTRUCTION SEQUENCE SCHEDULE

Definition A specified work schedule that coordinates the timing of land-disturbing activities and the installation of erosion and sedimentation control measures.

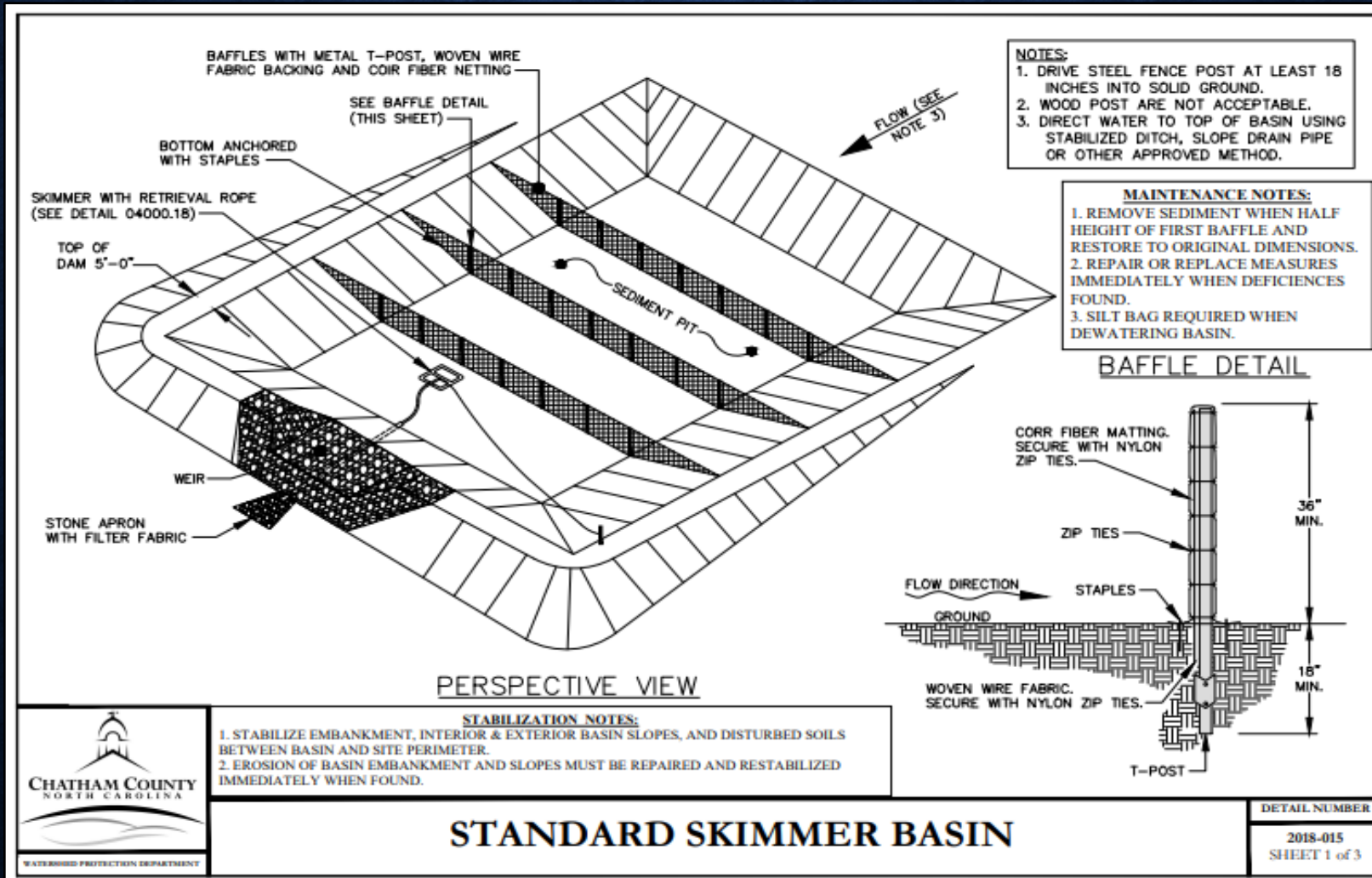
EROSION CONTROL SEQUENCE - STAGE 2

1. ONCE APPROVAL FOR PHASE 1 IS OBTAINED BEGIN CLEARING AND GRUBBING OF REMAINING DISTURBED AREAS AS SHOWN ON APPROVED PLAN.
2. SEE CULVERT #3 INSTALLATION SEQUENCE ON DETAIL SHEET 9F FOR STREAM CROSSING.
3. BEGIN INSTALLATION OF NEW STORM DRAINAGE NETWORKS AND SITE UTILITIES AS SITE IS BEING BROUGHT UP TO FINAL GRADE.
4. WHEN EXCAVATING FOR INSTALLATION OF STORM DRAINAGE NETWORKS, INSTALL FOREBAYS AT TEMPORARY RISER BASINS. SEE POND DETAIL SHEETS 6A-6D FOR DETAILS ON FOREBAYS.
5. CONSTRUCTION RUNOFF IN THE CULVERT F / STORM NETWORK G AREA TO BE DIRECTED TO TSB #3 AND TSB #4 FROM STAGE 1 DESIGN UNTIL CULVERT F IS FUNCTIONING AS CLEAN WATER BYPASS PIPE AND STAGE 2 RUNOFF DIRECTED TO TSB #4 IN STAGE 2 CONDITION.
6. ONCE THE PIPES FOR STORM SYSTEMS ARE COMPLETE, INSTALL PERMANENT DIVERSION DITCHES AND OTHER TEMPORARY MEASURES. INSTALL CHECK DAMS IN EACH AND STABILIZE WITH PERMANENT MATTING IMMEDIATELY.
7. COMPLETE THE INSTALLATION OF THE STORM DRAINAGE NETWORK AS THE SITE COMES TO FINAL GRADE. PLACE INLET PROTECTION AND HORSESHOE INLET PROTECTION AROUND STORM DRAINAGE STRUCTURES ONCE THEY ARE COMPLETED.
8. THE TEMPORARY SKIMMER BASINS INTERIOR TO THE SITE ARE TO BE REMOVED AND EVENTUALLY FILLED TO FINAL OR ORIGINAL EXISTING GRADE. CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT MUST INSPECT SITE CONDITIONS PRIOR TO REMOVAL OF TEMPORARY SKIMMER BASINS.
9. BRING SITE TO FINAL GRADE AND BEGIN EXCAVATION OF ROADWAYS.
10. CONSTRUCT CURB AND GUTTER AND STONE BASE TO PREP FOR PAVING. INSTALL SIDEWALKS. SEED/SOD AND INSTALL LANDSCAPING ALONG WITH ANY OTHER PERMANENT GROUND COVER.
11. SEED AND MULCH ALL DENUDED AREAS WITHIN 15 WORKING DAYS OF COMPLETION OF ANY PHASE OF CONSTRUCTION. EROSION CONTROL MATTING IS TO BE INSTALLED ON SLOPES 3:1 OR GREATER IMMEDIATELY AFTER SEEDING.
12. PAVE ROADS.
13. REQUEST FINAL APPROVAL BY CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT. IF THE INSPECTOR IS SATISFIED WITH UPSTREAM PERMANENT GROUND COVER, BEGIN REMOVING TEMPORARY EROSION CONTROL MEASURES.
14. AFTER STABILIZATION HAS OCCURRED REMOVE SEDIMENT FROM RISER BASIN/WATER QUALITY POND AND CONVERT TO PERMANENT STRUCTURE. CONTACT CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT FOR INSPECTION PRIOR TO BASIN CONVERSION. ALL WATER IS TO BE PUMPED TO SILT BAG BEFORE DISCHARGING FROM SITE (SEE DETAIL SHEET 9C). CONSTRUCTION RUNOFF FROM THE CONSERVANCY PHASE 1B WILL BE DIRECTED TO TRB #4. DO NOT COMPLETE FINAL CONVERSION UNTIL THAT AREA IS GRADED TO FINAL CONDITION AND STABILIZED.
15. REQUEST FINAL INSPECTION BY ENVIRONMENTAL INSPECTOR TO CLOSE OUR LAND-DISTURBING PERMIT

