

EROSION CONTROL PLAN REVIEW FOR LAND DEVELOPMENT

By: Justin Hasenfus, CPESC



WHY DO WE REVIEW PLANS?

Sedimentation Pollution Control Act:

Purpose: "...permit development of this State to continue with the least detrimental effects from pollution by sedimentation."





AGENDA

Part 1

- > Responsibilities
- > Requirements
- > Tools
- > Pointers

Part 2

- > Application
- Design Standards*
- > Miscellaneous



*Per the NCDEQ Erosion and Sediment Control Planning and Design Manual

PART 1



RESPONSIBILITIES

Compliance

- Plan must comply with applicable local, state, and federal laws and regulations
- Plan must not allow violation
- Plan must be legal

<u>Judgement</u>

- Best approximation with least erosion
- > Feasible
- > Practical
- > Economical

Consider

- > The design professional
- > The regulating agencies
- > The grading contractor
- Property owners
- > The general public



REQUIREMENTS: WHAT DOES THE SPCA REQUIRE?

§ 113A-51.

Preconstruction meeting (subject to staff availability)

§113A-56.(C)

NCDEQ has concurrent jurisdiction with local programs

§ 113A-57.

- Require erosion control
 measures to retain sediment
 within boundaries of tract(s)
 of land to be disturbed
- Graded slope requirements
- Require permanent ground cover sufficient to restrain erosion





REQUIREMENTS: WHAT DOES THE SPCA REQUIRE?

§ 113A-60.

- Local government ordinance shall at least meet and may exceed the minimum requirements of SPCA
- Provisions for residential plans
- No erosion control measures required at high sides of project





REQUIREMENTS: WHAT DOES THE SPCA REQUIRE?

<u>§ 113A-61</u>.

- ➤ Review timeframe (15,30 day) →
- Approve, disapprove, or approve with modifications
- Grant conditional approval for environmental and other development approvals & permits
- Disapproval for buffer authorizations allowed
- Disapproval based on compliance record allowed
- Person submitting plan entitled to public hearing to appeal

Review Timeframe Guidance

PLAN REVIEWS

- Plans are to be reviewed and a decision rendered to the applicant within 30 calendar days of receipt of a complete plan. Unlike with other permits, there is no pausing this "review clock". A complete plan is defined as follows:
 - a. An erosion and sediment control plan
 - b. Calculations in support of the design, if applicable
 - c. An authorized statement of financial responsibility
 - A letter of consent or permission letter from the landowner for the landdisturbance, if the landowner and the Financially Responsible Person/Party are not the same entity
 - e. Documentation of property ownership (e.g., deed)
 - f. Fees paid in full
- Plans are to be reviewed and a decision rendered to the applicant within 15 calendar days of receipt of a revised plan. There is no pausing this "review clock" once it has started.

Reference: G.S. 113A-61(b)



- > Local Ordinances
- Sedimentation Pollution Control Act
- NCDEQ Erosion & Sediment Control
 Planning and Design Manual
- Recorded Plats & Legal Instruments
- Reviewing Agencies
- Design Firm







WATERSHED PROTECTION DEPARTMENT P.O. Box 548

Pittsboro, NC 27312

Phone: (919) 545-8344 ● E-mail: justin.hasenfus@chathamcountync.gov ● Website: chathamcountync.gov

Soil Erosion and Sedimentation Control Checklist for Plan Submittals

The following information should be provided for all erosion control plan submittals. Mark NA for any items that are not applicable.

General Information

- Name of Project
- Name, address, and phone number of owner
- Registered Engineer's/Landscaper's/Land Surveyor's Seal
- Registered Engineer's/Landscaper's/Land Surveyor's name, address, phone
- Permit Application & Financial Ownership/Responsibility Form
- Deed(s) & easement documentation for all property to have disturbance
- Zoning of property
- Total acres of all property to have disturbance
- Disturbed acres
- Land-disturbing Permit fee
- ___ Plan review fee

Plan Features

- ____ Vicinity Map (1"=2,000', Subdivision phase for residential plans)
- North arrow and scale
- Legend
- Property lines & adjoining property owners
- Lot lines and lot/building numbers
- Project narrative describing the nature & purpose of activity
- ___ Existing and Proposed Contours
- Limits of disturbance (include all proposed soil disturbance and clearing; include utilities)
- Proposed disturbed areas shown on NRCS Soils map and USGS Topography map
- Existing roads, buildings, utilities, etc.
- Proposed roads, buildings, utilities, etc.
- Phased erosion control plans
- Construction sequence (site overall)
- Construction sequence (stream crossing)
- Retaining Wall: (include grading and drainage, profiles, , geogrid, location and wall number)
- Rock outcrops, seeps, springs
- Wetlands
- 100-year floodplain limits and elevations
- Easements Streams, lakes, ponds, drainage ways, etc.
- Riparian buffers Borrow/Waste Areas
- Parking/staging/material storage areas
- ___ Soil stockpile locations¹
- 1. Soil stockpile locations are required to be 50' from storm drain inlets, sediment basins, site perimeter sediment controls and surface waters. Silt fence is required on low sides.

Erosion and Sediment Control Measures

- ___ Standard details for all measures
- ___ Construction entrance(s)
- Silt fence (1/4 acre of drainage per 100 linear feet of silt fence, and not in concentrated flow)
- ___ Silt fence rock outlets
- ___ Temporary Diversion Ditches
- Check Dams
- Slope drains with inlet & outlet protection (primarily where slope drains enter basin)
- ___ Inlet Protection
- ___ Clean water conveyance(s) (where applicable)
- ___ Concrete Washouts²
- Seed and sod specifications chart(s)
- Skimmer basins (dimensions, volume, and surface area)
- ____ Size, type, and invert elevations for all storm sewer
- ____ Rock bench for skimmers
- ___ Ditch profiles with lining requirements & supporting calculations
- Permanent liners in channels/ditch lines
- ___ Temporary liners in diversion ditches
- Energy Dissipaters (sizes, dimensions, lining requirements & supporting calculations)
- ___ Label slopes greater than 3:1 and label required stabilization liner (e.g. SC150)
- ___ Temporary stream crossing details and sequence
- ___ Other proposed measures & necessary calculations

Other Requirements

- ___ Design calculations for culverts, storm sewers, and channels
- ___ Design calculations for sediment basins/traps
- ___ Fill in drainage easement (channel evaluation/design)
- ___ Stormwater plan (greater than 20,000 ft² disturbed), if applicable
- ____ Floodplain development permit, if applicable
- Permit and flood study required (work in floodway), if applicable
- Army Corp. of Engineers Section 404 Permit, if applicable
- ___NCDEQ 401 Certification, if applicable
- ___ Chatham County/Pittsboro Planning
- (Buffer authorizations, construction plan approval, etc.), if applicable
- NCDOT permits and approvals, if applicable
- NCG01 Ground Stabilization and Materials Handling Plan Sheet from NCDEQ webpage (projects subject to NPDES)
- ____NCG01 Inspection, Recordkeeping and Reporting Plan Sheet from NCDEQ webpage
- (projects subject to NPDES)

NCDEQ's webpage with NCG01 updates:

https://deq.nc.gov/NCG01

Chatham County Erosion Control Standard Details:

https://www.chathamcountync.gov/government/departments-programs-i-z/watershedprotection/erosion-control/standard-details

Chatham County GIS:

https://chathamncgis.maps.arcgis.com/apps/webappviewer/index.html?id=65367d5f69774726828390a

2. Concrete washouts are required to be 50' from storm drain inlets and surface waters.



NCG01 Permit Checklist (pages 7-9)

Permit No. NCG010000

PART II - STORMWATER POLLUTION PREVENTION PLAN

The Stormwater Pollution Prevention Plan (SWPPP) for this permit shall include the approved Erosion and Sedimentation Control (E&SC) Plan as well as any requirements in this Part that exceed the approved E&SC Plan. Items that are required in the SWPPP but are not part of the approved E&SC Plan may include, at a minimum, Section E, Item (1) (Ground Stabilization Timelines), Section F [Materials Handling]. DEMLR provides two sample plan sheets that permittees may add to their E&SC Plan set to fulfill Sections E (1) and F of this permit at https://deo.or.gov/NCG01. [NCCG 1134—57, 15A NCAC 046.0107]

Recommendations for preparing the E&SC plan as well as for designing, constructing, and maintaining the erosion and sedimentation control practices are contained in the North Carolina Erosion and Sediment Control Planning and Design Manual.

SECTION A: REQUIRED COMPONENTS OF THE EROSION AND SEDIMENTATION CONTROL

The E&SC Erosion and Sedimentation Control Plan shall include, at a minimum, the following components and those components shall be in compliance with all conditions of this permit. Hard and/or digital copies shall be submitted in accordance with the specifications of the E&SC plan authority. [15A NCAC 04b.0]077

1. Location Information

- Project location & labeled vicinity map (roads, streets, landmarks)
- _____ North arrow and scale
- Identification of the River Basin
- A copy of site disturbed area located on applicable USGS quadrangle and hardbound copy of
- the NRCS Soils maps to scale

 Latitude and longitude (in decimal degrees) at the project entrance
- [NCGS 113A-57, 15A NCAC 04b .0107]

2. Narrative and Construction Sequence

- Narrative describing the nature & purpose of the construction activity
- Construction sequence related to erosion and sediment control (including installation of critical measures prior to the initiation of the land-disturbing activity & removal of measures after areas they serve are permanently stabilized). Address all phases of construction and necessary practices associated with temporary stream bypasses and/or crossings.
- __ Estimated start and end dates

[NCGS 113A-57, 15A NCAC 04b .0107]

3. General Site Features

- ____ Property lines
- Existing and proposed contours (topographic lines)
- Stockpiled topsoil or subsoil locations
- Limits of disturbed area (with acreage labeled) within which all construction, material storage, grading, and related activities occur, including the following items as applicable:

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Permit No. NO

- Access to E&SC measures, lots that will be disturbed, and utilities that may extend
 offsite.
- Temporary access and haul roads, other than public roads, constructed or used in connection with the land-disturbing activity
- Berrow and waste areas created by the applicant. If the land-disturbing activity and any related borrow or waste activity are not conducted by the same person, they shall be considered separate land-disturbing activities
- Offsite borrow pits if the borrow pit is a construction support activity to the development
- Planned and existing building locations and elevations, if applicable
 Planned & existing road locations & elevations, including temporary access roads, if
- applicable
- Profiles of streets, utilities, and permanent ditch lines, if applicable
 - Lot lines and/or building numbers, if applicable
 Easements and drainageways, particularly required for offsite affected areas, if applicable
- Location and details associated with any onsite stone crushing or other processing of material excavated, if applicable. A mining permit will be required if the affected area associated with excavation, processing, stockpiles and transport of such materials comprises one or more acres, and materials will be leaving the development tract

[NCGS 113A-57, 15A NCAC 04b .0107]

4. Site Drainage Features

- Existing and planned drainage patterns (include off-site areas that drain through project and address temporary and permanent conveyance of stormwater over graded slopes)
- Surface waters, including the limits of wetlands, streams, lakes and ponds and all required local or state buffer zones as well as impact maps by the construction activity to these
- Method used to determine acreage of land being disturbed and drainage areas to all proposed E&SC measures (e.g. delineation map)
- Size, pipe material and location of culverts and sewers
- Soil information throughout the site and below culvert storm outlets, including soil type and special characteristics
- Name and classification of receiving water course where discharges are to occur

[NCGS 113A-57, 15A NCAC 04b .0107]

5. Plans Showing E&SC Measures

- Legend (provide appropriate symbols for all measures and reference them to the construction details)
- _____ Location of temporary and permanent E&SC measures
- Location of permanent stormwater quality and quantity control measures
- Construction drawings and details for temporary and permanent measures, including outlet structures. Show measures to scale on plan and include proposed contours where necessary. Ensure design storage requirements are maintained through all phases of construction.
- _____ Specifications for ground stabilization
- Maintenance requirements for measures
- Contact person responsible for maintenance, if the permittee wishes to designate one. If not, the financially responsible organization will be the contact for maintenance.

Permit No. NCG010000

 A note stating that material handling procedures for the items required in Part II, Section F will be followed.

Permit No. NCG010000

Standard details for structural BMPs to be installed to manage the anticipated materials listed in Part II, Section P such as construction debris management, concrete washout, paint washout, petroleum product storage and pesticide/herbicide handling, along with spill prevention practices.

[NCGS 113A-57, 15A NCAC 04b .0107]

6. Calculations

- Calculations for peak discharges of runoff from each outlet at pre-development, during construction and at completion. Provide all supporting data for the computation methods used (rainfall data for required storm events, time of concentration/storm duration, and runoff coefficients).
- Design calculations for culverts and storm sewers (include headwater, tailwater and outlet velocities)
- Discharge and velocity calculations for open channel and ditch flows (easement & rights-ofway)
- Design calculations for cross sections and method of stabilization for existing and planned channels (include temporary linings). Include appropriate permissible velocity and shear stress.
- Design calculations and construction details for energy dissipaters below culvert and storm sewer outlets (include stone/material specs & apron dimensions). Avoid discharges on fill slopes.
- Design calculations and dimensions for sediment traps and basins
- Total and disturbed drainage areas for silt fencing and other sediment controls

[NCGS 113A-57, 15A NCAC 04b .0107]

7. Vegetative Stabilization Shown on Plans

- Area & acreage to be stabilized with vegetation
- Method of soil preparation
- Seed type & rates (temporary & permanent)
- ____ Fertilizer type and rates
- ____ Fertilizer type and rates
 Mulch type and rates (include mulch anchoring methods)

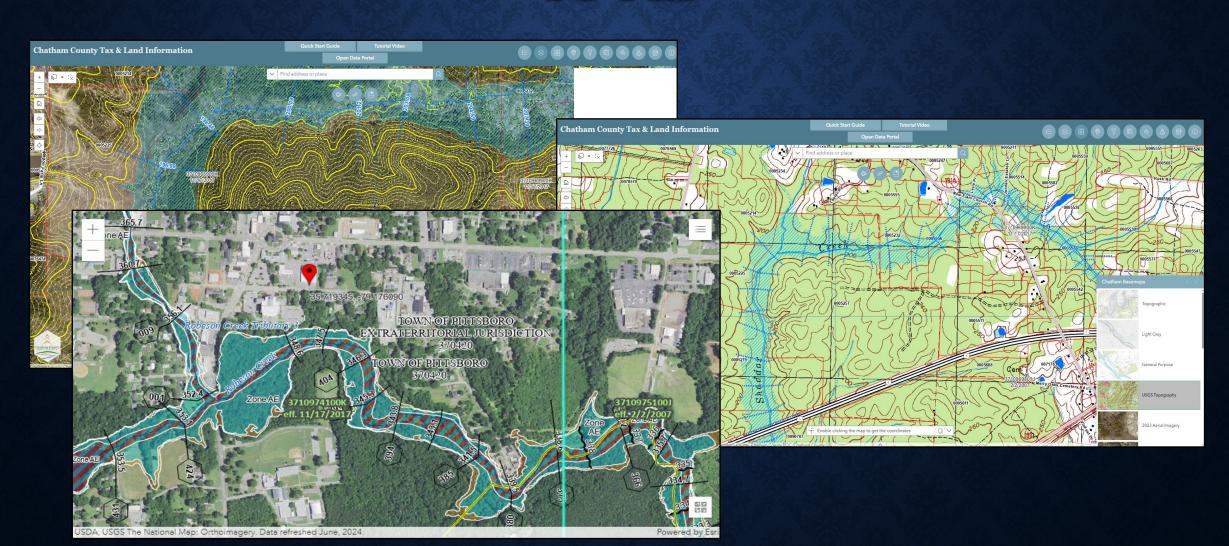
[NCGS 113A-57, 15A NCAC 04b .0107]

8. Documentation

- Properly executed Financial Responsibility/Ownership Form
- Certificate of assumed name, if the owner is a partnership
- Name of Registered Agent (if applicable)
- Copy of the most current Deed for the site. Please make sure the deed(s) and ownership information are consistent between the plan sheets, local records and this form.
- Provide latitude & longitude (in decimal degrees) at the project entrance.
- DWR Buffer Authorization, if required for project
- Copies of any recorded easements and/or agreements with adjoining property owners for landlocked parcels

[NCGS 113A-57, 15A NCAC 04b .0107]







DO:

- ➤ Have hard copy/electronic files
- > Allocate time appropriately
- > Say what you mean and explain why
- > Ask for more information
- Provide guidance where necessary
- > Pick your battles
- Cite comments/Provide comment response
- Consider new measures or approaches





DON'T:

- > Make large assumptions
- Skip reading ordinances or submittal requirements
- ➤ Forget to account for changes since last submittal
- > Ever use measures in surface waters other than those that are allowed
- > Get mad





IN GENERAL:

- Find a process that works
- > Catch the big stuff
- Focus your time on what's most important
- > Your goal is to produce a plan that is compliant and sufficient





l) Locate Project • GIS • Plat

2)Understand Scope

- Plan
- Application
- Narrative

- Clearly defined
- All work included
 - Required approvals

4) Dig In

- Choose a measure to start with
- Pick it apart
- Wash, rinse, repeat

- 5) Mental Site Walk
- Do an inspection in your head

6)Checklist

3) Verify Limits

> Verify the submittal meets the requirement

PART 2



APPLICATION

Check It

- All information present & correct
- NC Agent complete when required
- ➤ NC Secretary of State
- Authorized statement of financially responsibility & ownership
- Owner/FRP agreement

	turbing Permit Application
Apply Onlin	e: https://chathamcountync.viewpointcloud.com/categories/1082
PROJECT NAME:	DATE:
ADDRESS OF PROJECT:	
LATITUDE/LONGITUDE OF PROPERTY	AT SITE ENTRANCE:
PARCEL #(S):	
TOTAL DISTURBED ACRES or SQUARE F	EET:
PURPOSE OF ACTIVITY:	
FEE AMOUNT SUBMITTED:	RIVER BASIN: CAPE FEAR
ANTICIPATED START DATE:	ANTICIPATED END DATE:
Please provide a complete list of partners, in landowner is a group of individuals, corpora Name:	nanaging members and registered agents if the responsible entity or te organization or entity. Phone:
Address:	
E-Mail:	Signature:
*FINANCIALLY RESPONSIBLE PARTY	(applicable only if different from property owner)
Name:	Phone:
	Phone:
Name: Address: E-Mail: NORTH CAROLINA AGENT (applicable o	Phone:
Name: Address: E-Mail:	Phone:
Name: Address: E-Mail: NORTH CAROLINA AGENT (applicable of North Carolina)	Phone: Signature: Signature: only if owner or financially responsible party does not reside in
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Name Address: E-Mail: NORTH CAROLINA AGENT (applicable of North Carolina) Name Address: E-Mail: ENGINEER/SURVEYOR	Signature: Signature: Phone: Phone: EROSION CONTROL Person to contact should erosion & sediment control issues arise during land-disturbing activity:
Name: Address: E-Mail: NORTH CAROLINA AGENT (applicable of North Carolina) Name: Address: E-Mail: ENGINEER/SURVEYOR Company Name: Address:	Phone: Signature: Phone: Phone: EROSION CONTROL Person to contract should erosion & sediment control issues arise during land-disturbing activity: Contact Person:
Name Address: E-Mail: NORTH CAROLINA AGENT (applicable of North Carolina) Name Address: E-Mail: ENGINEER/SURVEYOR Company Name: Address: Contact Person:	Phone: Signature: Phone: Phone: EROSION CONTROL Person to contact should erosion & sediment control issues arise during land-disturbing activity: Contact Person: Company Name:
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Soil Erosion and Sedimentation Control Financial Responsibility/Ownership Form

PLEASE READ THE FOLLOWING INFORMATION:

- 1) This section must be signed in the presence of a Notary
- 2) This form must be signed by the property owner if an individual. If owned by a company or corporation, this form must be signed by an officer, director, partner, attorney-in-fact, or other person with authority to execute instruments for the corporation and accompanied
- by a complete list of all partners, managing members and registered agents of the company or corporation.

 3) This form must also be signed by the financially responsible party. Same provisions of #2 above apply.
- 4) If the landowner and financially responsible party are different, the completion and signing of this page shall serve as documentation acknowledging the landowner consents to and authorizes the financially responsible party to undertake the proposed land-disturbing activity on the landowner's marc(s) of land identified on this application.
- 5) By signing this form, I agree to receive erosion control plan review letters electronically at the email address provided on the application.
- 6) The information provided on this form is true and correct to the best of my knowledge and belief and was provided by me while under oath.
- 7) All Land-Disturbing permits are valid for up to (2) years from the date of issuance. If circumstances warrant, the permit may be estuded for (2) years per the conditions of the Cashama County Soil Brotion and Sedimentation Count Ordinance. Upon written notice, the Land-Disturbing permit may be revoked for failure to comply with the Ordinance. If the permit is revoked, all other permits and approvals are withheld until the property is once again in compliance with Chatham County regulations. Also, upon written notice, a civil peanly (fine) can be insignated against the property owner and/or additional financially responsible party (if my) for violations of the Chatham County Soil Evision and Sedimentation Countol Ordinance. This penalty is up to \$15000.00 per violation per day and is assessed duity for every day the property is in violation. Interfering with or humpering an impaction can result in a civil penalty without

OWNER OF PROPERTY:

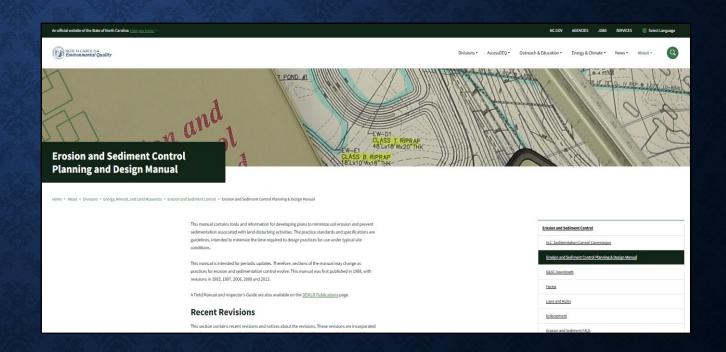
ompany (if applie	able):		
ignature:			
ADDITIONAL F	INANCIALLY RESPONSIBLE PARTY (if an	ıy):	
lame and Title:			
ompany:			
ompany:			
********	***		
	, a Notary Public of		County in the state
f	, a Notary Public of		County in the state personally
fppeared before me	, a Notary Public of do hereby certify that		County in the state personally
f ppeared before me bove form was exe	, a Notary Public of do hereby certify that this day and under oath acknowledged reading the infected by him or her.		County in the state personally
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Sources

- > Local Ordinance
- > NCDEQ Manual
- > Other supported documents and practices of sound engineering





BASINS

Sediment Basin (Riser Basin)

Design Criteria Summary: Temporary Sediment Basin:

> Primary Spillway: Riser/Barrel Pipe

Maximum Drainage Area: 100 acres

Minimum Sediment Storage Volume: 1800 cubic feet per acre of disturbed area 435 square feet per cfs of Q10 peak inflow Minimum Surface Area:

Minimum L/W Ratio: 2:1 Maximum L/W Ratio: 6:1 Minimum Depth: 2 feet

Skimmer(s) attached at bottom of riser Dewatering Mechanism:

pipe or flashboard riser

Minimum Dewatering Time: 48 hours 3 baffles* Baffles Required:

("Note: Basins less than 20 feet in length may use 2 baffles.)



Sediment Skimmer Basin

Design Criteria Summary:

Skimmer Sediment Basin

Primary Spillway: Trapezoidal spillway with impermeable

membrane

Maximum Drainage Area: 10 acres

Minimum Volume: 1800 cubic feet per acre of disturbed area Minimum Surface Area: 325 square feet per cfs of Q, peak inflow

Minimum L/W Ratio: 2:1 Maximum L/W Ratio: 6:1 Minimum Depth: 2 feet Dewatering Mechanism: Skimmer Minimum Dewatering Time: 2 days 3 baffles* Baffles Required:

(*Note: Basins less than 20 feet in length may use 2 baffles.)

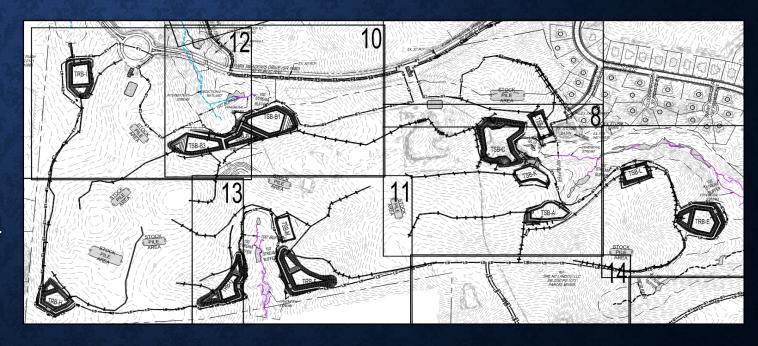




BASINS

Location

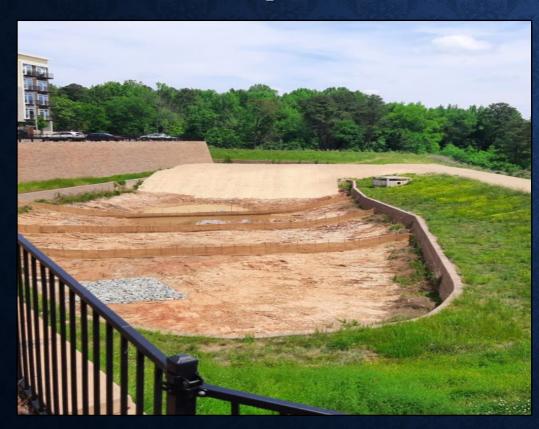
- > Locate in low areas
- > Capture majority of disturbed area
- ➤ Interfere minimally w/construction
- > Allows access for sediment removal
- > Never locate in surface waters



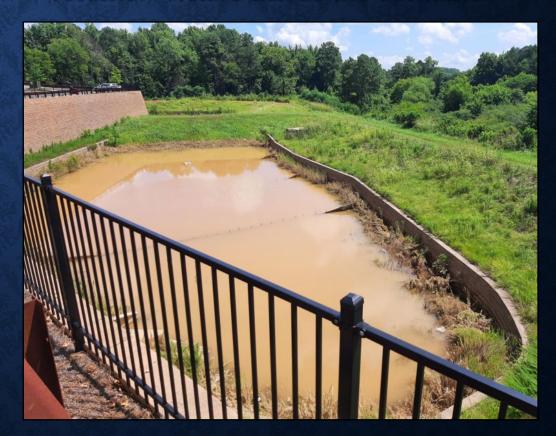


BASINS

Surface Area: "Shape of the container"



> Volume: "What's inside the container"





BASINS

Surface Area Required = $Q \times 435$ *

O = CIA

Q = peak rate of runoff (cfs)

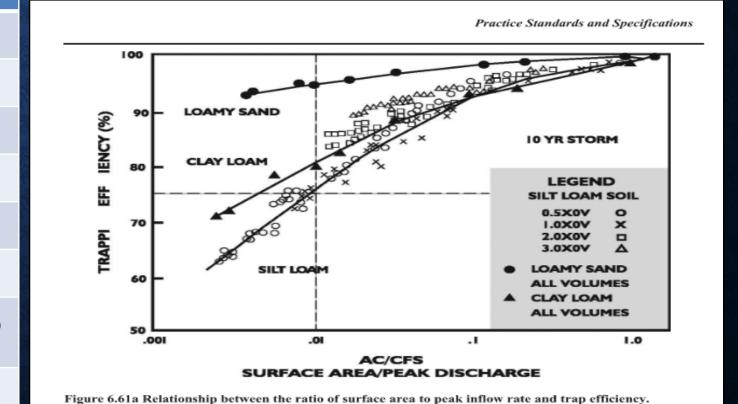
C = runoff coefficient

I = avg. intensity of rainfall (in/hr)

A = drainage area (acres)

*Skimmer Basin: Minimum 325 sq.ft./cfs of Q_{10} peak inflow

*Riser Basin = Minimum 435 sq.ft./cfs of Q_{10} peak inflow

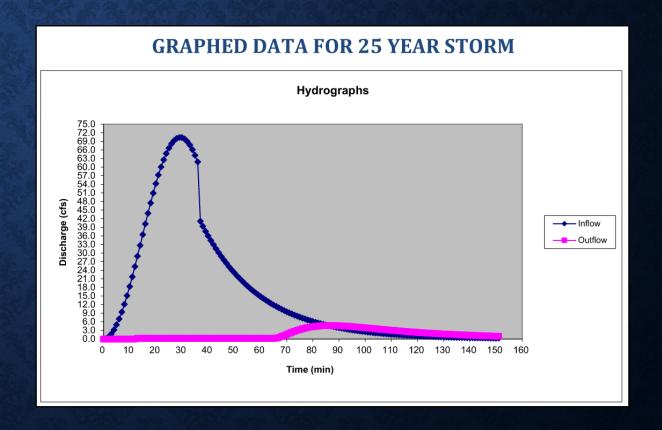




BASINS

Peak rate of runoff (Q)

- ➤ Maximum rate of water flow passing a specific point for the design storm
- > Calculated using:
 - Rational method or
 - NRCS method
- ➤ Rational method is Q = CIA and primarily used for drainage area < 50 acres
- > NRCS more accurate for areas > 20 acres





BASINS

Runoff coefficient (C)

- Relationship between rainfall rate and runoff rate
- Determined by soil type/land cover in the drainage area
- > Table of values in DEQ manual or ordinance
- Land use and soil cover homogenous in drainage area = use C value from table
- Land use and soil cover NOT homogenous, a weighted C value is calculated



Table 8.03b Value of Runoff Coefficient (C) for Rational Formula

Land Use	C	Land Use	С
Business:		Lawns:	
Downtown areas	0.70-0.95	Sandy soil, flat, 2%	0.05-0.10
Neighborhood areas	0.50-0.70	Sandy soil, ave., 2-7%	0.10-0.15
Residential:		Sandy soil, steep.	0.15-0.20
Single-family areas	0.30-0.50	7%	0.10 0.20
Multi units, detached	0.40-0.60	Heavy soil, flat, 2%	0.13-0.17
Multi units, Attached	0.60-0.75	Heavy soil, ave	0.18-0.22
Suburban	0.25-0.40	2-7%	0.10 0.22
Industrial:		Heavy soil, steep, 7%	0.25-0.35
Light areas	0.50-0.80	1.30	
Heavy areas	0.60-0.90	Agricultural land:	
Parks, cemeteries	0.10-0.25	Bare packed soil	
Parks, cemeteries	0.10-0.25	Smooth	0.30-0.60
Playgrounds	0.20-0.35	Rough	0.20-0.50
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		Cultivated rows	
Railroad yard areas	0.20-0.40	Heavy soil no crop	0.30-0.60
	0.10-0.30	Heavy soil with	
Unimproved areas	0.10-0.30	crop	0.20-0.50
Streets:		Sandy soil no crop	0.20-0.40
Asphalt	0.70-0.95	Sandy soil with	0.10-0.25
Concrete	0.80-0.95	crop Pasture	0.10-0.25
Brick	0.70-0.85	I martine u	0.15-0.45
		Heavy soil	0.15-0.45
Drives and walks	0.75-0.85	Sandy soil Woodlands	0.05-0.25
Roofs	0.75-0.85	woodiands	0.05-0.25
Roots	0.75-0.85		

NOTE: The designer must use judgement to select the appropriate C value within the range for the appropriate land use. Generally, larger areas with permeable soils, flat slopes, and dense vegetation should have lowest C values. Smaller areas with slowly permeable soils, steep slopes, and sparse vegetation should be assigned highest C values.

Source: American Society of Civil Engineers

8.03.6 Rev. 6/86



BASINS

Rainfall Intensity (I)

- Minimum design storm = 10 year
- > Average intensity (in/hr) for storm duration equal to time of concentration
- ➤ Time of concentration calculation should be provided
- > Data source included with submittal



NOAA Atlas 14, Volume 2, Version 3 Location name: Chapel Hill, North Carolina, USA* Latitude: 35.7617°, Longitude: -79.0596° Elevation: 365 ft**

* source: ESRI Maps
** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

PDS-b	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) ¹												
D	Average recurrence interval (years)												
Duration	1	2	5	10	25	50	100	200	500	1000			
5-min	4.98 (4.56-5.45)	5.86 6.76 (5.36-6.42) (6.18-7.39)		7.45 8.20 (6.80-8.15) (7.45-8.94)		8.72 (7.90-9.52)	9.19 (8.28-10.0)	9.60 (8.60-10.5)	10.0 10.4 (8.92-11.0) (9.14-11.4)				
10-min	3.98 (3.64-4.35)	4.69 (4.29-5.13)	5.41 (4.95-5.92)	5.96 (5.45-6.51)	6.53 (5.94-7.13)	6.95 (6.29-7.58)			7.94 (7.05-8.68)	8.18 (7.21-8.95)			
15-min	3.31 (3.03-3.62)	3.92 (3.60-4.30)	4.56 (4.17-4.99)	5.02 (4.59-5.49)	5.52 (5.02-6.02)	5.86 (5.31-6.40)	6.16 6.40 (5.54-6.72) (5.73-6.99)		6.67 (5.92-7.28)	6.85 (6.03-7.49)			
30-min	2.27 (2.08-2.48)	2.71 (2.48-2.97)	3.24 (2.96-3.55)	3.64 (3.33-3.98)	4.09 (3.72-4.46)	4.41 (4.00-4.82)			5.30 (4.71-5.79)	5.55 (4.88-6.07)			
60-min	1.42 (1.30-1.55)	1.70 (1.56-1.86)	2.08 (1.90-2.27)	2.37 (2.17-2.59)	2.72 (2.47-2.97)	2.99 (2.71-3.26)	3.25 (2.92-3.54)	3.49 (3.13-3.82)	3.80 (3.38-4.16)	4.05 (3.56-4.43)			
2-hr	0.833 (0.759-0.919)	1.00 (0.916-1.11)	1.24 (1.13-1.37)	1.43 (1.29-1.57)	1.66 (1.50-1.83)	1.85 (1.66-2.03)	2.03 (1.81-2.23)	2.21 (1.96-2.42)	2.44 (2.14-2.68)	2.63 (2.29-2.90)			
3-hr	0.589 (0.538-0.649)	0.710 (0.650-0.783)	0.880 (0.803-0.969)	1.02 (0.928-1.12)	1.20 (1.08-1.31)	1.34 (1.21-1.47)	1.49 (1.33-1.63)	1.64 (1.45-1.79)	1.83 (1.60-2.01)	2.00 (1.73-2.20)			
6-hr	0.354 (0.325-0.388)	0.427 (0.392-0.468)	0.529 (0.484-0.580)	0.614 (0.560-0.672)	0.725 (0.657-0.792)	0.817 (0.736-0.892)	0.910 (0.812-0.992)	1.01 (0.889-1.10)	1.14 (0.990-1.24)	1.24 (1.07-1.36)			
12-hr	0.208 (0.191-0.228)	0.250 (0.230-0.274)	0.312 (0.286-0.341)	0.364 (0.332-0.398)	0.434 (0.393-0.472)	0.493 (0.443-0.535)	0.553 (0.492-0.599)	0.616 (0.542-0.667)	0.704 (0.609-0.762)	0.779 (0.663-0.844			



BASINS

Volume Required	1800 cf/disturbed acre					
Volume Recommended	1800 cf/drainage acre					

Volume:

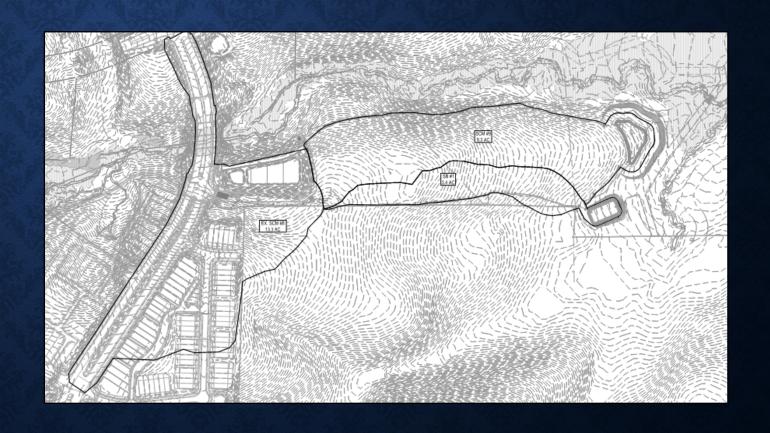
- > Measured to the elevation of crest of principal spillway:
 - Riser basin = top of riser
 - Skimmer basin = emergency spillway



BASINS

Drainage Area (A)

- > Map included to confirm inputs
- > Drainage area in acres
- Must include all drainage that would drain to basin





BASINS

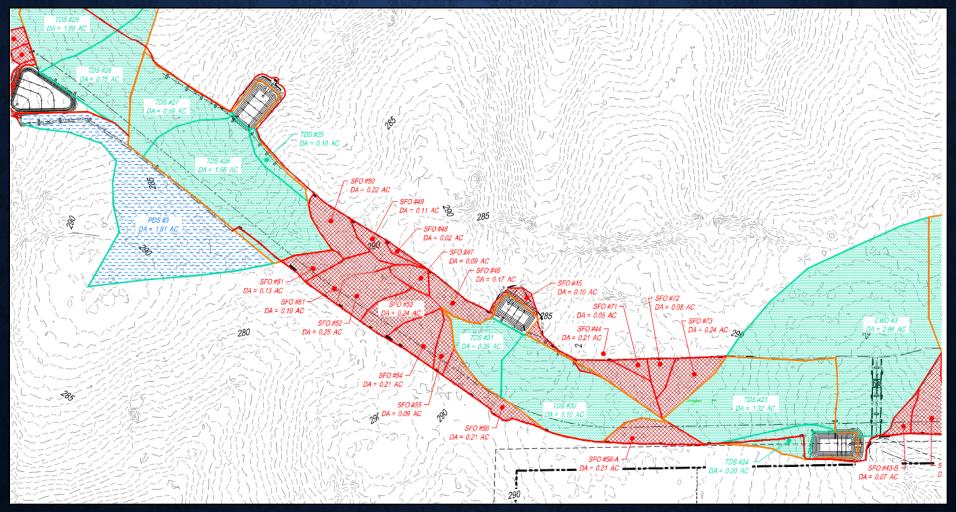
Drainage Maps Include:

- > Drainage values
- > Contours
- > Limits of disturbance
- > Measures analyzed





BASINS



Drainage Maps for silt fence outlets to confirm all are < 0.25 acres / 100 LF silt fence outside basin drainage.