*CONTRACTOR TO REESTABLISH TEMPORARY DIVERSION SWALES AT THE END OF EACH DAY TO ENSURE DRAINAGE TO THE APPROPRIATE STRUCTURE.

DESIGN STANDARDS

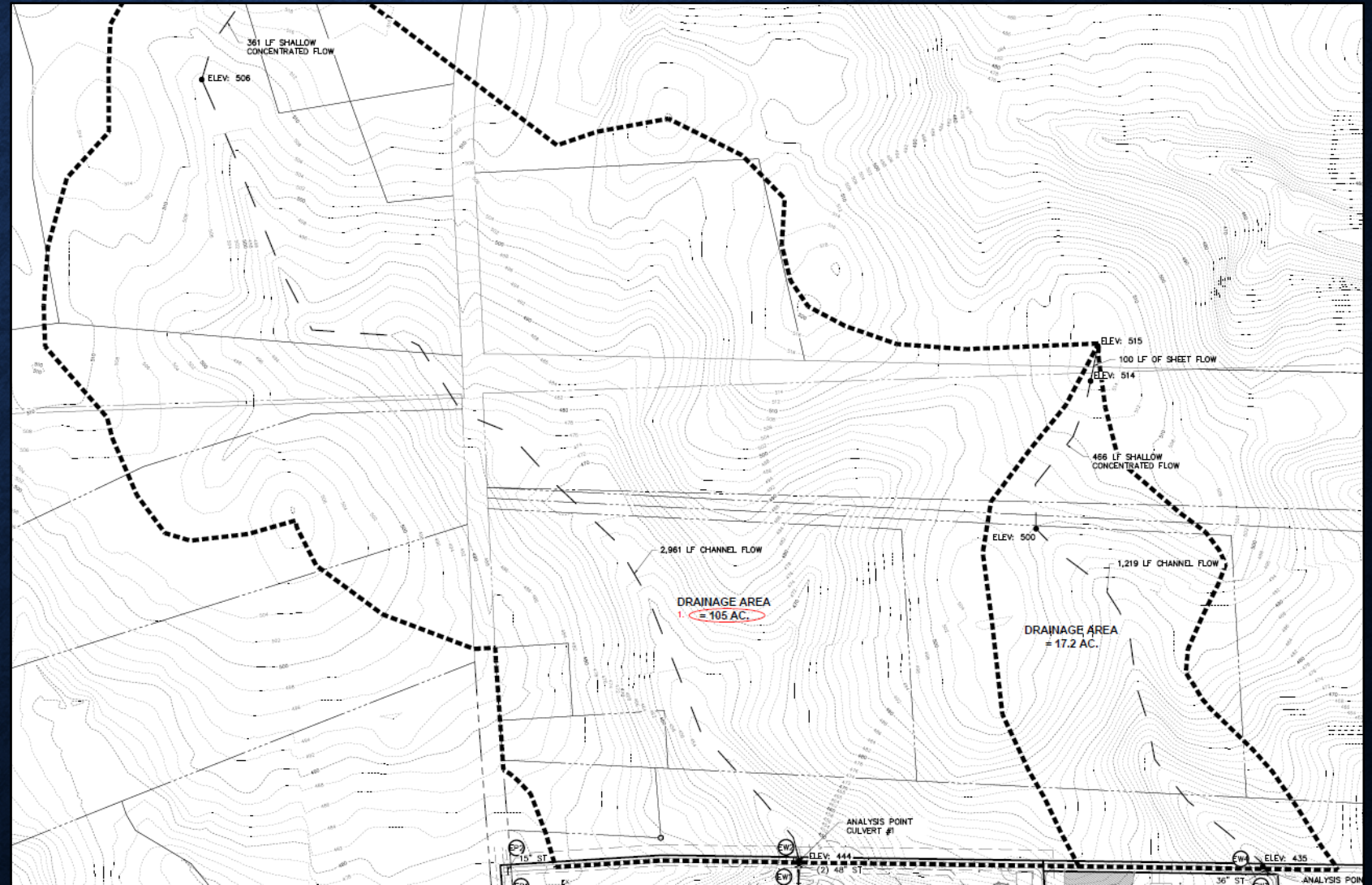
STANDARD DETAILS



DESIGN STANDARDS

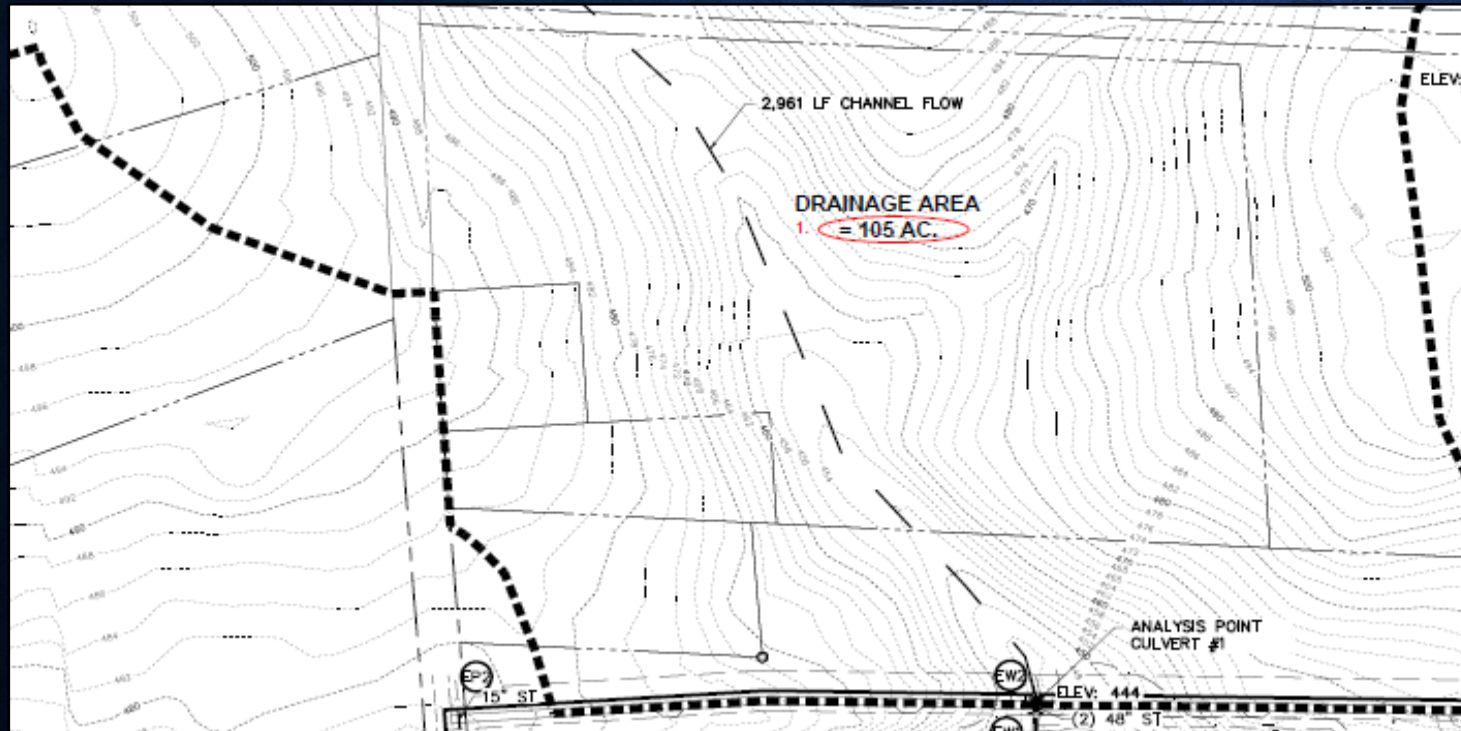
STORM DRAINAGE INFRASTRUCTURE - CULVERTS

- Verify design
- Verify inputs
- Check for overtopping



DESIGN STANDARDS

STORM DRAINAGE INFRASTRUCTURE - CULVERTS



Summary for Subcatchment D1: DA to Culvert 1

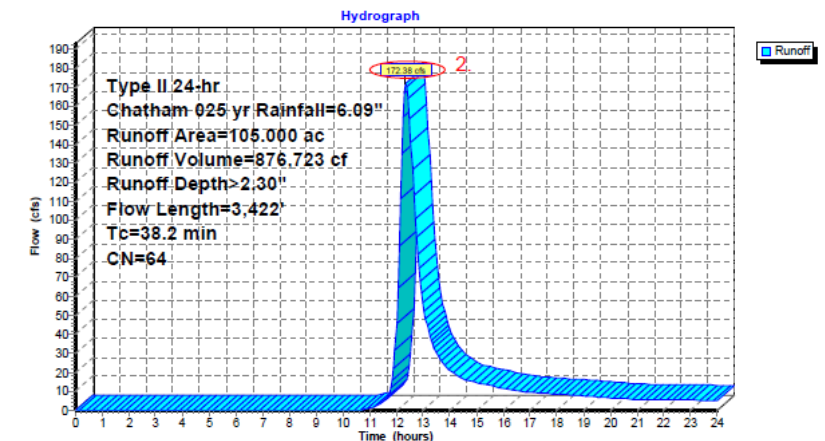
Runoff = 2. 172.38 cfs @ 12.36 hrs, Volume= 876,723 cf, Depth> 2.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type II 24-hr Chatham 025 yr Rainfall=6.09" 2.

Area (ac)	CN	Description
67.000	60	Woods, Fair, HSG B
2.000	98	Roofs, HSG B
36.000	69	Pasture/grassland/range, Fair, HSG B
1. 105.000	64	Weighted Average
103.000		98.10% Pervious Area
2.000		1.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.7	100	0.0100	0.06		Sheet Flow, Sheet Flow Woods: Light underbrush n= 0.400 P2= 3.60"
6.1	361	0.0388	0.98		Shallow Concentrated Flow, Shallow Concentrated Flow Woodland Kv= 5.0 fps
5.4	2,961	0.0209	9.22	92.18	Channel Flow, Channel Flow Area= 10.0 sf Perim= 9.0' r= 1.11' n= 0.025 Earth, clean & winding
38.2	3,422	Total			

Subcatchment D1: DA to Culvert 1





DESIGN STANDARDS

STORM DRAINAGE INFRASTRUCTURE - CULVERTS

Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Wednesday, Feb 16 2022

Culvert 1 - 25 Year

Invert Elev Dn (ft) = 442.50
Pipe Length (ft) = 23.70
Slope (%) = 2.53
Invert Elev Up (ft) = 443.10
Rise (in) = 48.0
Shape = Circular
Span (in) = 48.0
No. Barrels = 2
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Groove end w/headwall (C)
Coeff. K,M,c,Y,k = 0.0018, 2, 0.0292, 0.74, 0.2