BASINS

TEMPORARY SKIMMER BASIN & ORIFICE DIAMETER CALCULATIONS

BASIN	TYPE	DRAINAGE AREA	DISTURBED AREA	C VALUE	PEAK FLOW	DEPTH	LENGTH	WIDTH	WEIR LENGTH	VOLUME REQUIRED	VOLUME PROVIDED	SURFACE AREA REQUIRED	SURFACE AREA PROVIDED	SKIMMER SIZE	ORIFICE SIZE	Dewatering Time
		(AC)	(AC)		(CFS)	(FT)	(FT)	(FT)	(FT)	(CF)	(CF)	(SF)	(SF)	(IN)	(IN)	(DAYS)
SSB#1	SKIMMER	9.99	9.99	0.50	41.26	4.5	SEE EC	PLANS	38	35,964	42,517	17,948	19,014	4.0	3.25	3.02
SSB#2	SKIMMER	8.87	8.87	0.50	36.63	4.5	SEE EC	PLANS	34	31,932	36,939	15,935	16,319	4.0	3	3.08
SSB#3	SKIMMER	9.97	9.97	0.50	41.18	4.0	SEE EC	PLANS	38	35,892	97,108	17,912	51,871	5.0	4.5	3.22

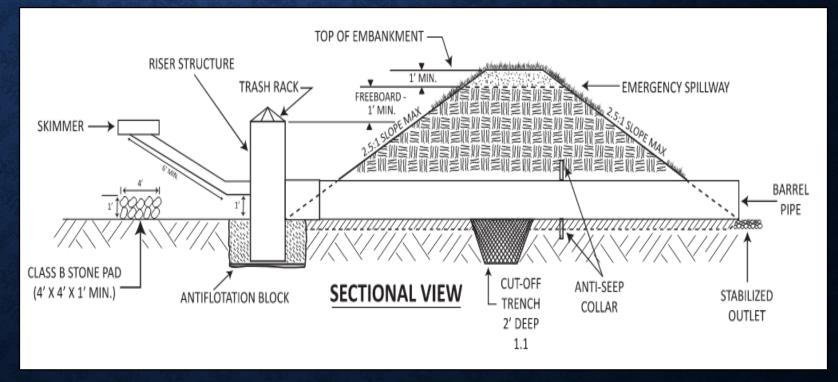
- > The reviewer can ask for clear representation of design information
- > This table should be in the plan or information otherwise called out



RISER BASINS

Risers:

- ➤ Design minimum 2-yr
- > Riser profiles provided
- > Protect against piping:
 - Watertight anti-seep collar
 - Filter diaphragm
- > Protect against flotation
 - Anti-flotation block
- > Cut-off trench

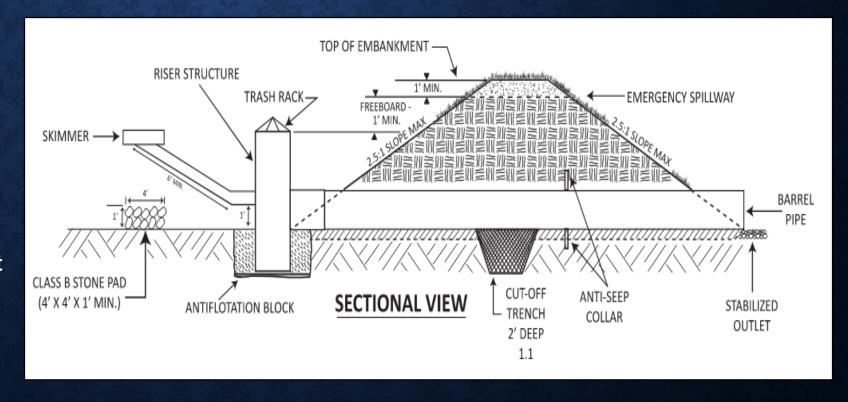




RISER BASINS

Embankment:

- Riser crest 1 ft. below emergency spillway crest
- ➤ Max Dam Height: 15 feet
- > Top width:
 - Fill height < 10 ft = 8.0 ft
 - Fill height 10-15 ft = 10.0 ft
- ➤ Sideslopes 2.5:1 or flatter

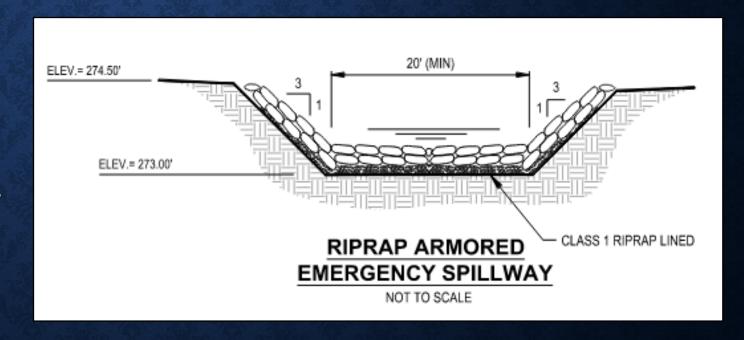




RISER BASINS

Emergency Spillway:

- Design minimum 10-yr
- ➤ Minimum 1 foot of freeboard
- ➤ Trapezoidal, sideslopes 3:1 or flatter
- Control section straight and at least 20 ft wide





RISER BASINS

Emergency Spillway:

- > Stabilized appropriately
- Velocity of flow discharged non-erosive



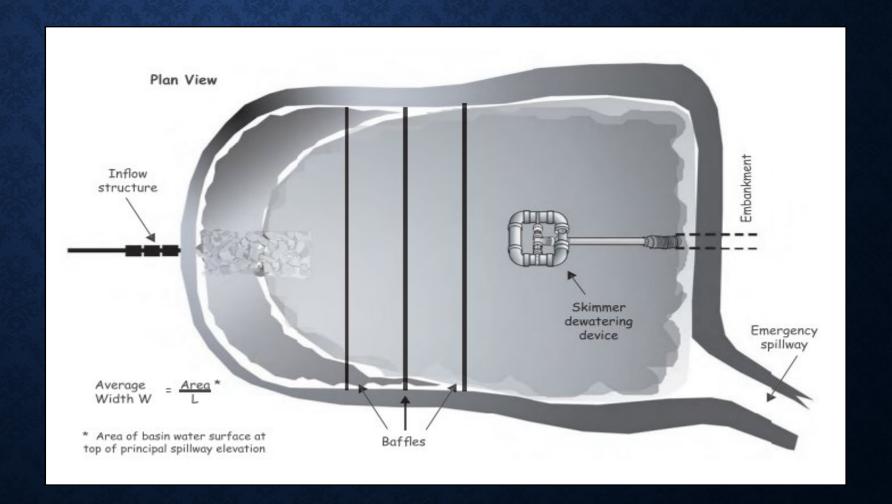




SKIMMER BASINS

Embankment:

- Max Dam Height: 5 feet
- > Top width: 5 feet
- ➤ Sideslopes 2:1 or flatter





SKIMMER BASINS

Emergency Spillway:

- > Same as riser basin
- ➤ Max depth of flow 6" from design storm elevation to spillway invert
- > Designed length

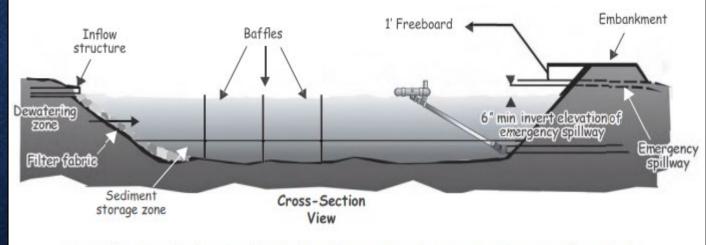


Figure 6.64c Example of a sediment basin with a skimmer outlet and emergency spillway. From Pennsylvania Erosion and Sediment Pollution Control Manual, March, 2000.



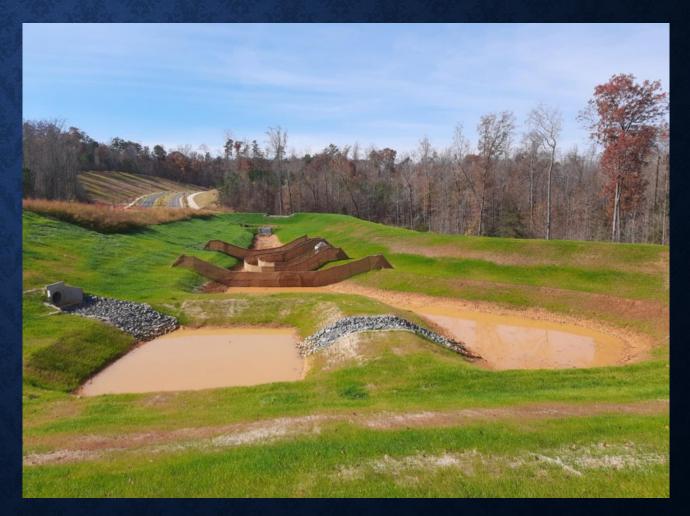
ALL BASINS

Embankment:

➤ Stabilized:

- Temporary < 1 year
- Permanent ≥ 1 year







ALL BASINS

Skimmer:

- Attached to base of riser or through bottom of dam for skimmer basins
- > Sized appropriately
- Dewater in 2-5 days

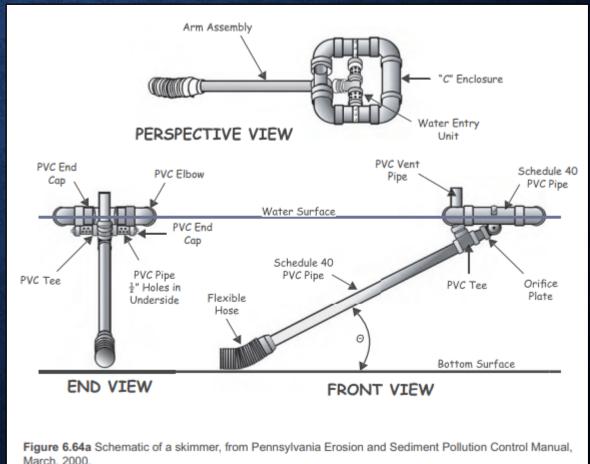




ALL BASINS

Skimmer:

- ➤ Schedule 40 PVC barrel
- > Sit on rip rap pad to prevent sticking
- > Retrieval rope



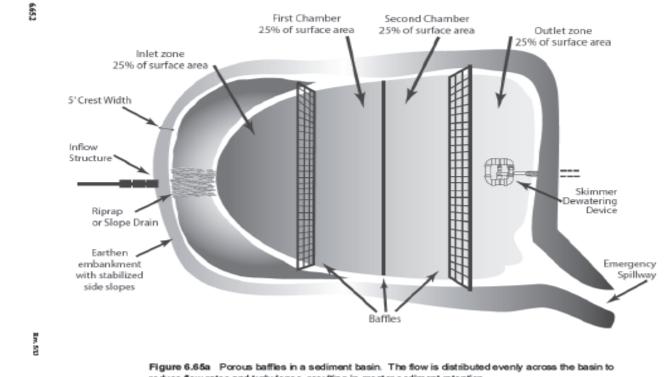
March, 2000.



ALL BASINS

Baffles:

- Minimum (3) porous baffles unless basin < 20 ft
- > Separate basin to even cells
- > Purpose is to spread out flow and utilize surface area
- > Should not be located directly at basin inlet



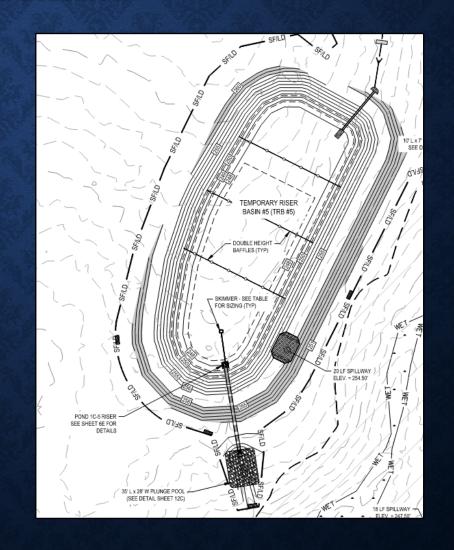
reduce flow rates and turbulence, resulting in greater sediment retention.



ALL BASINS

Baffles:

- Top of baffle fabric 6" higher than invert of spillway
- ➤ Top of baffle 2" lower than top of dam
- ➤ If temporary basin will be converted to permanent stormwater basin of greater depth, baffle height based on pool depth during use as temporary basin
- ➤ Increase in baffle height may be necessary





ALL BASINS





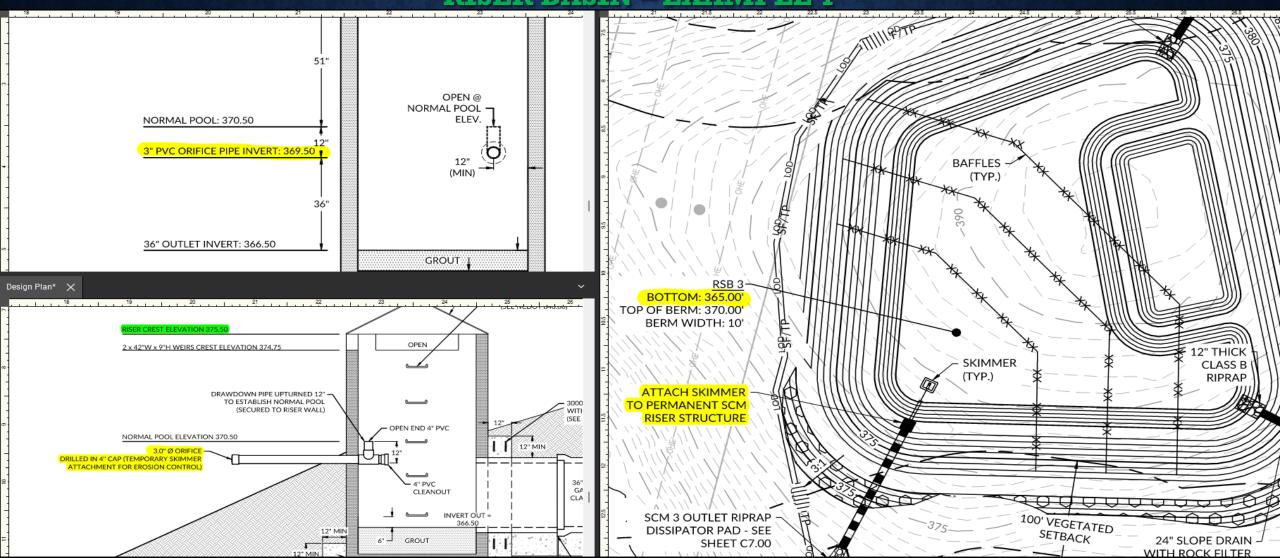








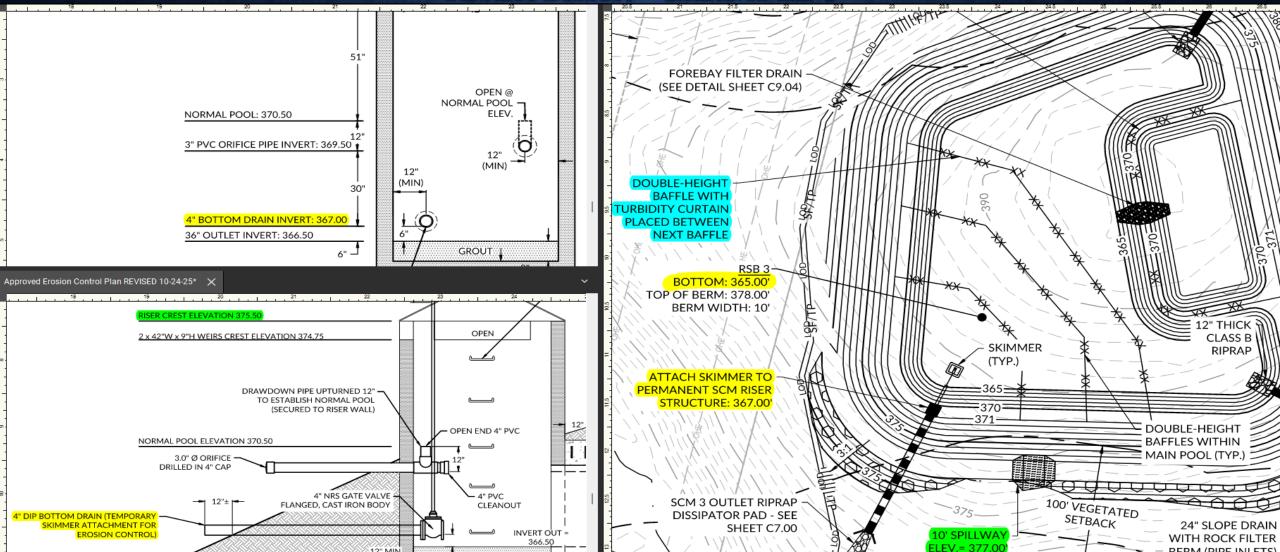
RISER BASIN – EXAMPLE 1







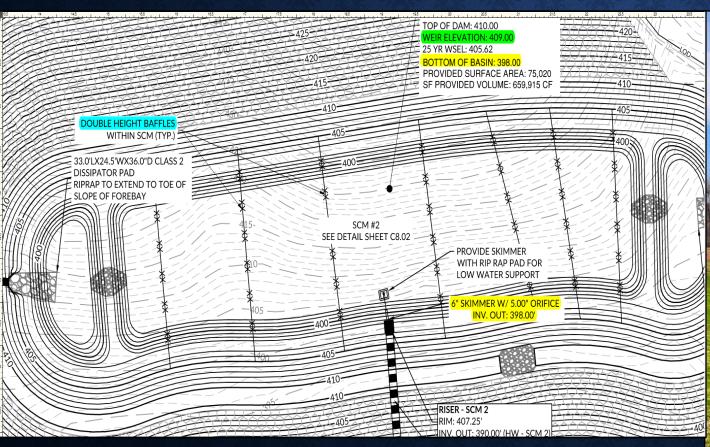








RISER BASIN – EXAMPLE 2



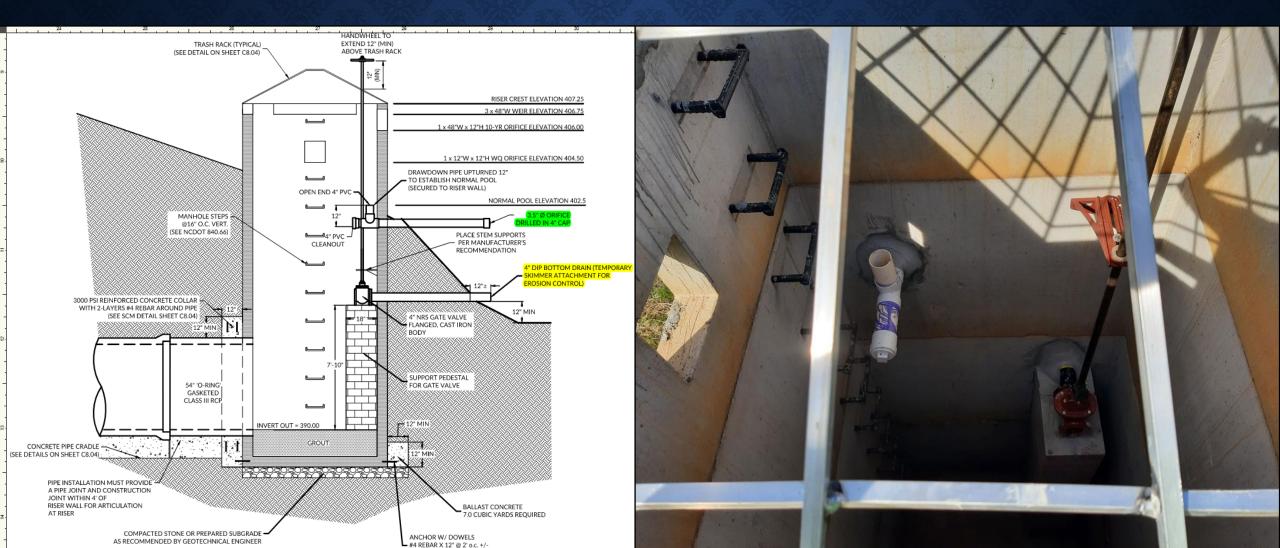
Sediment basin to permanent depth







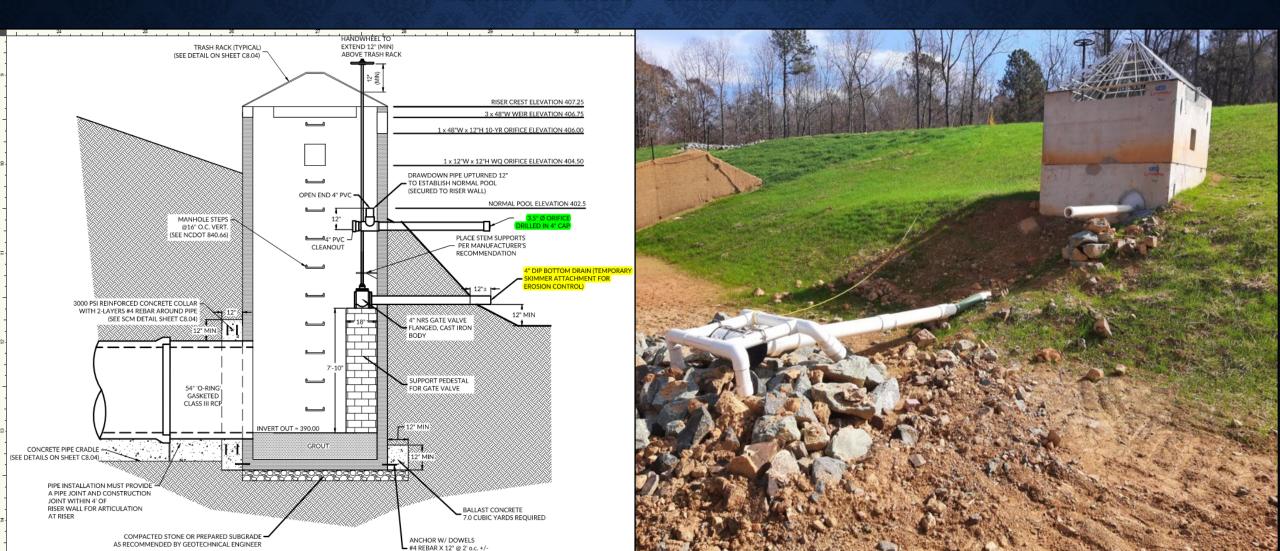
RISER BASIN – EXAMPLE 2







RISER BASIN – EXAMPLE 2





RISER BASIN - EXAMPLE 2





RISER BASIN – EXAMPLE 2





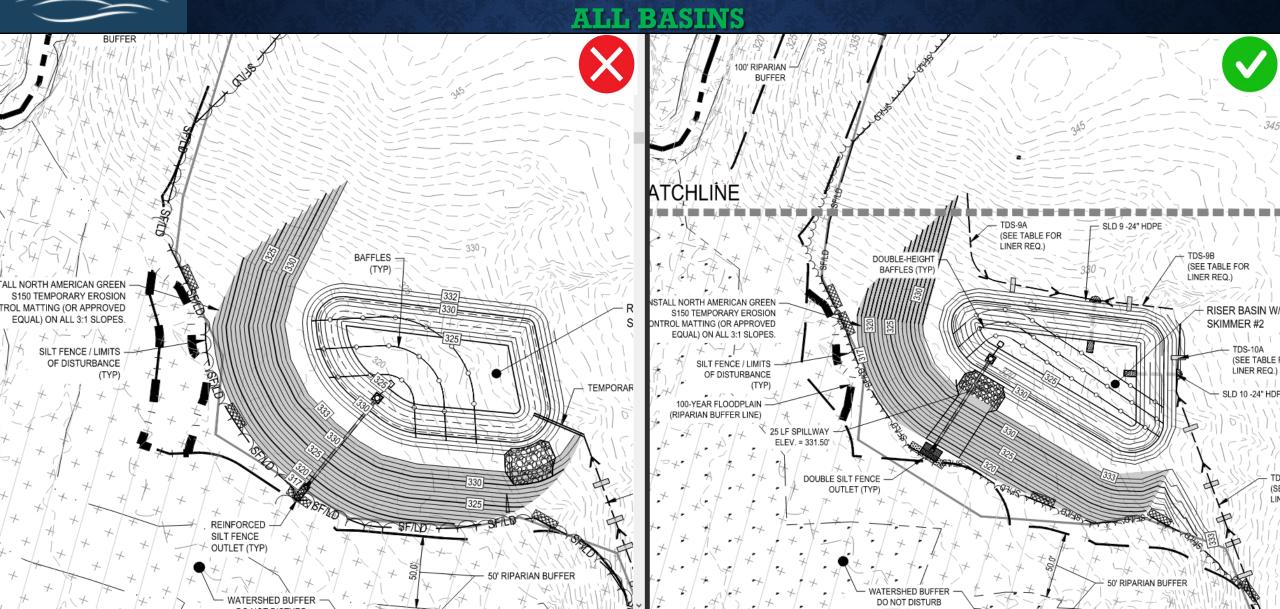
ALL BASINS

Other:

- \triangleright Basin shape = 2:1 to 6:1
- > Longest flow path
- ➤ No sheet flow
- > Stable inlets
 - Slope drain
 - Rip rap lined

	SLOPE DRAIN SIZING										
							Slope	Slope			
Slope	Diversion	Drainage		I (Q ₂₅ ,		Ditch	Drain Size	Drain Area	h Value	Slope Drain	
Drain ID	Ditch ID	Area (Ac)	С	in/hr)	Q ₂₅ (cfs)	Depth (ft)	(in)	(SF)	(ft)	Capacity (cfs)	
1	DD-1	0.06	0.60	8.30	0.30	1.0	15	1.23	0.375	3.62	
2	DD-2	0.61	0.60	8.30	3.04	1.5	15	1.23	0.875	5.53	
3	DD-3	0.24	0.60	8.30	1.20	1.0	15	1.23	0.375	3.62	
4	DD-4 & 7	2.43	0.60	8.30	12.10	2.0	24	3.14	1.000	15.13	
5	DD-5	1.00	0.60	8.30	4.98	1.5	15	1.23	0.875	5.53	
6	DD-6	1.48	0.60	8.30	7.37	1.5	18	1.77	0.750	7.37	
** Basin sid	de slope to l	be armored	with 12" N	CDOT Class	B Rip Rap (d	150 = 6") wit	th geotextile	e fabric base	·.		







ALL BASINS

Other:

Stabilize all areas disturbed by construction





> Outlet protection

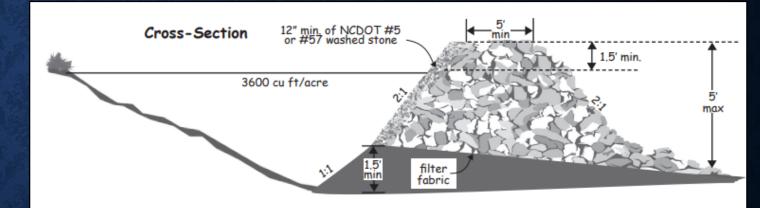


TEMPORARY SEDIMENT TRAPS

Criteria:

- > Used for smaller drainage areas
- ➤ Miniature basin with no skimmer
- > Capacity, stone outlet, and stabilization very important

Design Criteria	Summary:	Temporary Sediment Trap
	Primary Spillway:	Stone Spillway
	Maximum Drainage Area:	5 acres
	Minimum Volume:	3600 cubic feet per acre of disturbed area
	Minimum Surface Area:	435 square feet per cfs of Q ₁₀ peak inflow
	Minimum L/W Ratio:	2:1
	Minimum Depth:	3.5 feet, 1.5 feet excavated below grade
	Maximum Height:	Weir elevation 3.5 feet above grade
	Dewatering Mechanism:	Stone Spillway
	Minimum Dewatering Time:	N/A
	Baffles Required:	3



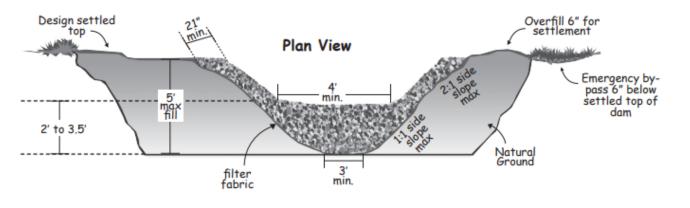


Figure 6.60a Plan view and cross-section view of a temporary sediment trap.



DITCH/SWALE

Parabolic



Trapezoidal



Triangular



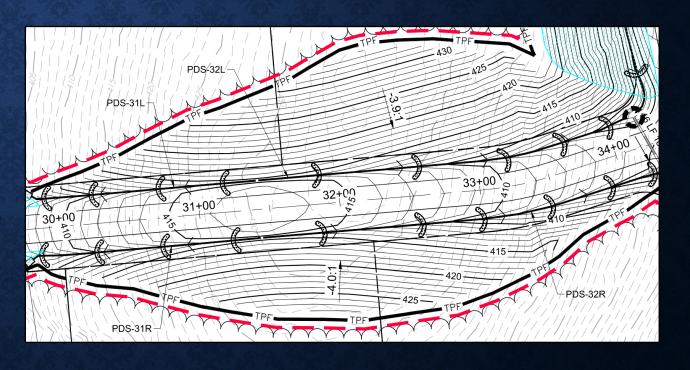
Design and site conditions dictate.



DITCH/SWALE

Capacity

- > 10-year storm w/o eroding
- > Increased capacity for flood hazards
- Check dams/wattles every 2' in grade change

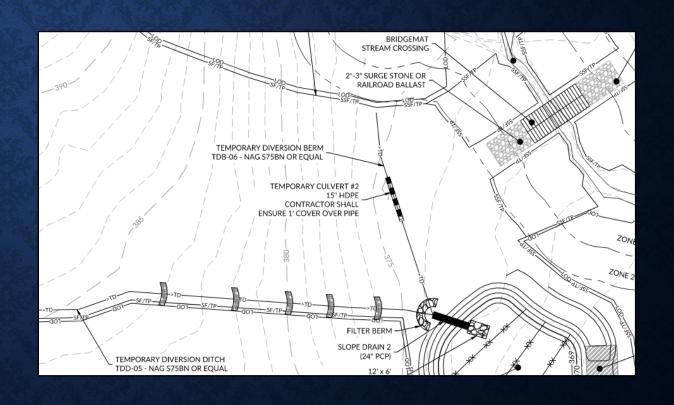




DITCH/SWALE

Grade

- > Positive drainage
- > Uniform or gradually increasing
- > 3:1 slopes or flatter
- > Ditch should be uninterrupted





DITCH/SWALE





DITCH/SWALE

Drainage Area

- > Drainage areas should match calcs
- Most onsite drainage should be accounted for
- ➤ Basin drainage > sum of ditches





DITCH/SWALE

Velocity

- ➤ Allowable design determined by soil conditions, type of vegetation, method of establishment
- > Maximum permissible velocities table
- ➤ Preferred < 5.0 ft/sec up to 6.0 ft/sec
- > Velocity > 2.0 ft/sec requires liner
 - Temporary liner: 2-year storm
 - Permanent liner: 10-year storm



Table 8.05a Maximum Allowable Design Velocities¹ for Vegetated Channels

Typical Channel Slope Application	Soil Characteristics ²	Grass Lining	Permissible Velocity for Established Gras Lining (ft/sec)
0-5%	Easily Erodible Non-plastic (Sands & Silts)	Bermudagrass Tall fescue Bahiagrass Kentucky bluegrass Grass-legume mixture	5.0 4.5 4.5 4.5 3.5
	Erosion Resistant Plastic (Clay mixes)	Bermudagrass Tall fescue Bahiagrass Kentucky bluegrass Grass-legume mixture	6.0 5.5 5.5 5.5 4.5
5-10%	Easily Erodible Non-plastic (Sands & Silts)	Bermudagrass Tall fescue Bahiagrass Kentucky bluegrass Grass-legume mixture	4.5 4.0 4.0 4.0 3.0
	Erosion Resistant Plastic (Clay mixes)	Bermudagrass Tall fescue Bahiagrass Kentucky bluegrass Grass-legume mixture	5.5 5.0 5.0 5.0 3.5
>10%	Easily Erodible Non-plastic (Sands & Silts)	Bermudagrass Tall fescue Bahiagrass Kentucky bluegrass	3.5 2.5 2.5 2.5
	Erosion Resistant Plastic (Clay mixes)	Bermudagrass Tall fescue Bahiagrass Kentucky bluegrass	4.5 3.5 3.5 3.5

Source: USDA-SCS Modified

NOTE: 1Permissible Velocity based on 10-year storm peak runoff

²Soil erodibility based on resistance to soil movement from concentrated flowing water.

*Before grass is established, permissible velocity is determined by the type of temporary liner used.



DITCH/SWALE

Shear Stress

- > Is the point at which soil will erode
- > Determines what liner is required
- ➤ Liner: Appendix 8 of NCDEQ Manual
- Calculated shear stress < permissible shear stress of liner

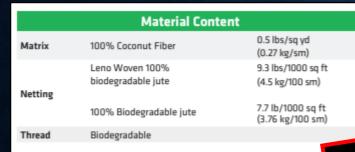
Table 8.05g
Permissible Shear Stresses
for Riprap and Temporary
Liners

	Permissible Unit Shea	r Stress, T _d
Lining Category	Lining Type	(lb/ft²)
Temporary	Woven Paper Net	0.15
	Jute Net	0.45
	Fiberglass Roving: Single	0.60
	Double	0.85
	Straw with Net	1.45
	Curled Wood mat	1.55
	Synthetic Mat	2.00
	d ₅₀ Stone Size (inches)	
Gravel Riprap	1	0.33
	2	0.67
Rock Riprap	6	2.00
	9	3.00
	12	4.00
	15	5.00
	18	6.00
	21	7.80
	24	8.00

Adapted From: FHWA, HEC-15, April 1983, pgs. 17 & 37.



DITCH/SWALE





Specification Sheet

BioNet® C125BN® Erosion Control Blanket

DESCRIPTION

The long-term double net ension control blanket shall be a machine-produced mat of 100% convent fiber with a functional long-vity of up to 18 months. (NOTE functional long-vity may vary depending upon climatic conditions, soil, geographical location, and elevation). The blanket shall be of consistent thickness with the concrust evenly distributed over the entire area of the mat. The blanket shall be covered on the top and bottom sides with 100% blooding aduble vower natural organic fiber netting. The natting shall consist of machine directional strands formed from two intertwined years with cross directional strands formed from two intertwined years with cross directional strands formed from two intertwined years with cross directional strands formed from two intertwined years with cross directional strands formed from two intertwined years with cross directional strands formed from two intertwined years with cross directional strands formed from two intertwined years with cross directional strands formed from two intertwined years and transfer of the control of the strands for the control of the strands for the strands of the control of the strands of the s

The C125BN shall meet Type 4 specification requirements established by the Erosion Control Technology Council (ECTC) and Federal Highway Administration's (FHWA) FD-03 Section 2017

Material Content 100% Coconut Fiber 0.5 (bs/kg/yd) (0.27 kg/ym)

Lenc Wosen 100% 9.3 lbs/1000 sq ft blodegradable jute (4.5 kg/100 sup ft 100% Blodegradable jute 7.7 lb/1000 sq ft (2.76 kg/100 sm)

| Bioderradabl

Weight a 10%

secongracasore

\$2.22 lbs (23.69 kg)

90 sq yd (66.9 sm) 100 sq yd (83.61 sm)
Leno weave top only Leno weave top and botton

57(8)(1)

Index Property	Test Method	Typical
Thickness	ASTM DES2S	0.22 in. (5.84 mm)
Resiliency	ECTC Guidelines	85%
Water Absorbency	ASTM 01117	360%
Mass/Unit Area	ASTM 6475	9.79 oz/ey (222 g/um)
Swell	ECTC Guidelines	40%
Smolder Resistance	ECTC Guidelines	Yes
Stiffness	ASTM DI388	0.11 co-in
Light Penetration	ASTM DESET	16.2%
Tensile Strength - MD	ASTM DEBIG	206.4 lbs/ft (2.06 kH/m)
Elongation - MD	ASTM DGB18	15.2%
Tensile Strength - TO	ASTIM DEBIR	145.2 lbs/ft (2.15 kN/m)
Elongation - TD	ASTM DG818	12.9%
Blomass Improvement	ASTIM 7922	472%

Design Permissible Shear Stress and Shear Stress 2.35 per (112 Pu)

Invegetated Shear Stress
Invegetated Velocity 10.0 tps (3.05 m/s)

Slope Design Data: C Factor

anap	a minming fall	
4.37	21-21	- 4
0.0001	0.018	0.0
0.002	0.040	0.0
0.007	0.070	0.0
	0.0001 0.0002	0.0001 0.018 0.003 0.040

Roughness Coefficie

Manningsn
0.022
0.022-0.014
0.014



Western Enrigh 4609 E. Boonville-New Harmony Rd N

65.25 lbs (29.61 kg)

nagreen.com 800-772-2040 COSTI, North Abentum Common or engineered insultermant from Windows Commo Consiste granted in adult applications desired for the facilities between our problemed and the own or more U.S. patients. Other U.S. patients are precining, and certain therapy patients and garden applications may also most in Spatients and precining and certain therapy patients and patients. They applications may also most in Spatients in optication applications and patients. They described the optication of the contribution of any followed lates or makes of the time over contemplated, and for makes of the contribution of any followed lates. Provided in the U.S. Provided

EC RMX MPDS_COSBN_119

Design Permissible Shear Stress

Unvegetated Shear Stress 2.35 psf (112 Pa)
Unvegetated Velocity 10.0 fps (3.05 m/s)

Slope Design Data: C Factors

	Slope Gradients (S)						
Slope Length (L)	≤ 3:1	3:1 - 2.1	≥ 2:1				
≤ 20 ft (6 m)	0.0001	0.018	0.050				
20-50 ft	0.003	0.040	0.060				
≥ 50 ft (15.2 m)	0.007	0.070	0.070				

Roughness Coefficients - Unveg.

Flow Depth	Manning's n
≤ 0.50 ft (0.15 m)	0.022
0.50 - 2.0 ft	0.022-0.014
≥ 2.0 ft (0.60 m)	0.014



DITCH/SWALE

Physical Characteristics of Drainage Swale									
Chan Bot	Side	Side Slope	Design	Channel	Wetted	Hydraulic	Channel		
Width (ft)	Slope	Length (ft)	Depth (ft)	Area (sf)	Perim., Pw (ft)	Radius (ft)	Slope (ft/ft)		
2	2	4.5	2	12.0	10.94	1.10	0.04		

Shear Stress, T= y*(d/12)*s

T = shear stress in lb/sq. ft. y = unit weight of water, 62.4 lb/cu. ft.

Rational Equation for 2 year storm				Additio	Swales		
		Drainage	Q2	Upstream swale#	Upstream swale#	Upstream swale#	Total Flow (cfs)
	Intensity	Area	Discharge		-	-	Total Flow (CIS)
C Factor	Inch/hr	(Acres)	(Cfs)	Flow (cfs)	Flow (cfs)	Flow (cfs)	
0.50	5.88	1.13	3.32	0.00	0.00	0.00	3.32

Rational Equation for 10 year storm				Rational Equation for 10 year storm Addition flow from upstream Swales				
		Drainage	Q10	Upstream swale#	Upstream swale#	Upstream swale#	Total Flow (cfs)	
l	Intensity	Area	Discharge				Total Flow (CIS)	
C Factor	Inch/hr	(Acres)	(Cfs)	Flow (cfs)	Flow (cfs)	Flow (cfs)		
0.50	7.48	1.13	4.23	0.00	0.00	0.00	4.23	

		Bare Soil		
Material	Recom	mended n Values f	or Flow Depth	Permissible
Waterial	0-0.5 ft	0.5-2.0 ft	> 2.0 ft	velocitiy (fps)
Bare Soil	0.023	0.02	0.02	2.00

Temporary Liners								
*Material	Recom	Allowable						
Waterial	0-0.5 ft	0.5-2.0 ft	> 2.0 ft	Shear Stress (lb/sf)				
		Unvegetated						
S75	0.055	0.028	0.021	1.55				
S150	0.055	0.028	0.021	1.65				
SC150	0.050	0.025	0.018	1.80				
C125	0.022	0.014	0.014	2.25				
C350	0.040	0.025	0.02	2.25				
P300	0.034	0.024	0.02	2.00				

Rip Rap Liners									
Material	Recom	Recommended n Values for Flow Depth							
Waterial	0-0.5 ft	0.5-2.0 ft	> 2.0 ft	Shear Stress (lb/sf)					
Class A	0.044	0.033	0.03	1.65					
Class B	0.066	0.041	0.034	3.00					
Class I	0.104	0.069	0.035	4.50					
Class II		0.078	0.04	6.00					

Spreadsheet works for trapezoidal channel only

Side slope = horiz./vert.

Output taken from AutoCad's Trapezoidial Ditch Calculater

d = flow depth in in. s = channel slope in ft./ft. * The Material lists refers to North American Green products. Contractor to use referenced material or approved equal.