Embankment

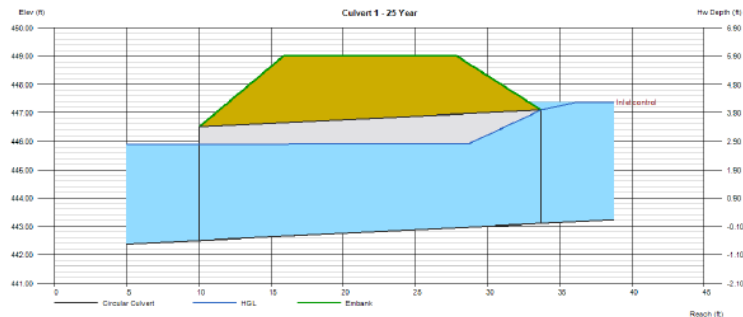
Top Elevation (ft) = 449.00
Top Width (ft) = 12.00
Crest Width (ft) = 20.00

Calculations

Qmin (cfs) = 172.38
Qmax (cfs) = 172.38
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 172.38
Qpipe (cfs) = 172.38
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 7.56
Veloc Up (ft/s) = 9.13
HGL Dn (ft) = 445.91
HGL Up (ft) = 445.91
Hw Elev (ft) = 447.38
Hw/D (ft) = 1.07
Flow Regime = Inlet Control



Summary for Subcatchment D1: DA to Culvert 1

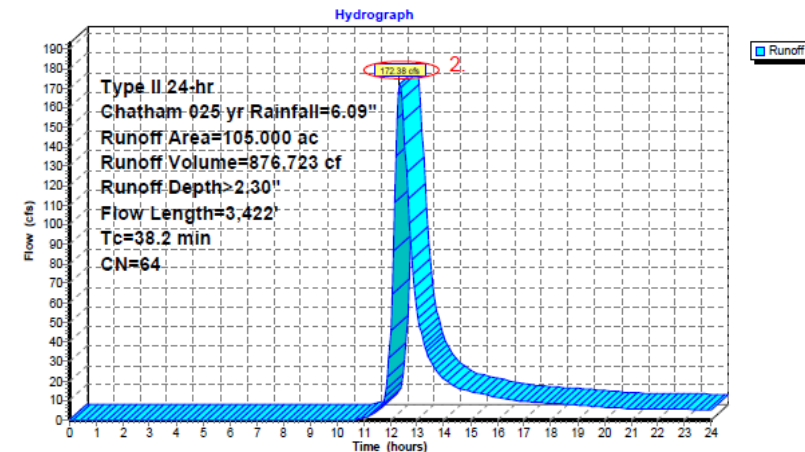
Runoff = 2 @ 172.38 cfs @ 12.36 hrs, Volume= 876,723 cf, Depth> 2.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type II 24-hr Chatham 025 yr Rainfall=6.09" 2.

Area (ac)	CN	Description
67.000	60	Woods, Fair, HSG B
2.000	98	Roofs, HSG B
36.000	69	Pasture/grassland/range, Fair, HSG B
1. 105.000	64	Weighted Average
103.000		98.10% Pervious Area
2.000		1.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.7	100	0.0100	0.06		Sheet Flow, Sheet Flow Woods: Light underbrush n= 0.400 P2= 3.60"
6.1	361	0.0388	0.98		Shallow Concentrated Flow, Shallow Concentrated Flow Woodland Kv= 5.0 fps
5.4	2,961	0.0209	9.22	92.18	Channel Flow, Channel Flow Area= 10.0 sf Perim= 9.0' r= 1.11' n= 0.025 Earth, clean & winding
38.2	3,422	Total			

Subcatchment D1: DA to Culvert 1





DESIGN STANDARDS

STORM DRAINAGE INFRASTRUCTURE – INLET REPORTS

Q25-YEAR Report

Stormwater Studio 2024 v 3.0.0.35

Project Name: STORM SYSTEM B

11-11-2024

Line No.	Inlet ID	Drain Area (ac)	Total Area (ac)	Runoff Coeff (C)	Total C x A	Inlet Time (min)	Tc System (min)	i Inlet (in/hr)	Incr Q (cfs)	Total Runoff (cfs)	Capac. Full (cfs)	Vel Ave (ft/s)	Line Size (in)	Line Length (ft)	Line Slope (%)	Invert Up (ft)	Invert Dn (ft)	Grnd/Rim Elev Up (ft)	
1	B1	4.580	6.930	0.60	4.16	5.0	5.3	7.95	21.85	32.60	46.66	6.64	30	197.00	1.29	290.55	288.00	295.60	
2	B2	2.350	2.350	0.60	1.41	5.0	5.0	7.95	11.21	11.21	16.41	3.57	24	104.48	0.53	291.60	291.05	296.05	
3	E1	0.900	1.640	0.60	0.98	5.0	5.3	7.95	4.29	7.72	16.28	4.44	18	52.00	2.40	272.75	271.50	276.80	
4	E2	0.740	0.740	0.60	0.44	5.0	5.0	7.95	3.53	3.53	7.50	2.21	18	72.20	0.51	273.32	272.95	277.05	
5	C1	2.980	5.870	0.60	3.52	5.0	5.2	7.95	14.22	27.73	71.69	6.00	30	36.00	3.06	270.60	269.50	275.40	
6	C2	2.890	2.890	0.60	1.73	5.0	5.0	7.95	13.79	13.79	16.41	4.45	24	76.00	0.53	271.50	271.10	275.40	
7	F1	9.010	9.010	0.35	3.15	5.0	5.0	7.95	25.08	25.08	49.73	6.14	30	204.00	1.47	279.00	276.00	286.65	
8	G1	1.200	5.570	0.60	3.34	5.0	5.5	7.95	5.73	26.01	31.15	8.53	24	58.00	1.90	275.10	274.00	283.45	
9	G2	2.940	3.620	0.60	2.17	5.0	5.3	7.95	14.03	17.04	14.89	9.64	18	102.00	2.01	277.75	275.70	283.45	
10	G3	0.680	0.680	0.60	0.41	5.0	5.0	7.95	3.24	3.24	4.58	2.81	15	72.00	0.50	282.86	282.50	284.94	
11	G4	0.750	0.750	0.60	0.45	5.0	5.0	7.95	3.58	3.58	6.41	2.11	18	72.00	0.37	281.72	281.45	284.51	
12	I1	3.360	18.960	0.60	8.47	5.0	5.5	7.95	16.03	65.97	136.02	17.22	30	50.00	11.00	273.00	267.50	277.34	
13	CB I2	0.130	14.630	0.60	5.87	5.0	5.3	7.95	0.62	46.03	41.53	9.38	30	78.00	1.03	274.30	273.50	279.57	
14	I3	5.320	14.500	0.45	5.80	5.0	5.3	7.95	19.04	45.49	65.95	9.27	30	29.00	2.59	275.25	274.50	280.50	
15	I4	5.940	5.940	0.30	1.78	5.0	5.0	7.95	14.17	14.17	22.91	4.51	24	38.00	1.03	276.14	275.75	280.01	
16	CB J1	0.040	2.380	0.60	1.43	5.0	5.1	7.95	0.19	11.29	48.34	8.25	24	81.00	4.57	271.20	267.50	278.84	
17	CB J2	0.040	2.340	0.60	1.40	5.0	5.1	7.95	0.19	11.13	23.08	5.99	24	24.00	1.04	271.65	271.40	278.84	
18	FES J3	2.300	2.300	0.60	1.38	5.0	5.0	7.95	10.97	10.97	15.62	7.67	18	38.43	2.21	273.00	272.15	277.93	
19	K1	2.340	5.870	0.60	3.52	5.0	5.3	7.95	11.16	27.66	66.43	12.60	24	51.00	8.63	277.40	273.00	282.10	
20	K2	3.530	3.530	0.60	2.12	5.0	5.0	7.95	16.84	16.84	17.30	5.36	24	102.58	0.58	278.20	277.60	282.10	
21	L1	1.420	4.680	0.60	2.81	5.0	5.2	7.95	6.77	22.10	68.32	11.69	24	54.79	9.13	277.50	272.50	281.05	
22	L2	3.260	3.260	0.60	1.96	5.0	5.0	7.95	15.55	15.55	22.73	5.91	24	104.00	1.01	278.75	277.70	282.05	
23	GI M2	0.840	0.840	0.60	0.50	5.0	5.0	7.95	4.01	4.01	10.58	2.71	18	49.29	1.01	277.00	276.50	280.73	
24	GI M1	0.790	0.790	0.60	0.47	5.0	5.0	7.95	3.77	3.77	17.50	3.23	18	56.89	2.78	276.08	274.50	281.02	

Notes: IDF File = Chatham County.IDF, Return Period = 25-yrs.

Project File: 2024-11-08 Phase 1A SD.sws

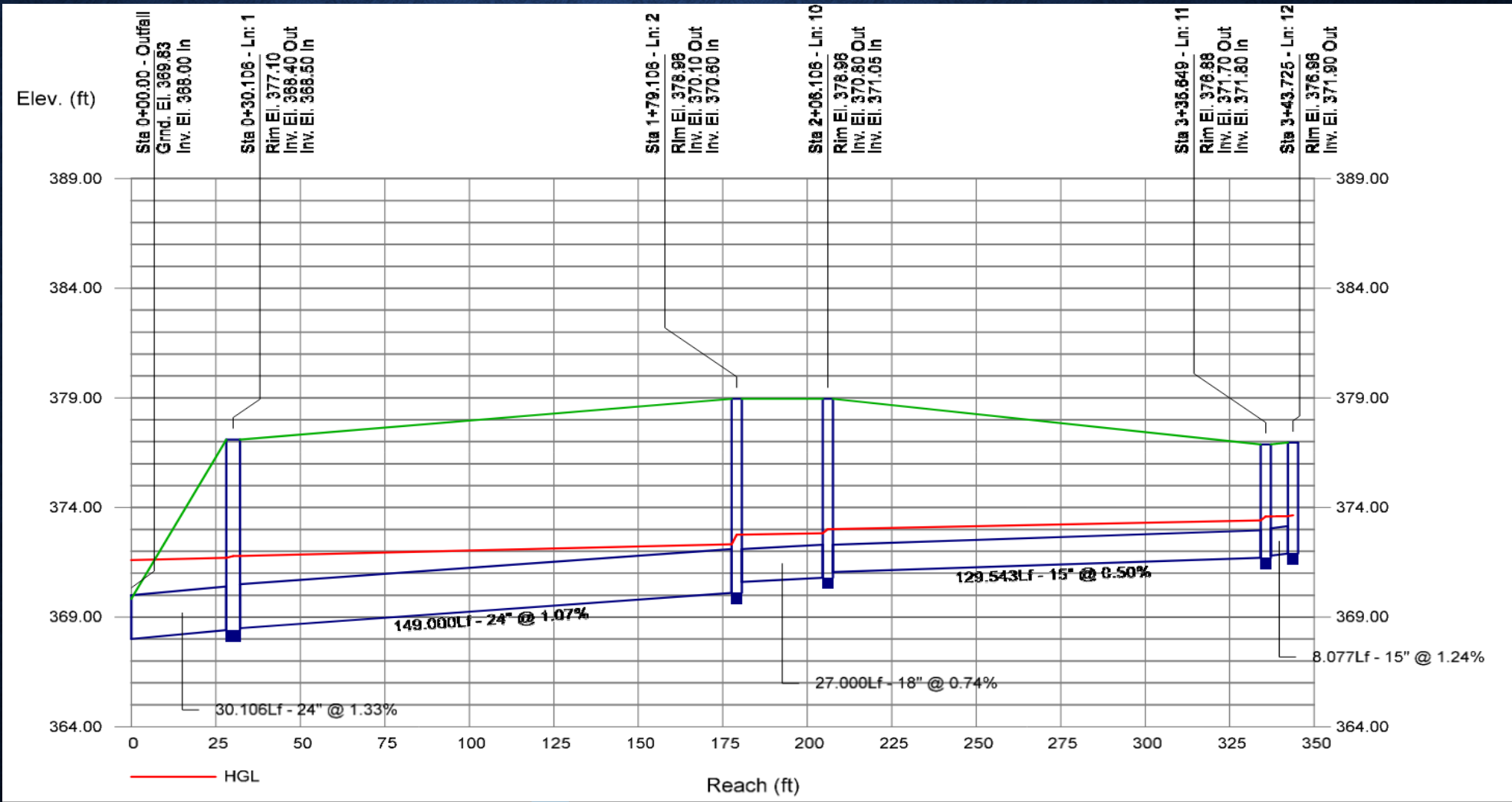
- Verify design
- Verify inputs
- Check for capacity