Mannings	Q	AutoCad	AutoCad	Shear
"n"	Allow.	Depth (in)	Velocity (fps)	Stress (lb/sf)
0.020	190.12	3.1	5.17	0.65

Bare Soil calculations (Q2)

Temporary liner calculations (Q2)								
Mannings "n"	Q Allow.	AutoCad Depth (in)	AutoCad Velocity (fps)	Shear Stress (lb/sf)				
0.028	135.80	3.7	4.09	0.77				

Permanent liner calculations (Q10)									
Mannings	Q	AutoCad	AutoCad	Shear					
"n"	Allow.	Depth (in)	Velocity (fps)	Stress (lb/sf)					
0.030	126.75	4.4	4.17	0.92					

Permanent Liners							
*Material	Allowable						
Material	0-0.5 ft	0.5-2.0 ft	> 2.0 ft	Shear Stress (lb/sf)			
Vegetated							
C350 Phase 2	0.044	0.044	0.044	4.50			
C300 Phase 2	0.044	0.044	0.044	4.00			
C350 Phase 3	0.049	0.049	0.049	8.00			
P300 Phase 3	0.049	0.049	0.049	8.00			

Permanent Liners						
Material	Recommended n Values	Permissible velocitiy (fps)				
Grass Mixture	0.030	5.00				

Selected Temporary Liner: S75 Selected Permanent Liner: Grass Mixture

Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Wednesday, Aug 21 2024

ROADSIDE DITCH #1 10 YR STORM

Trapezoidal

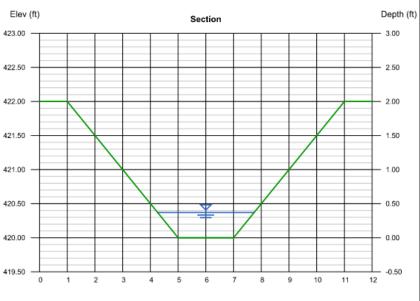
Bottom Width (ft) = 2.00Side Slopes (z:1) = 2.00, 2.00 Total Depth (ft) = 2.00Invert Elev (ft) = 420.00 Slope (%) = 4.00 N-Value = 0.030

Calculations

Compute by: Known Q Known Q (cfs) = 4.23

Highlighted

Depth (ft) = 0.37Q (cfs) = 4.230Area (sqft) = 1.01 Velocity (ft/s) = 4.17 Wetted Perim (ft) = 3.65Crit Depth, Yc (ft) = 0.45Top Width (ft) = 3.48EGL (ft) = 0.64



Reach (ft)



DITCH/SWALE

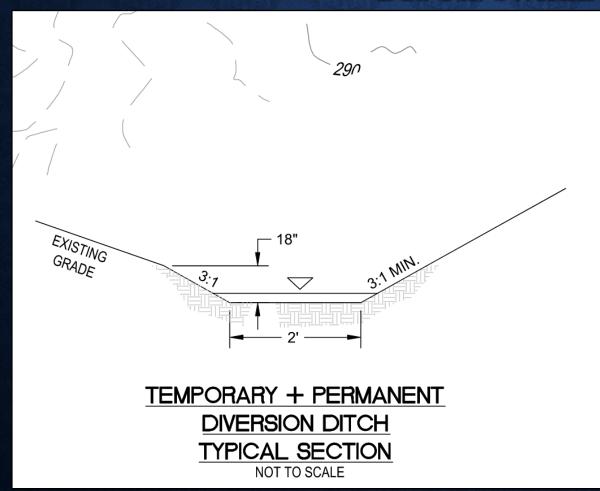
Design Storm	25-YR
Rainfall Intensity	8.26

						CHANNE	L DESIGN IN	FORMATION					
CHANNEL I.D.	DRAINAGE AREA (AC)	WEIGHTED 'C' COEFFICIENT	CHANNEL FLOW (CFS)	CHANNEL SLOPE (%)	CHANNEL FLOW DEPTH (FT)	CHANNEL MINIMUM DEPTH (FT)	CHANNEL BOTTOM WIDTH (ET)	CHANNEL SIDE SLOPES	CHANNEL VELOCITY (FPS)	ALLOWABLE VELOCITY (FPS)	CHANNEL SHEAR STRESS (PSF)	ALLOWABLE SHEAR STRESS (PSF)	DITCH LINING
CWTD #1	8.25	0.15	10.23	3.00	0.72	1.50	1.00	3:1	4.51	5.00	1.34	1.60	NAG S75BN OR EQUAL
CWTD #2	6.02	0.15	7.46	2.80	0.72	1.50	1.00	3:1	3.29	5.00	1.25	1.60	NAG S75BN OR EQUAL
SCMTD #1	1.65	0.50	6.83	7.00	0.52	1.50	1.00	3:1	5.20	10.00	2.25	2.25	NAG C125BN OR EQUAL
SCMTD #2	0.22	0.50	0.91	5.00	0.65	1.50	1.00	3:1	0.47	8.00	2.03	2.10	NAG SC150BN OR EQUAL
SCMTD #3	1.29	0.50	5.33	3.00	0.67	1.50	1.00	3:1	2.65	5.00	1.25	1.60	NAG S75BN OR EQUAL
SCMTD #4	0.93	0.50	3.83	4.00	0.49	1.50	1.00	3:1	3.11	5.00	1.23	1.60	NAG S75BN OR EQUAL
SCMTB #5	4.03	0.50	16.66	3.90	0.77	1.50	1.00	3:1	6.52	8.00	1.87	2.10	NAG SC150BN OR EQUAL
SCMTD #6	0.91	0.50	3.74	1.60	0.65	1.50	1.00	3:1	1.94	5.00	0.65	1.60	NAG S75BN OR EQUAL
SCMTD #7	1.41	0.15	1.74	3.00	0.77	1.50	1.00	3:1	0.68	5.00	1.44	1.60	NAG S75BN OR EQUAL
SCMTD #8	2.94	0.41	9.92	1.20	0.77	1.50	1.00	3:1	3.88	5.00	0.57	1.60	NAG S75BN OR EQUAL
SCMTB #9	4.66	0.50	19.24	1.00	0.65	1.50	1.00	3:1	9.99	10.00	0.40	2.25	NAG C125BN OR EQUAL
SCMTD #10	1.46	0.50	6.02	2.20	0.77	1.50	1.00	3:1	2.35	5.00	1.05	1.60	NAG S75BN OR EQUAL

Ditch depth, bottom width, sideslopes, and stabilization liner should be in plan.



DITCH/SWALE



Swale ID	Lining Type (or equal)
TDS-19	Eronet DS75
TDS-20	Eronet DS75
TDS-21	Eronet DS75
TDS-22	Eronet SC150
TDS-23	Eronet DS75
TDS-24	Eronet DS75
TDS-25	Eronet DS75
TDS-26	Eronet DS75
TDS-27	Eronet DS75
CWD-2	Eronet DS75
CWD-3	Eronet SC150
PDS-3	Eronet P300

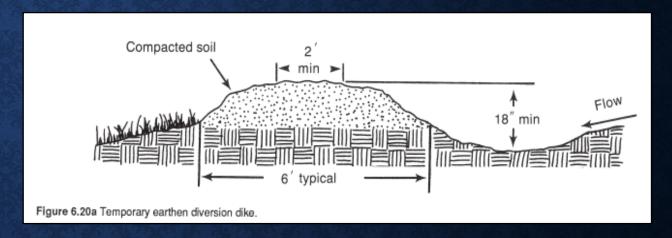
Ditch depth, bottom width, sideslopes, and stabilization liner should be in plan.

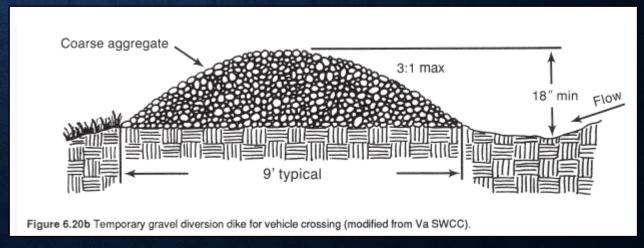


TEMPORARY DIVERSIONS

Criteria

- > Route construction runoff
- Divert offsite drainage around site
- ➤ Similar design to ditches
- ➤ Drainage area < 5 acres
- ➤ Design to 10-year storm







WATER BARS

Criteria

- Used: dirt roadways, utility corridors, and approaches to surface waters
- Similar design to diversions
- > Spacing based on slope

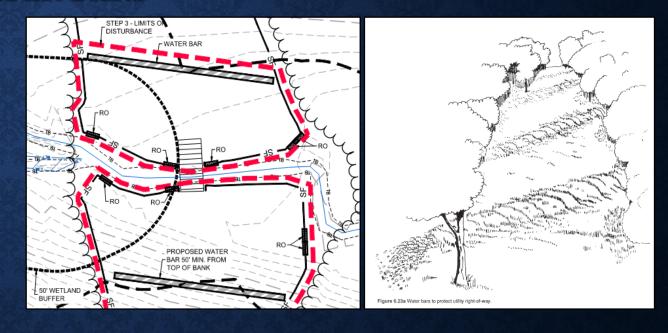
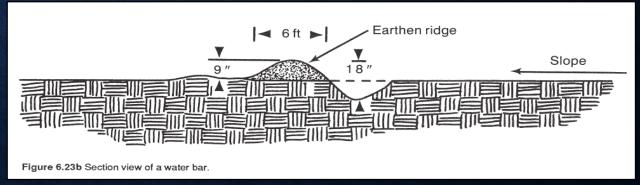


Table 6.23a Spacing of Water Bars on Right-of-Way Less than 100 ft Wide

Slope (%) Spacing (Ft)	1
<5 125	ı
5 to 10 100	ı
10 to 20 75	9
20 to 35 50	
>35 25	

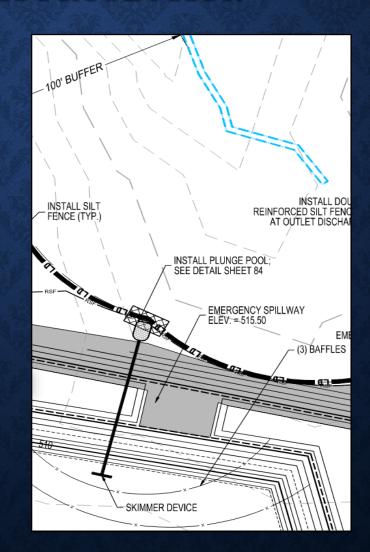


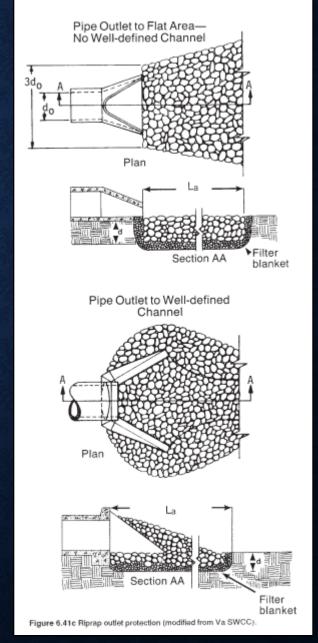


OUTLET PROTECTION

Criteria

- Channel, diversion, and conduit outlets must be stable
- Exit velocity for required design must be non-erosive
- Required design = conveyance
- > Filter fabric underliner

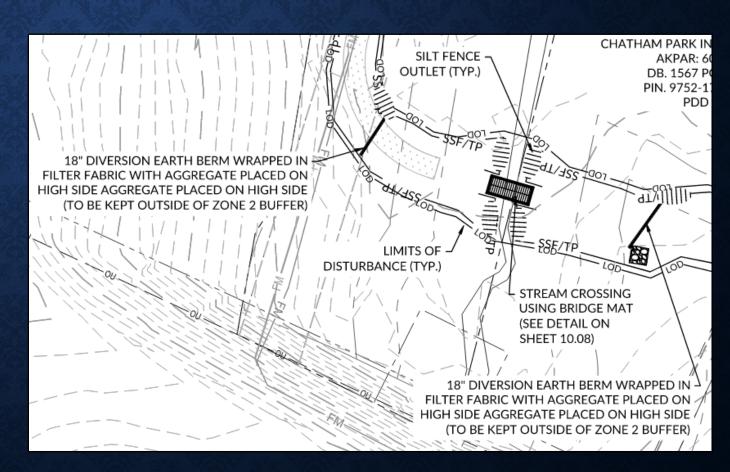






TEMPORARY STREAM CROSSING

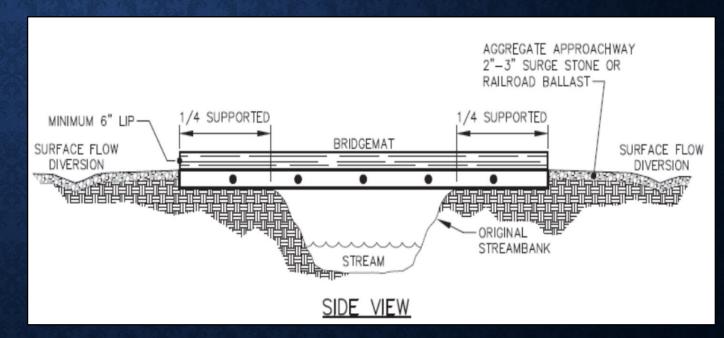
- > Try to avoid crossing streams
- If possible, complete development separately on each side
- ➤ Installed perpendicular to stream
- Often most sensitive area on site





TEMPORARY STREAM CROSSING - BRIDGEMAT

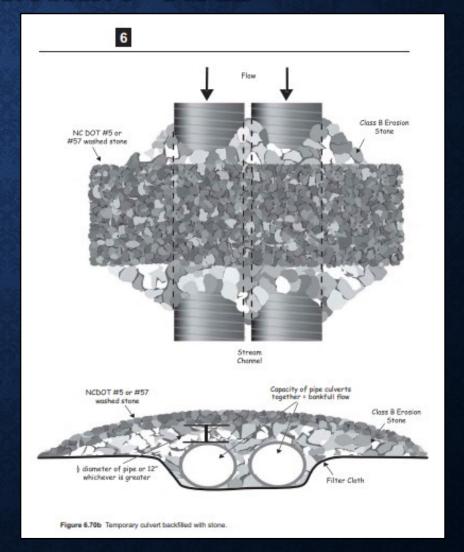
- > Preferred Method:
 - ➤ Least disturbance to stream bed, banks, and surrounding area
 - ➤ Often no stream impacts
 - Least obstruction to flow & fish migration
- ➤ Must support expected loads





TEMPORARY STREAM CROSSING - PIPED

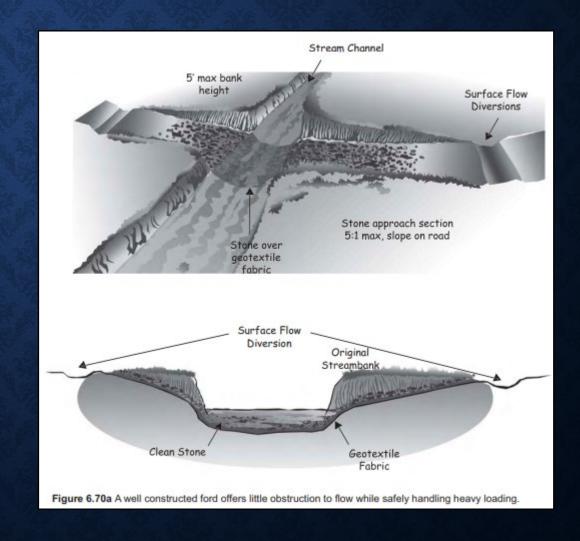
- ➤ Adaptable to most site situations
- ➤ Safely support heavy loads
- Installation and removal causes considerable disturbance to stream and surrounding area
- > Greatest obstruction to flood flows
- Subject to blockage and washout





TEMPORARY STREAM CROSSING - FORD

- > Used:
 - For steep areas subject to flash flooding
 - Normal flows < 3 inches deep
 - Intermittent streams
 - Only when crossings are infrequent
 - Wide, shallow watercourses
- ➤ Safely handle heavy loads
- ➤ Little to no obstruction of flow
- Could involve major disturbance in-stream
- > Approach area subject to erosion

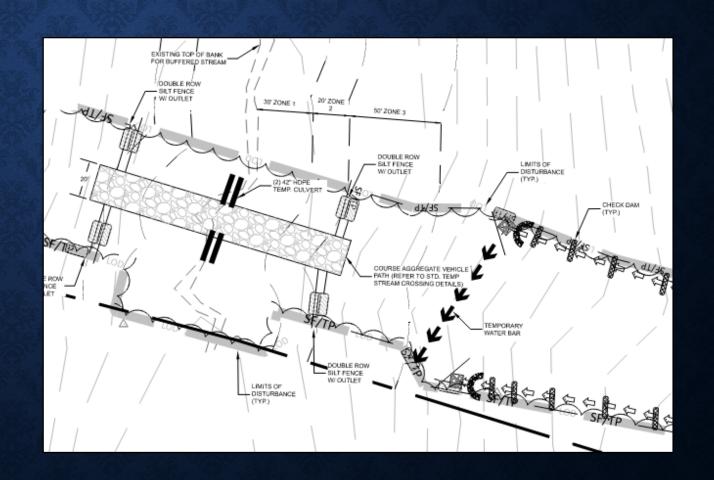




TEMPORARY STREAM CROSSING

Criteria

- ➤ Anticipated life = 1 year
- Minimum design: Pass bankfull flow or peak flow from 2-yr storm (10-25 yr. recommended)
- Consider overflow areas for larger storms
- Design flow velocity non-erosive for receiving stream
- Design other EC measures
 associated with stream crossing to
 10-year peak storm runoff

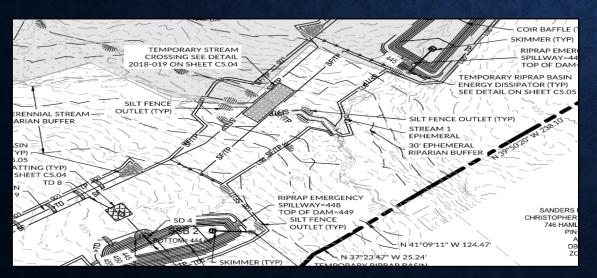


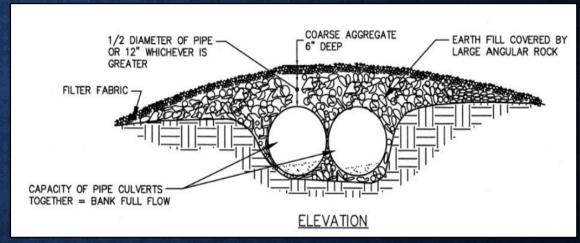


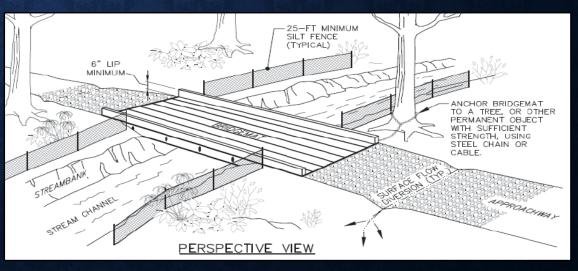
TEMPORARY STREAM CROSSING

Criteria

- > Silt fence at four corners
- Coarse aggregate approachways
- Lip to prevent sedimentation to stream
- ➤ Divert surface water





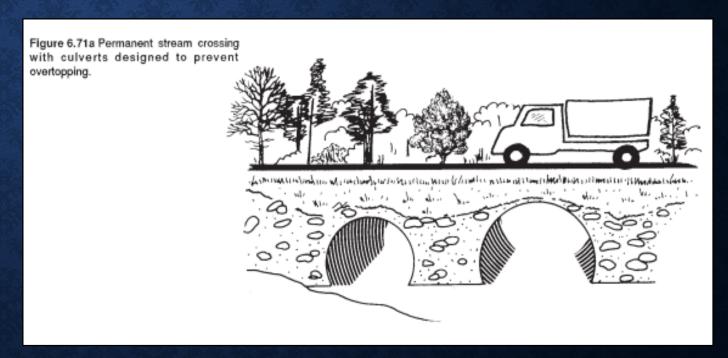




PERMANENT STREAM CROSSING

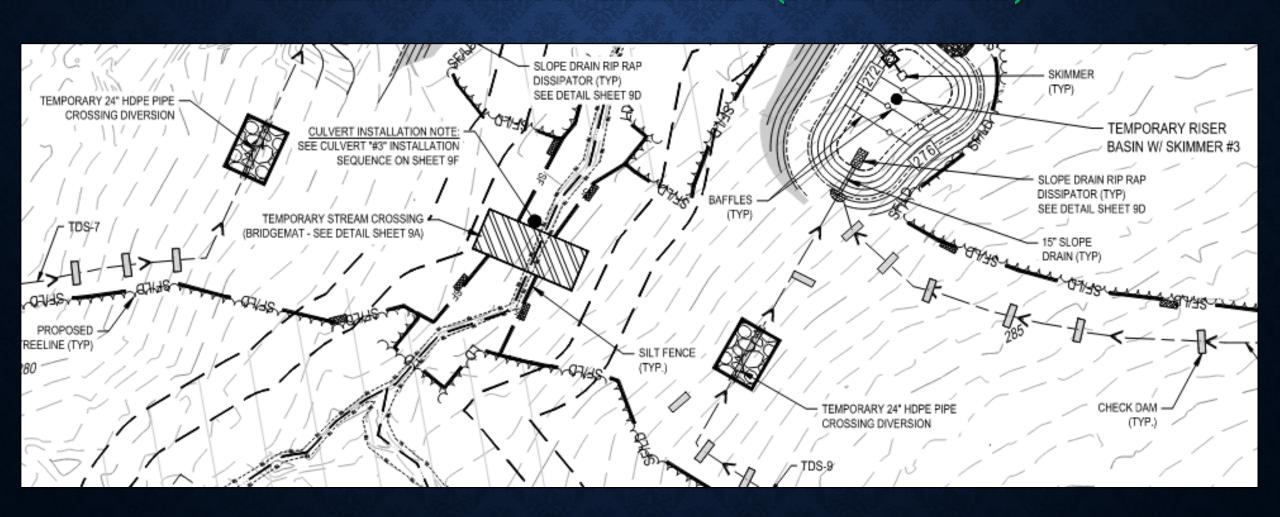
Planning Considerations

- Locate in higher, better drained sections of stream to minimize flooding
- ➤ Permanent protection measures where road water enters stream to prevent erosion
- Stream section at crossing protected from erosion due to flood flows



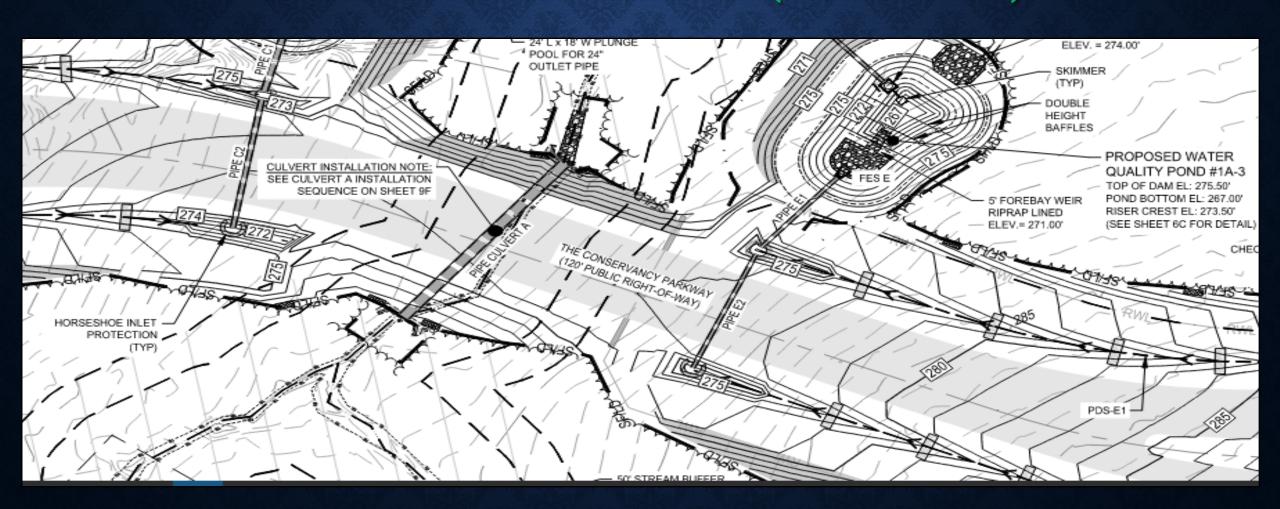


STREAM CROSSINGS – EXAMPLE 1 (PUMP AROUND)





STREAM CROSSINGS -EXAMPLE 1 (PUMP AROUND)





STREAM CROSSINGS – EXAMPLE 1 (PUMP AROUND)

CHANNNEL IMPACT SEQUENCE

GENERAL: WITH STAGE 1 CONSTRUCTION, CROSS STREAM WITH BRIDGEMAT TO ACCESS REMAINDER OF SITE AND INSTALL INITIAL TEMPORARY EROSION CONTROL MEASURES. SILT FENCE TO BE INSTALLED OUTSIDE OF TOP OF BANKS OF STREAM WITH REINFORCED SILT FENCE OUTLETS AS SHOWN.

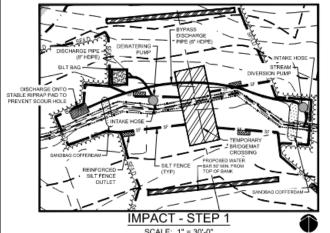
THE CONTRACTOR SHALL NOT IMPACT THE STREAM UNTIL THE CULVERT AND HEADWALLS HAVE BEEN DELEIVERED ON-SITE AND READY FOR INSTALLATION. WORK WITHIN THE STREAM SHOULD ONLY BE UNDERTAKEN WITH THE EXPECTATION OF 4-5 DAYS OF DRY WEATHER AS FORECASTED BY NWS.

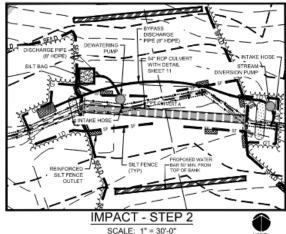
STEP 1: AFTER UPSTREAM DIVERSIONS ARE FUNCTIONING; INSTALL WATER BARS (DIVERSION) AS SHOWN. NO ACTIVITY ALLOWED WITHIN STREAM CHANNEL. LAY 8" FUSED HDPE BYPASS DISCHARGE PIPE ON TOP OF GROUND AND POSITION STREAM DIVERSION PUMP UPSTREAM OF COFFER DAM. INSTALL SILT BAG AT DOWNSTREAM END OF 8" HDPE BYPASS DISCHARGE PIPE (OUTSIDE TOP OF BANK) INSTALL UPSTREAM AND DOWNSTREAM SANDBAG COFFER DAMS. CLEARING ONLY AS NECESSARY WITHIN THE STREAM AND NORTHERN TOP OF BANK TO INSTALL COFFER DAM AND DOWNSTREAM CLASS II RIP RAP DISSIPATOR. ACTIVATE BYPASS PUMPING SYSTEM.

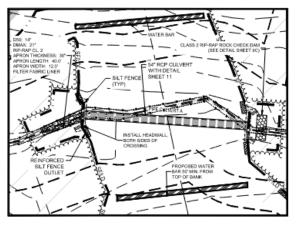
STEP 2: WHILE MAINTAINING BYPASS PUMPING SYSTEM, EXCAVATE CHANNEL FOR CULVERT PIPE STARTING FROM DOWNSTREAM END. PLACE ON A 4,000 PSI CONCRETE MUD SILL AS DIRECTED BY GEOTECHNICAL ENGINEER. EXCAVATE GRADE 4' BEYOND OUTSIDE EDGE OF RCP ON EACH SIDE TO ALLOW ADEQUATE WIDTH FOR RECOMPACTION. INSTALL RCP STARTING FROM DOWNSTREAM END THEN HEADWALLS. UTILIZE EARTHEN MATERIAL STOCKPILED AS SELECT BACKFILL TO PIPE SPRINGLINE (APPROVED BY GEOTECHNICAL ENGINEER). INSTALL RIP RAP DISSIPATOR. IMPACT NORTHERN CHANNEL TOP OF BANK AND BEYOND ONLY AS NECESSARY TO INSTALL PIPE AND HEADWALL.

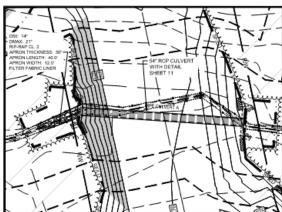
STEP 3: AFTER PIPE AND HEADWALLS ARE COMPLETE AND BACKFILLED; INSTALL SILT FENCING ALONG HEADWALL AND ON NORTH SIDE OF EXISTING CHANNEL BANK AS SHOWN WITH REINFORCED SILT FENCE OUTLETS. REMOVE UPPER COFFER DAM AND REPLACE WITH CLASS II RIP RAP FILTER WITH WASH STONE FACE (ROCK CHECK DAM WITH CLASS II RIP RAP - SEE DETAIL SHEET 9C).

STEP 4: AFTER TEMPORARY GROUND STABILIZATION IS ACHIEVED; REMOVE UPSTREAM RIP RAP FILTERS AND MAINTAIN SILT FENCE AT TOE OF SLOPES AND ALONG HEADWALLS.









IMPACT - STEP 3 SCALE: 1" = 30'-0"







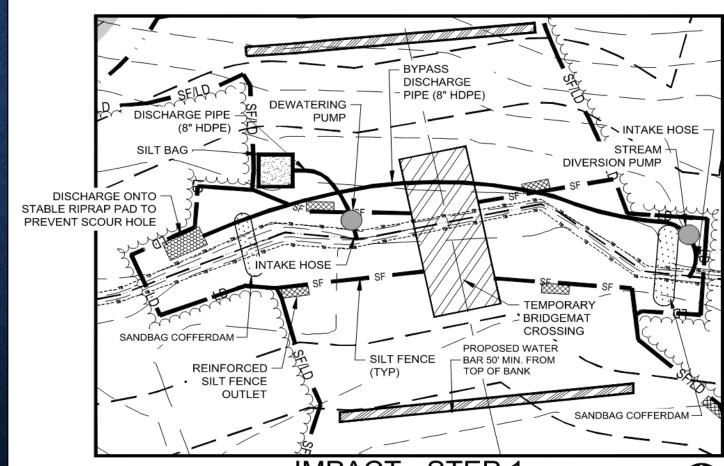
STREAM CROSSINGS – EXAMPLE 1 (PUMP AROUND)

CHANNNEL IMPACT SEQUENCE

GENERAL: WITH STAGE 1 CONSTRUCTION, CROSS STREAM WITH BRIDGEMAT TO ACCESS REMAINDER OF SITE AND INSTALL INITIAL TEMPORARY EROSION CONTROL MEASURES. SILT FENCE TO BE INSTALLED OUTSIDE OF TOP OF BANKS OF STREAM WITH REINFORCED SILT FENCE OUTLETS AS SHOWN.

THE CONTRACTOR SHALL NOT IMPACT THE STREAM UNTIL THE CULVERT AND HEADWALLS HAVE BEEN DELEIVERED ON-SITE AND READY FOR INSTALLATION. WORK WITHIN THE STREAM SHOULD ONLY BE UNDERTAKEN WITH THE EXPECTATION OF 4-5 DAYS OF DRY WEATHER AS FORECASTED BY NWS.

STEP 1: AFTER UPSTREAM DIVERSIONS ARE FUNCTIONING; INSTALL WATER BARS (DIVERSION) AS SHOWN. NO ACTIVITY ALLOWED WITHIN STREAM CHANNEL. LAY 8" FUSED HDPE BYPASS DISCHARGE PIPE ON TOP OF GROUND AND POSITION STREAM DIVERSION PUMP UPSTREAM OF COFFER DAM. INSTALL SILT BAG AT DOWNSTREAM END OF 8" HDPE BYPASS DISCHARGE PIPE (OUTSIDE TOP OF BANK) INSTALL UPSTREAM AND DOWNSTREAM SANDBAG COFFER DAMS. CLEARING ONLY AS NECESSARY WITHIN THE STREAM AND NORTHERN TOP OF BANK TO INSTALL COFFER DAM AND DOWNSTREAM CLASS II RIP RAP DISSIPATOR. ACTIVATE BYPASS PUMPING SYSTEM.

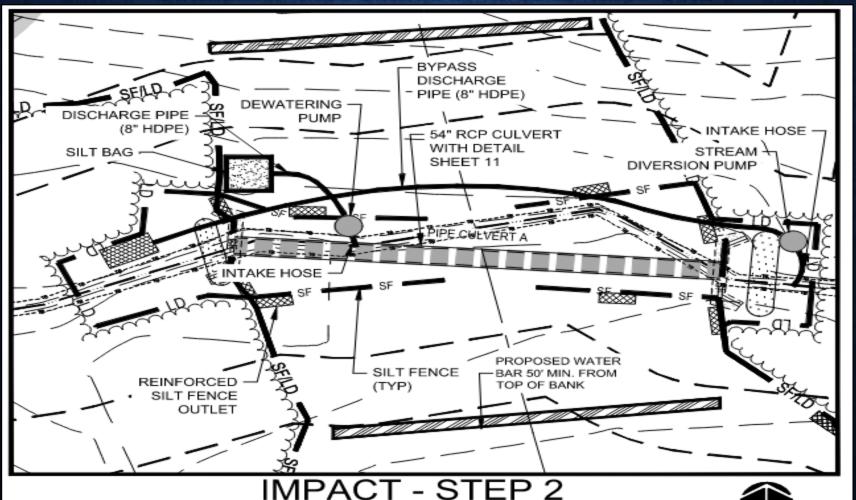


IMPACT - STEP 1





STREAM CROSSINGS – PUMP AROUND

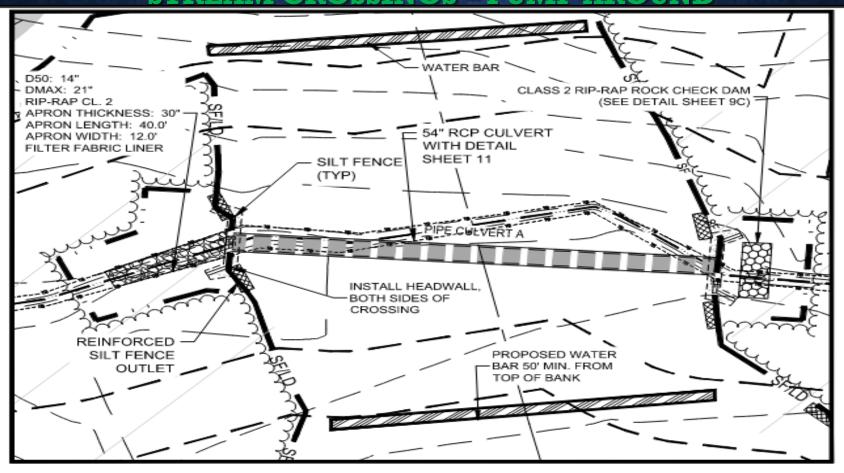


IMPACT - STEP 2





STREAM CROSSINGS – PUMP AROUND

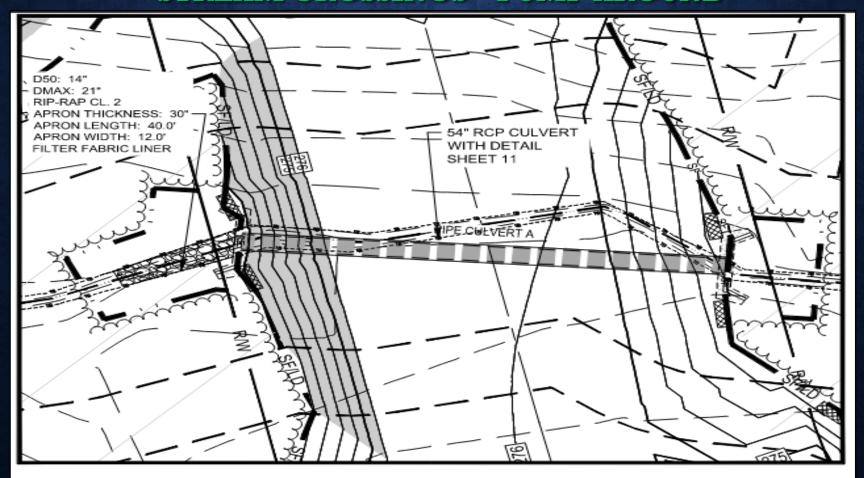


IMPACT - STEP 3





STREAM CROSSINGS – PUMP AROUND

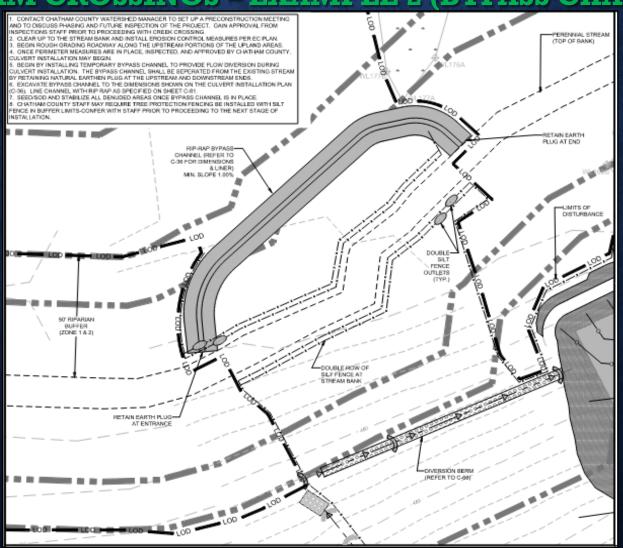


IMPACT - STEP 4



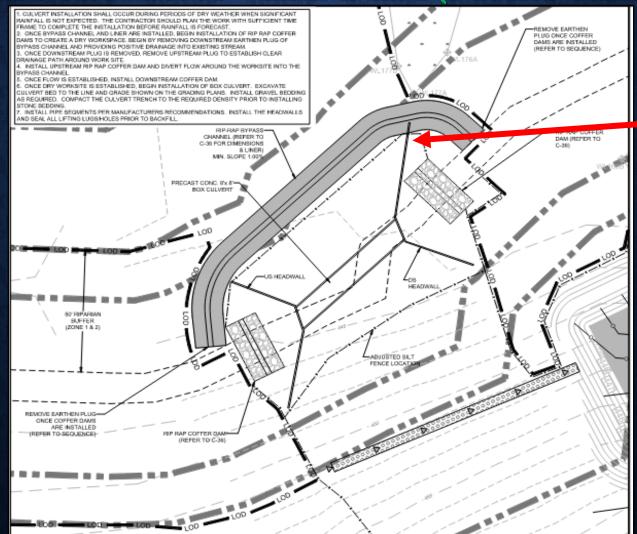


STREAM CROSSINGS – EXAMPLE 2 (BYPASS CHANNEL)





STREAM CROSSINGS - EXAMPLE 2 (BYPASS CHANNEL)







CONSTRUCTION SEQUENCE

Detailed

- > Instructs what to do and when
- > Presented in stages
- Lists required inspections
- ➤ Instructs to clear only as necessary to install initial measures

EROSION CONTROL SEQUENCE - STAGE 1

- OBTAIN GRADING PERMIT / FINAL APPROVAL FROM CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT.
- CONTACT THE CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT TO SET UP A PRE-CONSTRUCTION MEETING PRIOR TO ANY LAND DISTURBANCE WORK PERFORMED.
- THE CONTRACTOR SHALL KEEP A COPY OF THE APPROVED EROSION CONTROL PLANS ON SITE AT ALL TIMES.
- DELINEATE THE PROPOSED TREELINE/DISTURBED LIMIT LINE AND INSTALL ALL PROPOSED SILT FENCE AND TREE
 PROTECTION FENCE THROUGHOUT THE ENTIRE PROJECT AREA ALONG WITH TEMPORARY CONSTRUCTION ENTRANCE.
 INSTALL CLEAN WATER DIVERSIONS / BERMS.
- INSTALL TEMPORARY SILT FENCE OUTLETS IN ALL LOCATIONS AS SHOWN ON THE PLANS. FIELD LOCATE SILT FENCE OUTLETS AT APPROPRIATE LOW SPOTS AT THE SHOWN LOCATIONS.
- ONCE THE PREVIOUS MEASURES ARE INSTALLED, CLEAR ONLY AS REQUIRED TO INSTALL TEMPORARY SKIMMER BASINS, TEMPORARY DIVERSION BERMS/ DITCHES, AND OTHER TEMPORARY MEASURES AS SHOWN ON THE APPROVED PLAN.
- INSTALL CHECK DAMS AND TEMPORARY MATTING IN TEMPORARY DIVERSION DITCHES.
- THE CONTRACTOR SHALL STABILIZE ALL DIVERSIONS, AND RISER/SKIMMER BASINS IMMEDIATELY UPON THEIR CONSTRUCTION.
- 9. ALL MEASURES SHALL BE INSTALLED AND INSPECTED FOR COMPLIANCE BY CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT PRIOR TO COMMENCEMENT OF ANY PROPOSED R.O.W. CLEARING/GRUBBING AND EXCAVATION. CALL FOR AI INSPECTION FOR COMPLIANCE PRIOR TO ANY ADDITIONAL CLEARING. IF SITE IS APPROVED, PROCEED TO STAGE 2.

*CONTRACTOR TO REESTABLISH TEMPORARY DIVERSION SWALES AT THE END OF EACH DAY TO ENSURE DRAINAGE TO THE APPROPRIATE STRUCTURE.



CONSTRUCTION SEQUENCE

Thoughtful

- Explains transition between stages
- ➤ Includes total scope of work
- > Clarifies any gray areas
- > Define how long sediment basins remain
- > Clearly state when conversion can occur
- > Steps to close plan/permit

CONSTRUCTION SEQUENCE SCHEDULE

Definition A specified work schedule that coordinates the timing of land-disturbing activities and the installation of erosion and sedimentation control measures.