DESIGN STANDARDS

STORM DRAINAGE INFRASTRUCTURE – PROFILES

➤ Included for storm network

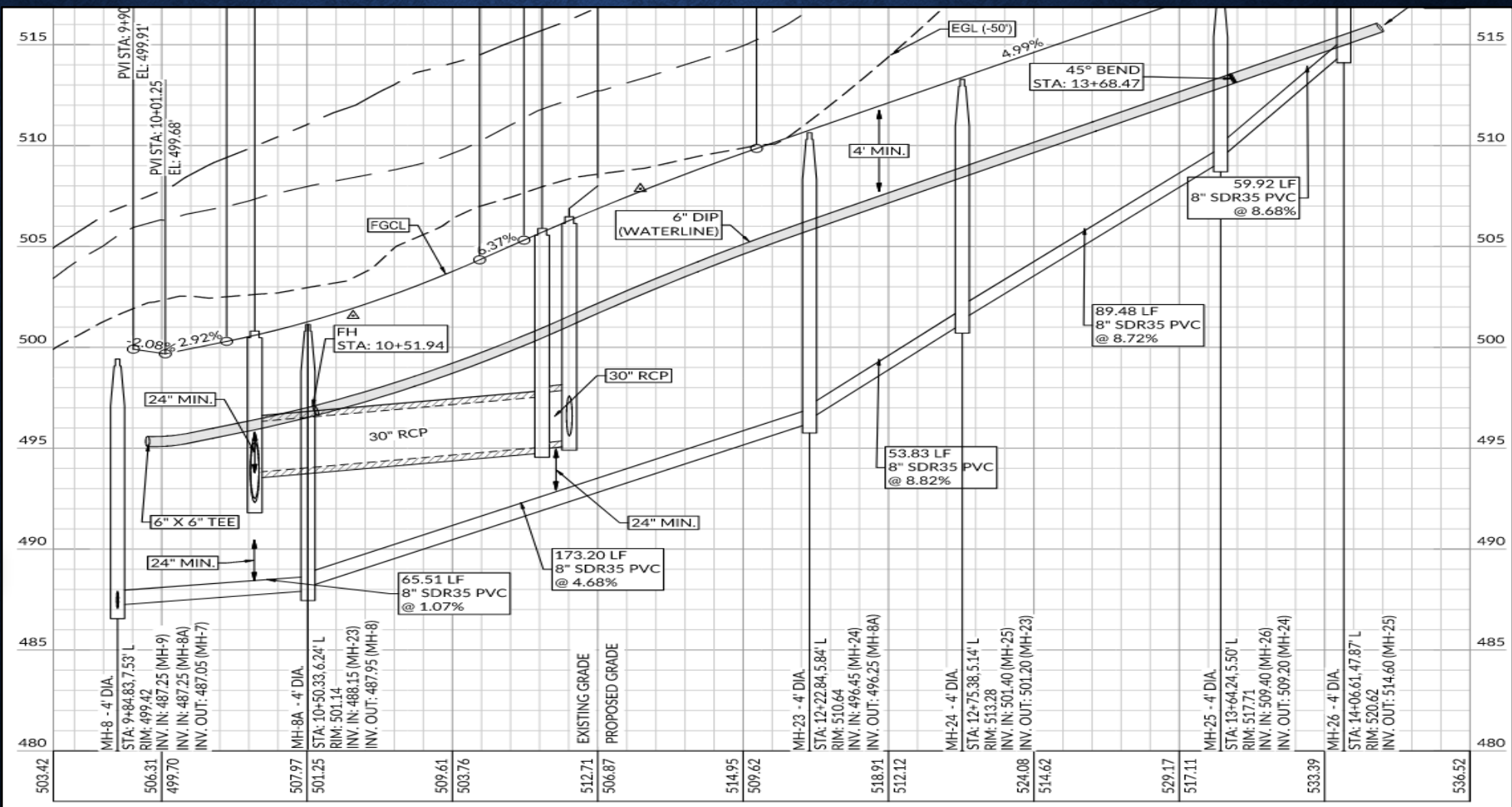




DESIGN STANDARDS

ROAD PROFILES

- Included for roads
- Verify minimum cover
- Verify minimum separation





MISCELLANEOUS

NCG01

Soil Stockpile Area

- 50 feet from surface waters, storm inlets, sediment basins, and ditches*
- 50 feet from site perimeter*
- Silt fence low sides offset 5 feet from toe of stockpile

Concrete Washout

- 50 feet from surface waters, storm inlets, sediment basins, and ditches*
- Protect nearby inlets
- Signage

Staging, Parking, Fueling

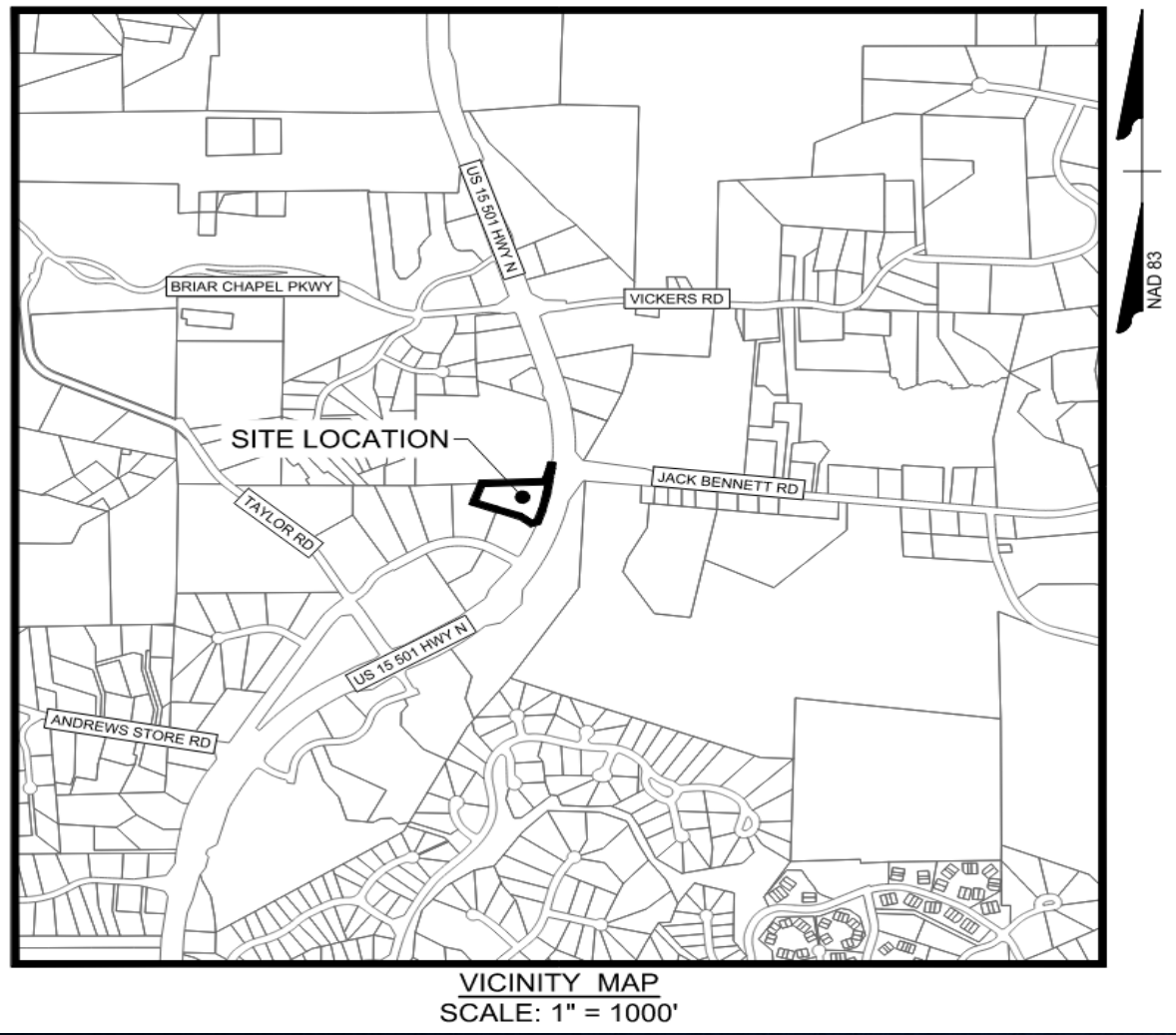
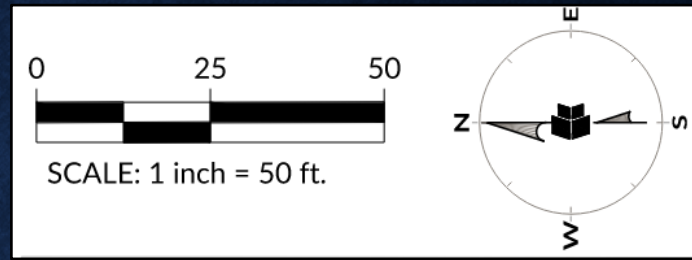
- 50 feet from surface waters, storm inlets, sediment basins, and ditches*
- Locate fueling areas