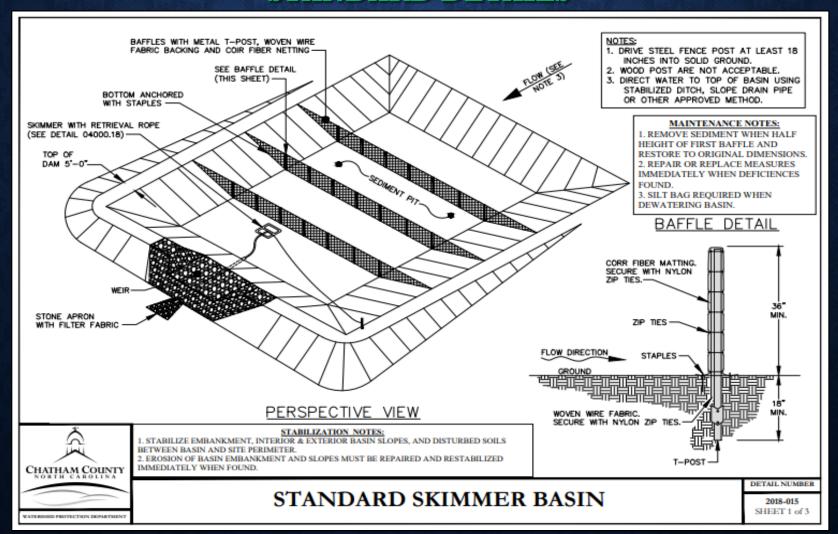
EROSION CONTROL SEQUENCE - STAGE 2

- ONCE APPROVAL FOR PHASE 1 IS OBTAINED BEGIN CLEARING AND GRUBBING OF REMAINING DISTURBED AREAS AS SHOWN ON APPROVED PLAN
- SEE CULVERT #3 INSTALLATION SEQUENCE ON DETAIL SHEET 9F FOR STREAM CROSSING.
- BEGIN INSTALLATION OF NEW STORM DRAINAGE NETWORKS AND SITE UTILITIES AS SITE IS BEING BROUGHT UP TO FINAL GRADE
- WHEN EXCAVATING FOR INSTALLATION OF STORM DRAINAGE NETWORKS, INSTALL FOREBAYS AT TEMPORARY RISER BASINS. SEE POND DETAIL SHEETS 6A-6D FOR DETAILS ON FOREBAYS.
- CONSTRUCTION RUNOFF IN THE CULVERT F / STORM NETWORK G AREA TO BE DIRECTED TO TSB #3 AND TSB #4 FROM STAGE 1 DESIGN UNTIL CULVERT F IS FUNCTIONING AS CLEAN WATER BYPASS PIPE AND STAGE 2 RUNOFF DIRECTED TO TSB #4 IN STAGE 2 CONDITION.
- ONCE THE PIPES FOR STORM SYSTEMS ARE COMPLETE, INSTALL PERMANENT DIVERSION DITCHES AND OTHER TEMPORARY MEASURES INSTALL CHECK DAMS IN EACH AND STABILIZE WITH PERMANENT MATTING IMMEDIATELY.
- COMPLETE THE INSTALLATION OF THE STORM DRAINAGE NETWORK AS THE SITE COMES TO FINAL GRADE. PLACE INLET PROTECTION AND HORSEHOE INLET PROTECTION AROUND STORM DRAINAGE STRUCTURES ONCE THEY ARE COMPLETED.
- THE TEMPORARY SKINWER BASINS INTERIOR TO THE SITE ARE TO BE REMOVED AND EVENTUALLY FILLED TO FINAL OR ORIGINAL EXISTING GRADE. CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT MUST INSPECT SITE CONDITIONS PRIOR TO REMOVAL OF TEMPORARY SKINMER BASINS.
- BRING SITE TO FINAL GRADE AND BEGIN EXCAVATION OF ROADWAYS.
- CONSTRUCT CURB AND GUTTER AND STONE BASE TO PREP FOR PAVING. INSTALL SIDEWALKS. SEED/SOD AND INSTALL LANDSCAPING ALONG WITH ANY OTHER PERMANENT GROUND COVER.
- SEED AND MULCH ALL DENUDED AREAS WITHIN 15 WORKING DAYS OF COMPLETION OF ANY PHASE OF CONSTRUCTION, EROSION CONTROL MATTING IS TO BE INSTALLED ON SLOPES 3:1 OR GREATER IMMEDIATELY AFTER SEEDING.
- PAVE ROADS.
- REQUEST FINAL APPROVAL BY CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT. IF THE INSPECTOR IS SATISFIED WITH UPSTREAM PERMANENT GROUND COVER. BEGIN REMOVING TEMPORARY EROSION CONTROL MEASURES.
- 14. AFTER STABILIZATION HAS OCCURRED REMOVE SEDIMENT FROM RISER BASINIWATER QUALITY POND AND CONVERT TO PERMANENT STRUCTURE, CONTACT CHATHAM COUNTY WATERSHED PROTECTION DEPARTMENT FOR INSPECTION PRIOR TO BASIN CONVERSION, ALL WATER IS TO BE PUMPED TO SILT BAG BEFORE DISCHARGING FROM SITE (SEE DETAIL SHEET 9C), CONSTRUCTION RUNOFF FROM THE CONSERVANCY PHASE 18 WILL BE DIRECTED TO TRB #4. DO NOT COMPLETE FINAL CONVERSION UNTIL THAT AREA IS GRADED TO FINAL CONDITION AND STABILIZED.
- REQUEST FINAL INSPECTION BY ENVIRONMENTAL INSPECTOR TO CLOSE OUR LAND-DISTURBING PERMIT

"CONTRACTOR TO REESTABLISH TEMPORARY DIVERSION SWALES AT THE END OF EACH DAY TO ENSURE DRAINAGE TO THE APPROPRIATE STRUCTURE.



STANDARD DETAILS





STANDARD DETAILS

nting the details and specifications on this plan sheet will result in the construc Ivity being considered compliant with the Ground Stabilization and Materials Handling tions of the NCGO1 Construction General Permit (Sections E and F, respectively). The mittee shall comply with the Erosion and Sediment Control plan approved by the

SECTION E: GROUND STABILIZATION

Required Ground Stabilization Timeframes								
Si	te Area Description	Stabilize within this many calendar days after ceasing land disturbance	Timeframe variations					
(a)	Perimeter dikes, swales, ditches, and perimeter slopes	7	None					
(b)	High Quality Water (HQW) Zones	7	None					
(c)	Slopes steeper than 3:1	7	If slopes are 10' or less in length and are not steeper than 2:1, 14 days are allowed					
(d)	Slopes 3:1 to 4:1	14	-7 days for alopes greater than 50° in length and with slopes steeper than 4:1 -7 days for perimeter diles, swales, ditches, perimeter slopes and HQW Zones -10 days for Falls Lake Watershed					
(e)	Areas with slopes flatter than 4:1	14	-7 days for perimeter dikes, swales, ditches, perimeter slopes and HQW Zones -10 days for Falls Lake Watershed unless					

racticable but in no case longer than 90 calendar days after the last land disturbing ctivity. Temporary ground stabilization shall be maintained in a manner to render the surface stable against accelerated erosion until permanent ground stabilization is achieved.

GROUND STABILIZATION SPECIFICATION

abilize the ground sufficiently so that rain will not dislodge the soil. Use one of the

- other malches and tackifiers

- without temporary grass seed
- · Hydroseeding
 - . Uniform and evenly distributed ground cover . Structural methods such as concrete, asphalt or

other mulches and tackifiers

- Select flocculants that are appropriate for the soils being exposed during onstruction, selecting from the NC DWR List of Approved PAMS/Flocculants
- Apply flocculants at or before the inlets to Erosion and Sediment Control Measure PAMS/Flocculants and in accordance with the manufacturer's instructions

EQUIPMENT AND VEHICLE MAINTENANCE

- Maintain vehicles and equipment to prevent discharge of fluids.
- Provide drip pans under any stored equipment.
- Identify leaks and repair as soon as feasible, or remove leaking equipment from the
- 4. Collect all spent fluids, store in separate containers and properly dispose a hazardous waste (recycle when possible)
- Remove leaking vehicles and construction equipment from service until the proble
- 6. Bring used fuels, lubricants, coolants, hydraulic fluids and other petroleum product

LITTER, BUILDING MATERIAL AND LAND CLEARING WASTE

- Provide a sufficient number and size of waste containers (e.g dumpster, trash recentacle) on site to contain construction and domestic waster
- Locate waste containers at least 50 feet away from storm drain inlets and surface waters unless no other alternatives are reasonably available.
- Locate waste containers on areas that do not receive substantial amounts of runo from upland areas and does not drain directly to a storm drain, stream or wetland Cover waste containers at the end of each workday and before storm events or
- provide secondary containment. Repair or replace damaged waste containers. Anchor all lightweight items in waste containers during times of high winds. Empty waste containers as needed to prevent overflow. Clean up immediately i
- Dispose waste off-site at an approved disposal facility.

- PAINT AND OTHER LIQUID WASTE er liquid waste into storm drains, streams or wetlands Locate paint washouts at least 50 feet away from storm drain inlets and surface
- Contain liquid wastes in a controlled area.
- Prevent the discharge of soaps, solvents, detergents and other liquid wastes from

PORTABLE TOILETS

- Install portable toilets on level ground, at least 50 feet away from storm drain: streams or wetlands unless there is no alternative reasonably available. If 50 foo offset is not attainable, provide relocation of portable toilet behind silt fence or place
- Provide staking or anchoring of portable toilets during periods of high winds or in high
- Monitor portable toilets for leaking and properly dispose of any leaked material Utilize a licensed sanitary waste hauler to remove leaking portable toilets and replace

EARTHEN STOCKPILE MANAGEMEN

- Show stockpile locations on plans. Locate earthen-material stockpile areas at least 50 feet away from storm drain inlets, sediment basins, perimeter sediment control
- Provide stable stone access point when feasible
- Stabilize stockpile within the timeframes provided on this sheet and in accorda with the approved plan and any additional requirements. Soil stabilization is define s vegetative, physical or chemical coverage techniques that will restrain accelerated



CONCRETE WASHOUTS

- Do not discharge concrete or cement slurry from the site Dispose of, or recycle settled, hardened concrete residue in accordance with local
- and state solid waste regulations and at an approved facility. Manage washout from mortar mixers in accordance with the above item and in
- addition place the mixer and associated materials on impervious barrier and with lot perimeter silt fence.
- Install temporary concrete washouts per local requirements, where applicable. If ar alternate method or product is to be used, contact your approval authority for review and approval. If local standard details are not available, use one of the two types of temporary concrete washouts provided on this detail.
- Do not use concrete washouts for dewatering or storing defective curb or sidewall sections. Stormwater accumulated within the washout may not be numbed into o discharged to the storm drain system or receiving surface waters. Liquid waste m be pumped out and removed from project.
- Locate washouts at least 50 feet from storm drain inlets and surface waters unless install protection of storm drain inlet(s) closest to the washout which could receive
- Locate washouts in an easily accessible area, on level ground and install a stone entrance pad in front of the washout. Additional controls may be required by the
- Install at least one sign directing concrete trucks to the washout within the project limits. Post signage on the washout itself to identify this location.
- Remove leavings from the washout when at approximately 75% capacity to limit overflow events. Replace the tarp, sand bags or other temporary structural
- products, follow manufacturer's instructions. 10. At the completion of the concrete work, remove remaining leavings and disnose of in an approved disposal facility. Fill pit, if applicable, and stabilize any disturbance caused by removal of washout.

ERBICIDES, PESTICIDES AND RODENTICIDES

- Store and apply herbicides, pesticides and rodenticides in accordance with label
- Store herbicides, pesticides and rodenticides in their original containers with the label, which lists directions for use, ingredients and first aid steps in case of
- Do not store herbicides, pesticides and rodenticides in areas where flooding is possible or where they may spill or leak into wells, stormwater drains, group or surface water. If a spill occurs, clean area immediately. Do not stocknile these materials onsite

- . Create designated hazardous waste collection areas on-site
- Place hazardous waste containers under cover or in secondary of
- . Do not store hazardous chemicals, drums or bagged materials directly on the groun

SELF-INSPECTION, RECORDISERPING AND REPORTING

elf-inspections are required during normal business hours in accordance with the table

below. When adverse weather or site conditions would cause the salety of the inspection personnel to be in jeopardy, the impaction may be delayed until the next business day on which it is sale to perform the impaction. In addition, when a storm execut of equal to or greater than 1.0 inch occurs outside of normal business hours, the self-inspection shall be performed upon the commencement of the next business day. Any time when inspection

inspect	Prequency (during normal (sustance hours)	Inspection recents must include:					
[1] Rain gauph maintained in good working ender	Dully	Delig months anounce. If no delig may appear be resident or benefits person, and no listely excellent or benefits person, and no listely excellent experience, and no listely excellent experience or excellent excellent or consultant excellent exce					
(2) EROC Measures	At least once per 7 calendar days and within 24 house of a sale event 5 LO ledt in 24 house	Item and time of the requested, Date and time of the inspection, tells and time of the inspection, tells and time of the person performing the inspection, tells and time of whether the sensures were appearing properly. Description of statistic beaco electric for the research, Description, existence, and date of correction actions taken.					
(S) Stormwater discharge suctable (S9Cs)	At least once per 7 calendar days and within 24 fours of a said event g LO inch in 24 fours	Memofination of the discharge outfalls inspected. Date and fisce of the inspection. Marke of the partner performing the inspection. Marke of the partner performing the inspection, cuch as oil shores. For indicators of convention pollutions cuch as oil shores. Fourtners per perspected office or disclosed-sides. Molaration of Visible sealment leaving this site, 6. Description, envisions, and seal entire personal country.					
4 Perimeter ef sibn	At least once per 7 calendar days and within 24 hours of a tale event a LO leath in 26 hours	If visible settinents from to found outside site limits, then a necessi- of the following shall be made: 1. Actions solden no learn up or ratefilize the sediment that has left the disclaims, 2. Description, evidence, and date of correctine actions taken, and 3. An explanation as to the actions taken to control future releases.					
(N) Sáreame ar wetlands posite or offsite	At least arms per 7 calendar days and within 24	If the steems or welfood has increased stable sedimentation or a stream has visible increased subjectly from the construction activity, then a resent of the following shall be made:					

Decorption, redevice and dark of correction actions taken, not forwards of the conglisted reports to the appropriate (Delaise Reports) of the conglisted reports to the appropriate (Delaise Regional Office per Part 11, Section C. Bern (1)(b) of this power. The phase of grading finestablished of perinters (ASC eventures, bearing and problem, broadlation of storm drainings tacklists, completion of all land-distanting satisfy, control section or redevelopment, permanent.

In addition to the EBSC plan documents above, the following items shall be kept on the site and available for inspectors at all times during normal business hours, unless the Division provides a site-specific exemption based on unique site conditions that make

The approved E&SC plan as well as any approved deviation shall be kept on the site. The

approved E&SC plan must be kept up to date throughout the coverage under this permit

itial and date each E&SC measure on a cop-

initial installation of the FRSC measures or if

plan or complete, date and sign on inspection

round cover specifications.

corrective action

eport to indicate compliance with approved

mplete, date and sign an inspection report

Initial and date a copy of the approved E&SC

the EBSC measures are modified after initial

he following items pertaining to the E&SC plan shall be kept on site and available for

(a) Lack DBA. Healine has been instructed and design of grapheroff device from the locations, dimensions and relative elevations shown on the agenoved 685C plan.

This documentation is required quent the properties of the properties of the properties of 685C plan. This documentation is required quent the

(b) A phase of grading has been completed. Initial and date a copy of the approved E&SC

inspection at all times during normal business hours.

(a) Each EBSC measure has been installed

in accordance with the approved E850

requirements for all EBSC measures.

(a) Corrective actions have been taken

- (a) This General Permit as well as the Certificate of Coverage, after it is received.
- record the required observations on the Inspection Record Form provided by the Division or a similar inspection form that includes all the required elements. Use of shown to provide equal access and utility as the hard-copy records.

3. Documentation to be Retained for Three Years

- All data used to complete the e-NOI and all inspection records shall be maintained for a period of three years after project completion and made available upon request. [40 CFR 122.41]
- DRAW DOWN OF SEDIMENT BASINS FOR MAINTENANCE OR CLOSE OUT diment basins and traps that receive rusoff from drainage areas of one acre or more shall use outlet structures that withdraw water from the surface when these devices need to be drawn do for maintenance or close out unless this is infeasible. The circumstances in which it is not feasible to withdraw water from the surface shall be rare (for example, times with extended cold weather Non-surface withdrawals from sediment basins shall be allowed only when all of the following criteria have been met:
- (a) The EBSC plan authority has been growided with documentation of the non-surface withdrawal and the specific time periods or conditions in which it will occur. The non-surface withdrawal
- shall not commence until the EBSC plan authority has approved these items, (b) The non-surface withdrawal has been reported as an articipated bypass in accordance with Part III, Section C, Item (2)(c) and (d) of this penni
- b) Dewatering discharges are treated with controls to minimize discharges of poliutants from stormwater that is removed from the sediment basin. Examples of appropriate controls include
- properly sited, designed and maintained dewatering tanks, weir tanks, and filtration systems,
- (d) Vegetated, upland areas of the sites or a properly designed stone pad is used to the extent feasible at the outlet of the dewatering treatment devices described in item (c) above,
- le) Velocity dissipation devices such as check dams, sediment traps, and riprap are provided at the discharge points of all dewatering devices, and
- 5 Sediment removed from the devatering treatment devices described in Item (c) above is disposed of in a manner that does not cause deposition of sediment into waters of the United State

SECTION C: REPORTING

Permittees shall report the following occurrences Visible sediment deposition in a stream or wetland

- . They are 25 gallons or more.
- . They are less than 25 gallors but cannot be cleaned up within 24 hours.
- . They cause sheen on surface waters (regardless of volume), or
- . They are within 100 feet of surface waters (regardless of volume).
- of the Clean Water Act (Ref. 40 CFR 110.3 and 40 CFR 117.3) or Section 102 of CERCLA
- |d| Anticipated bypasses and unanticipated bypasses
- (e) Noncompliance with the conditions of this permit that may endanger health or the

. Reporting Timeframes and Other Requirements

After a permittee becomes aware of an occurrence that must be reported, he shall contact other requirements listed below. Occurrences outside normal business hours may also be sported to the Department's Environmental Emergency Center personnel at (800)

(b) Oil spills and

benith or the

- Within 24 Agurs, an aral or electronic notification . Within 7 coloneler days, a report that contains a description of the sediment and actions taken to address the cause of the deposition Division staff may wrive the requirement for a written report on a
- case-by-case basis. If the stream is samed on the <u>SC.0.000 kg as</u> impaired for sediment-related causes, the permittee may be required to perform additional anothoring, inspections or apply more stringent practices if staff determine that additional explanements are needed to assure complaints
- with the federal or state impaired-waters conditions Witten 24 hours, an anal or electronic ation about the date, time, nature, volume are
- The report shall include an evaluation of the anticipated quality and

- Within 24 Agent, an oral or electronic notification. with the condition Within 7 celevaler days, a report that contains a description of the of this permit that
 - secongularios, and riscouser to opioido or noncompliance, including exact dates and times, and if the maneeriplance has not seen corrected, the unfolgated time noncompliance is expected to concentrate and the properties of planned to reduce, eliminate, and review times to the concompliance (40 CPR 322.41(8)6). Division staff may waive the requirement for a written report on a
 - case by case basis.