*Unless no other reasonable alternative exists. Denote on plan.



MISCELLANEOUS

BASIC INFORMATION

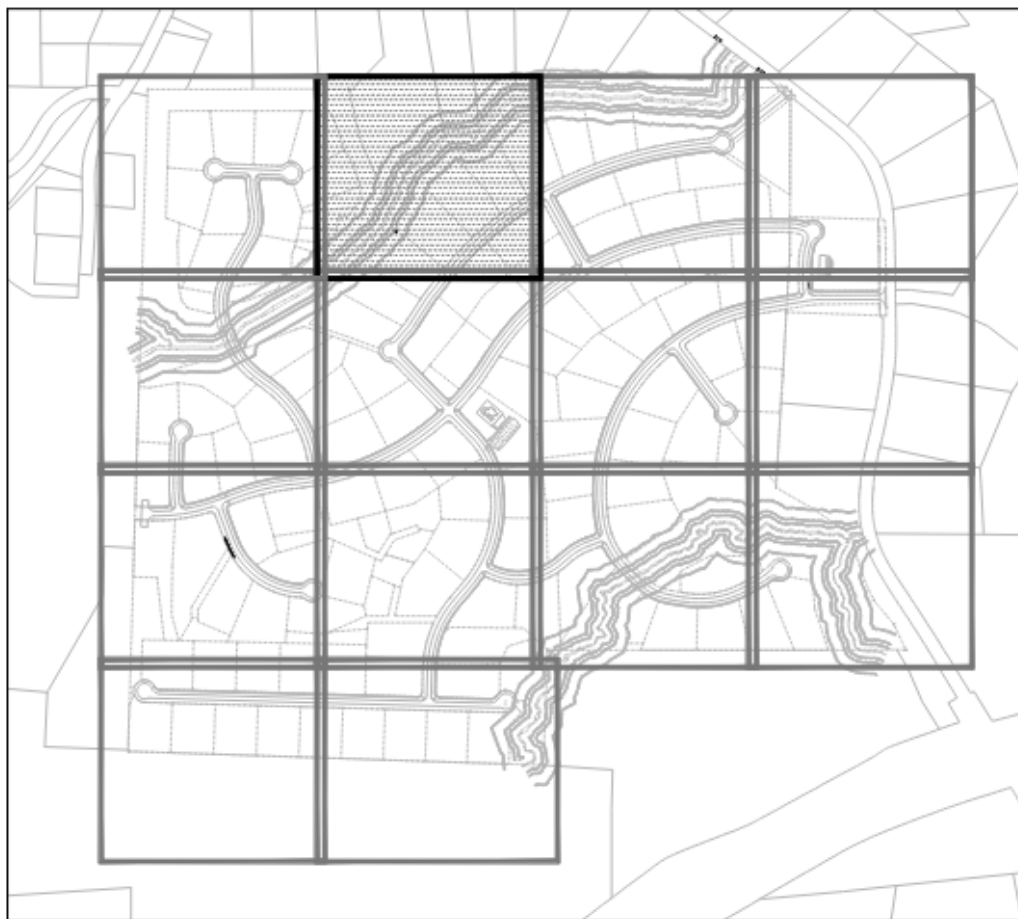


VICINITY MAP
SCALE: 1" = 1000'

SITE DATA TABLE	
PROJECT NAME:	CHATHAM COUNTY EMS
PROJECT ADDRESS:	9251 US 15 501 NORTH FEARRINGTON, NC 27312
TOTAL ACREAGE:	3.03 AC
DEED BOOK / PAGE:	BK: 460, PG: 0669
PIN NUMBER:	977503415782
ZONING:	R-1
USE:	VACANT/WATER TOWER
ONSITE IMPERVIOUS AREA EXISTING:	0.02± ACRES (1,003 SF)
ONSITE IMPERVIOUS AREA PROPOSED:	0.36 ± ACRES (15,568 SF)
TOTAL DISTURBED AREA:	1.56 ± ACRES (61,623 SF)
RIVER BASIN:	CAPE FEAR
STREAM:	HAW RIVER (JORDAN LAKE)
SURFACE WATER CLASSIFICATION:	WS-IV NSW

MISCELLANEOUS

SHEET INDEX



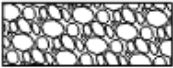











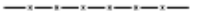










SHEET INDEX





















SCALE: 1"=750'

Sheet List Table

Sheet Number	Sheet Title
C0.0	COVER SHEET
C1.0	OVERALL EXISTING CONDITIONS & DEMOLITION PLAN
C1.1	EXISTING CONDITIONS & DEMOLITION PLAN
C2.0	EROSION CONTROL PLAN - PHASE 1
C2.1	EROSION CONTROL PLAN - PHASE 2
C2.2	EROSION CONTROL DETAILS
C2.3	EROSION CONTROL DETAILS
C2.4	EROSION CONTROL DETAILS
C2.5	NCG01 GROUND STABILIZATION AND MATERIALS HANDLING
C2.6	NCG01 SELF-INSPECTION, RECORDKEEPING AND REPORTING
C3.0	SITE PLAN
C3.1	ADJACENT INTERSECTION LOCATIONS
C3.2	OVERALL US15-501 CROSS SECTION MAP
C3.3	US15-501 CROSS SECTIONS
C3.4	US15-501 CROSS SECTIONS
C3.5	US15-501 CROSS SECTIONS
C3.6	INTERSECTION SIGHT DISTANCE PROFILE
C3.7	INTERSECTION SIGHT DISTANCE PROFILE
C4.0	GRADING AND STORM DRAINAGE PLAN
C4.1	BIORETENTION PLAN & PROFILE
C4.3	PRE-DEVELOPMENT DRAINAGE AREA MAP
C4.4	POST-DEVELOPMENT DRAINAGE AREA MAP
C5.0	UTILITY PLAN
C6.0	NOTES AND DETAILS
C6.1	NOTES AND DETAILS
C6.2	NOTES AND DETAILS
C6.3	NOTES & DETAILS
C6.4	NOTES AND DETAILS
C6.5	NOTES AND DETAILS
C6.6	NOTES AND DETAILS
L1.0	OVERALL LANDSCAPE PLAN
L1.1	LANDSCAPE DETAIL PLAN

MISCELLANEOUS LEGEND

EROSION CONTROL LEGEND	
	CONSTRUCTION ENTRANCE/EXIT
	SKIMMER SEDIMENT BASIN BOTTOM AREA
	RIP RAP FILTER DAM
	SILT FENCE OUTLET
	STRAW WATTLE
	TEMP. SEED/MULCH/PERM. SEED
	OUTLET PROTECTION
	SKIMMER DEVICE W/ OUTLET PIPE
	SLOPE DRAIN INLET PROTECTION
	TEMPORARY HDPE SLOPE DRAIN
	INLET PROTECTION
	DIVERSION DITCH
	SILT FENCE
	TREE PROTECTION FENCE
	COIR MESH SEDIMENT BAFFLES
	DISTURBANCE LIMITS
	DRAINAGE AREA LIMITS
	PROPOSED MINOR CONTOUR
	PROPOSED MAJOR CONTOUR
	DITCH LINER-ECX-1 TEMPORARY REINFORCEMENT MAT
	DITCH LINER-ECX-2 PERMANENT REINFORCEMENT MAT
	DITCH LINER-ECX-3 PERMANENT REINFORCEMENT MAT
	SLOPE MATTING-ECSC-3 COCONUT TURF REINFORCEMENT MAT

LEGEND	
	PROPERTY LINE (PL)
	RIGHT-OF-WAY LINE
	SETBACK LINE
	UTILITY EASEMENT
	EXISTING MAJOR CONTOUR
	EXISTING MINOR CONTOUR
	PROPOSED MAJOR CONTOUR
	PROPOSED MINOR CONTOUR
	PROPOSED TREE PROTECTION FENCE / LIMITS OF DISTURBANCE
	PROPOSED SILT FENCE
	PROPOSED SILT FENCE / LIMITS OF DISTURBANCE
	TEMPORARY DIVERSION DITCH
	CLEAN WATER DIVERSION DITCH
	TEMPORARY STONE CHECK DAM
	HORSESHOE INLET PROTECTION FILTER
	INLET PROTECTION
	CONCRETE WASHOUT
	SILT FENCE OUTLET
	CONSTRUCTION ENTRANCE
	ROLLED EROSION CONTROL PRODUCTS

- Contains all erosion control measures
- Between legend and callouts, everything accounted for
- All applicable sheets have own legends (grading, utility, etc.)

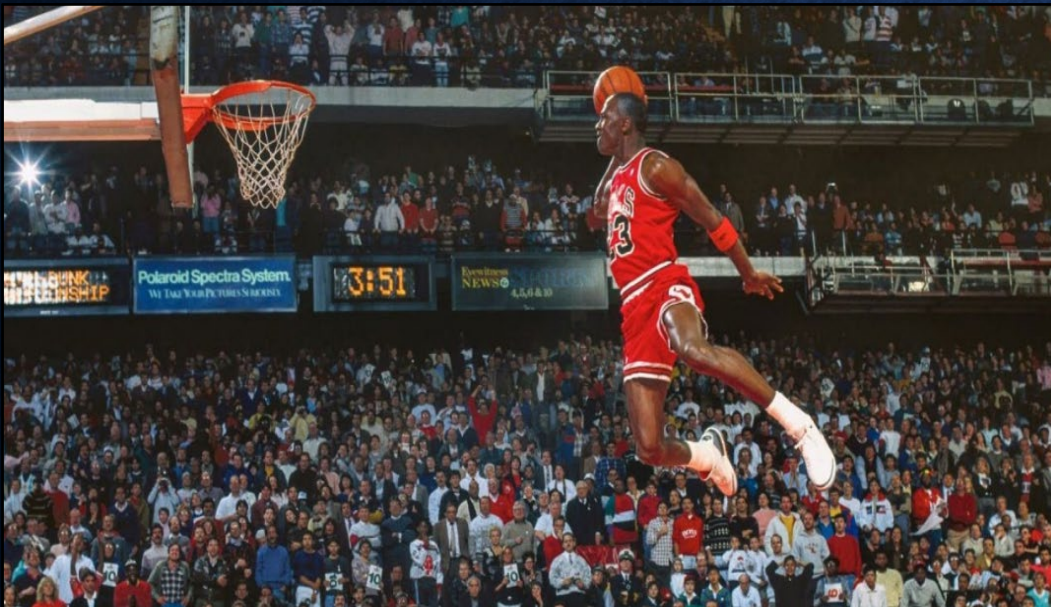
PLAN REVIEW

CONCLUSION

A good plan can be the difference between *environmental compliance* and *environmental damage*.

Make your plan work for you so your planned measures:

Perform like this:



And not like this:





THANK YOU FOR YOUR ATTENTION!

**Please remember to complete the
end of workshop evaluation:**

<https://bit.ly/2025EscEval>