NCG01 SELF-INSPECTION, RECORD KEEPING AND REPORTING

EFFECTIVE: 04/01/19

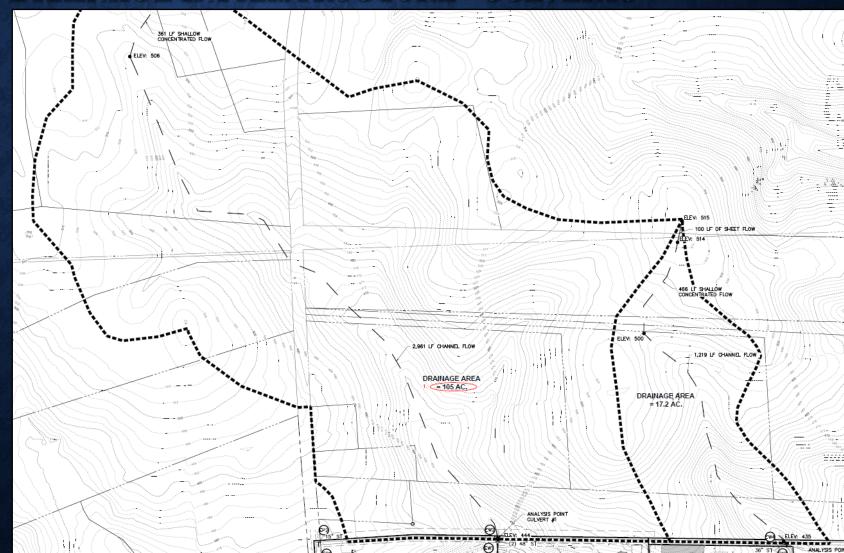
NCG01 GROUND STABILIZATION AND MATERIALS HANDLING

EFFECTIVE: 04/01/19



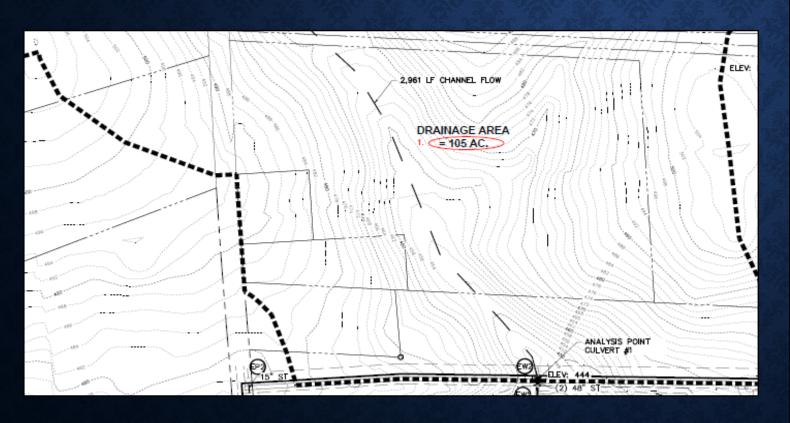
STORM DRAINAGE INFRASTRUCTURE - CULVERTS

- > Verify design
- > Verify inputs
- Check for overtopping



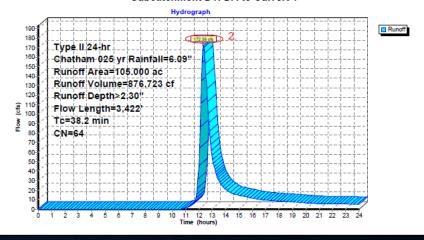


STORM DRAINAGE INFRASTRUCTURE - CULVERTS



Summary for Subcatchment D1: DA to Culvert 1 2. 172.38 cfs @ 12.36 hrs, Volume= 876,723 cf, Depth> 2.30" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type II 24-hr Chatham 025 yr Rainfall=6.09 2. Area (ac) CN Description Woods, Fair, HSG B Roofs, HSG B 69 Pasture/grassland/range, Fair, HSG B 105,000 64 Weighted Average 98.10% Pervious Area 103.000 2.000 1.90% Impervious Area Tc Length Slope Velocity Capacity Description (feet) (ft/sec) 0.0100 Sheet Flow, Sheet Flow Woods: Light underbrush n= 0.400 P2= 3.60" Shallow Concentrated Flow, Shallow Concentrated Flow Woodland Kv= 5.0 fps Channel Flow, Channel Flow 2.961 0.0209 Area= 10.0 sf Perim= 9.0' r= 1.11' n= 0.025 Earth, clean & winding 38.2 3,422 Total

Subcatchment D1: DA to Culvert 1





STORM DRAINAGE INFRASTRUCTURE - CULVERTS

= 172.38 = 172.38 = (dc+D)/2

2 = 172.38 = 172.38 = 0.00 = 7.56 = 9.13 = 445.91 = 447.38 4.

= 1.07

= Inlet Control

Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Wednesday, Feb 18 2022

Calculations

Hw/D (ft)

Flow Regime

Culvert 1 - 25 Year

Invert Elev Dn (ft)

Top Elevation (ft)

Top Width (ft)

Crest Width (ft)

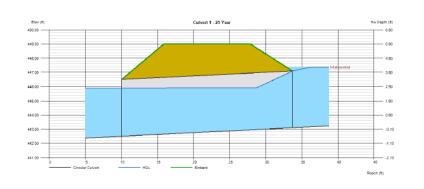
Pipe Length (ft)	= 23.70	Qmin (cfs)
Slope (%)	= 2.53	Qmax (cfs)
nvert Elev Up (ft)	= 443.10	Tailwater Elev (ft)
Rise (in)	= 48.0	
Shape	= Circular 3	Highlighted
Span (in)	=48.0 2	Qtotal (cfs)
Vo. Barrels	₹2) 3.	Qpipe (cfs)
ı-Value	= 0.013 3.	Qovertop (cfs)
Culvert Type	= Circular Concrete	Veloc Dn (ft/s)
Culvert Entrance	 Groove end w/headwall (C) 	Veloc Up (ft/s)
Coeff. K,M,c,Y,k	= 0.0018, 2, 0.0292, 0.74, 0.2	HGL Dn (ft)
		HGL Up (ft)
mbankment	4	Hw Elev (ft)

= 442.50

=449.00 4.

= 12.00

= 20.00



Summary for Subcatchment D1: DA to Culvert 1

Runoff = 2. 172.38 cfs@ 12.36 hrs, Volume= 876,723 cf, Depth> 2.30"

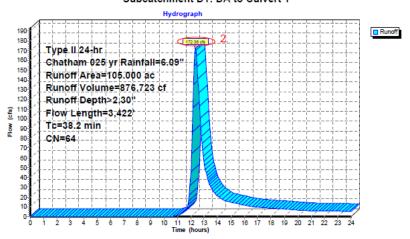
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Type II 24-hr Chatham 025 yr Rainfall=6.09 2.									
Area	(ac) C	N Des	cription						
67.000 60 Woods, Fair, HSG B									
2.	.000 9		fs, HSG B						
36.	.000 6	9 Past	ture/grassl	and/range,	Fair, HSIG B				
1 (105.	.000 6	34 Wei	ghted Aver	rage					
103	.000		0% Pervio						
2.	.000	1.90	% Impervi	ous Area					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·				
26.7	100	0.0100	0.06		Sheet Flow, Sheet Flow				
					Woods: Light underbrush n= 0.400 P2= 3.60"				
6.1	361	0.0388	0.98		Shallow Concentrated Flow, Shallow Concentrated Flow				
					Woodland Kv= 5.0 fps				
5.4	2,961	0.0209	9.22	92.18	Channel Flow, Channel Flow				
	-				Area= 10.0 sf Perim= 9.0' r= 1.11'				

38.2 3.422 Total

Subcatchment D1: DA to Culvert 1

n= 0.025 Earth, clean & winding





STORM DRAINAGE INFRASTRUCTURE – INLET REPORTS

System

Inlet

Q25-YEAR Report

Drain

Runoff

Coeff

Total

Inlet

Time

Stormwater Studio 2024 v 3.0.0.35

Inlet

Line

No.

Project Name: STORM SYSTEM B

Grnd/Rim

Elev Up

Invert

Invert

11-11-2024

	.5	71.00	71.00	000	0		0,000		~	rtanon		7110	0.20	Longin	Ciopo	"			1
		(ac)	(ac)	(C)		(min)	(min)	(in/hr)	(cfs)	(cfs)	(cfs)	(ft/s)	(in)	(ft)	(%)	(ft)	(ft)	(ft)	
1	B1	4.580	6.930	0.60	4.16	5.0	5.3	7.95	21.85	32.60	46.66	6.64	30	197.00	1.29	290.55	288.00	295.60	
2	B2	2.350	2.350	0.60	1.41	5.0	5.0	7.95	11.21	11.21	16.41	3.57	24	104.48	0.53	291.60	291.05	296.05	
3	E1	0.900	1.640	0.60	0.98	5.0	5.3	7.95	4.29	7.72	16.28	4.44	18	52.00	2.40	272.75	271.50	276.80	
4	E2	0.740	0.740	0.60	0.44	5.0	5.0	7.95	3.53	3.53	7.50	2.21	18	72.20	0.51	273.32	272.95	277.05	
5	C1	2.980	5.870	0.60	3.52	5.0	5.2	7.95	14.22	27.73	71.69	6.00	30	36.00	3.06	270.60	269.50	275.40	
6	C2	2.890	2.890	0.60	1.73	5.0	5.0	7.95	13.79	13.79	16.41	4.45	24	76.00	0.53	271.50	271.10	275.40	
7	F1	9.010	9.010	0.35	3.15	5.0	5.0	7.95	25.08	25.08	49.73	6.14	30	204.00	1.47	279.00	276.00	286.65	
8	G1	1.200	5.570	0.60	3.34	5.0	5.5	7.95	5.73	26.01	31.15	8.53	24	58.00	1.90	275.10	274.00	283.45	
9	G2	2.940	3.620	0.60	2.17	5.0	5.3	7.95	14.03	17.04	14.89	9.64	18	102.00	2.01	277.75	275.70	283.45	
10	G3	0.680	0.680	0.60	0.41	5.0	5.0	7.95	3.24	3.24	4.58	2.81	15	72.00	0.50	282.86	282.50	284.94	
11	G4	0.750	0.750	0.60	0.45	5.0	5.0	7.95	3.58	3.58	6.41	2.11	18	72.00	0.37	281.72	281.45	284.51	Г
12	11	3.360	18.960	0.60	8.47	5.0	5.5	7.95	16.03	65.97	136.02	17.22	30	50.00	11.00	273.00	267.50	277.34	
13	CB 12	0.130	14.630	0.60	5.87	5.0	5.3	7.95	0.62	46.03	41.53	9.38	30	78.00	1.03	274.30	273.50	279.57	
14	13	5.320	14.500	0.45	5.80	5.0	5.3	7.95	19.04	45.49	65.95	9.27	30	29.00	2.59	275.25	274.50	280.50	
15	14	5.940	5.940	0.30	1.78	5.0	5.0	7.95	14.17	14.17	22.91	4.51	24	38.00	1.03	276.14	275.75	280.01	
16	CB J1	0.040	2.380	0.60	1.43	5.0	5.1	7.95	0.19	11.29	48.34	8.25	24	81.00	4.57	271.20	267.50	278.84	
17	CB J2	0.040	2.340	0.60	1.40	5.0	5.1	7.95	0.19	11.13	23.08	5.99	24	24.00	1.04	271.65	271.40	278.84	
18	FES J3	2.300	2.300	0.60	1.38	5.0	5.0	7.95	10.97	10.97	15.62	7.67	18	38.43	2.21	273.00	272.15	277.93	
19	K1	2.340	5.870	0.60	3.52	5.0	5.3	7.95	11.16	27.66	66.43	12.60	24	51.00	8.63	277.40	273.00	282.10	Г
20	K2	3.530	3.530	0.60	2.12	5.0	5.0	7.95	16.84	16.84	17.30	5.36	24	102.58	0.58	278.20	277.60	282.10	
21	L1	1.420	4.680	0.60	2.81	5.0	5.2	7.95	6.77	22.10	68.32	11.69	24	54.79	9.13	277.50	272.50	281.05	
22	L2	3.260	3.260	0.60	1.96	5.0	5.0	7.95	15.55	15.55	22.73	5.91	24	104.00	1.01	278.75	277.70	282.05	
23	GI M2	0.840	0.840	0.60	0.50	5.0	5.0	7.95	4.01	4.01	10.58	2.71	18	49.29	1.01	277.00	276.50	280.73	
24	GI M1	0.790	0.790	0.60	0.47	5.0	5.0	7.95	3.77	3.77	17.50	3.23	18	56.89	2.78	276.08	274.50	281.02	

Total

Runoff Full

Capac.

Vel

Ave

Line

Length

Line

Size

Line

Slope

Notes: IDF File = Chatham County.IDF, Return Period = 25-yrs.

Project File: 2024-11-08 Phase 1A SD.sws

Verify design

> Verify inputs

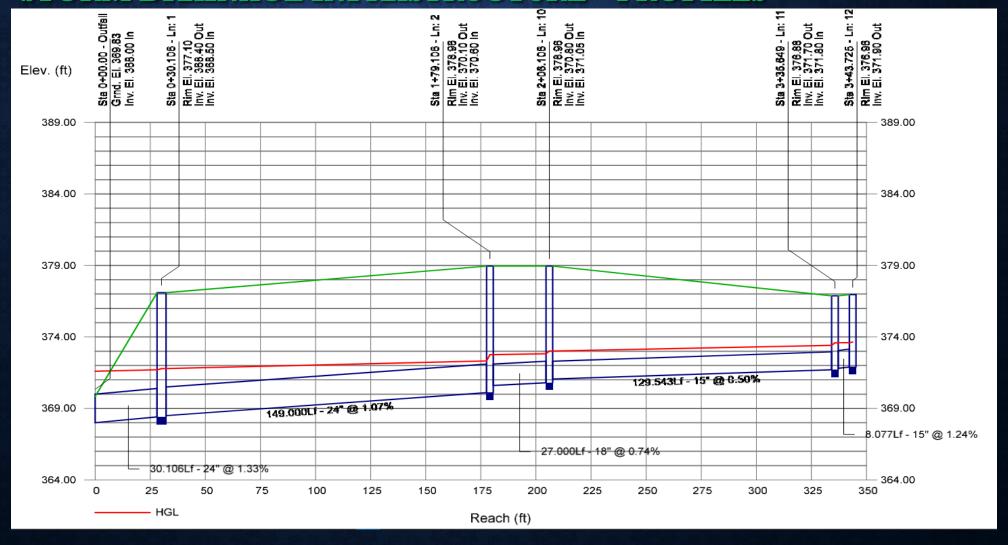
> Check for capacity



➤ Included for storm network

DESIGN STANDARDS

STORM DRAINAGE INFRASTRUCTURE – PROFILES

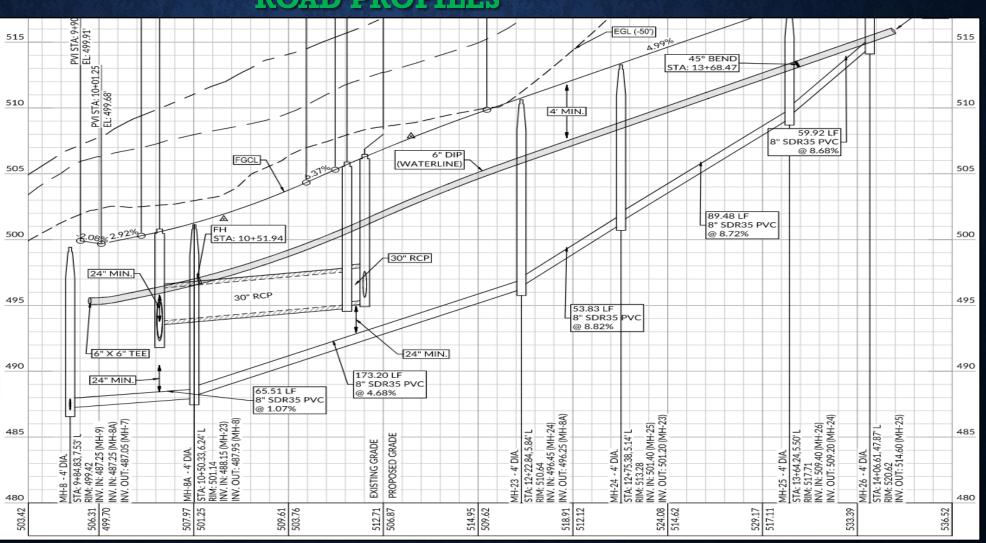




ROAD PROFILES



- Verify minimum cover
- Verify minimum separation





NCG01

Soil Stockpile Area

- 50 feet from surface waters, storm inlets, sediment basins, and ditches*
- 50 feet from site perimeter*
- Silt fence low sides offset 5 feet from toe of stockpile

Concrete Washout

- 50 feet from surface waters, storm inlets, sediment basins, and ditches*
- Protect nearby inlets
- Signage

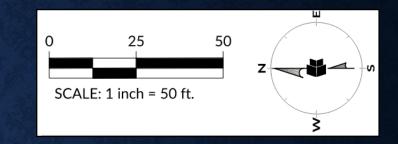
Staging, Parking, Fueling

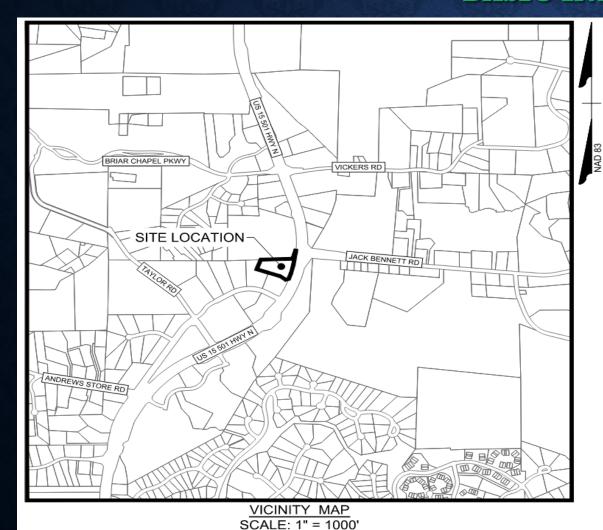
- 50 feet from surface waters, storm inlets, sediment basins, and ditches*
- Locate fueling areas

*Unless no other reasonable alternative exists. Denote on plan.



BASIC INFORMATION

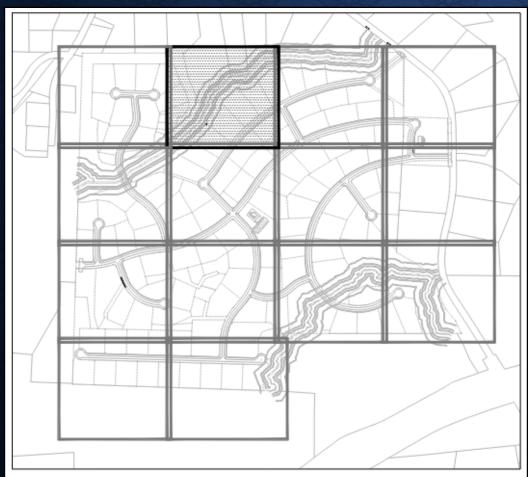




SITE DATA TABLE							
PROJECT NAME:	CHATHAM COUNTY EMS						
PROJECT ADDRESS:	9251 US 15 501 NORTH FEARRINGTON, NC 27312						
TOTAL ACREAGE:	3.03 AC						
DEED BOOK / PAGE:	BK: 460, PG: 0669						
PIN NUMBER:	977503415782						
ZONING:	R-1						
USE:	VACANT/WATER TOWER						
ONSITE IMPERVIOUS AREA EXISTING:	0.02± ACRES (1,003 SF)						
ONSITE IMPERVIOUS AREA PROPOSED:	0.36 ± ACRES (15,568 SF)						
TOTAL DISTURBED AREA:	1.56 ± ACRES (61,623 SF)						
RIVER BASIN:	CAPE FEAR						
STREAM:	HAW RIVER (JORDAN LAKE)						
SURFACE WATER CLASSIFICATION:	WS-IV NSW						



SHEET INDEX



SHEET INDEX

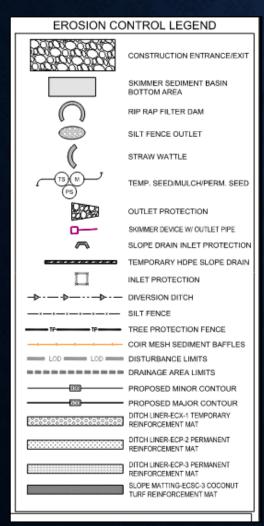
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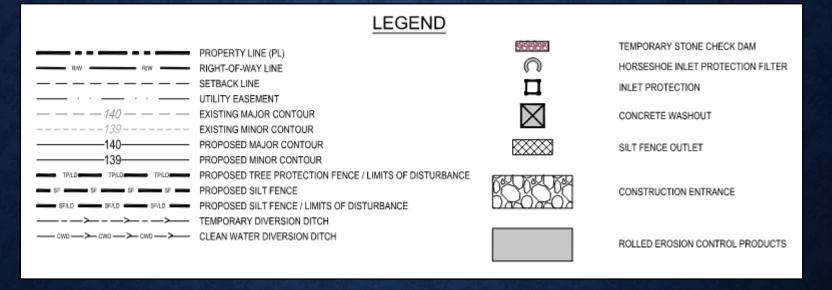
Sheet List Table

Sheet Number	Sheet Title				
C0.0	COVER SHEET				
C1.0	OVERALL EXISITNG CONDITIONS & DEMOLITION PLAN				
C1.1	EXISITNG CONDITIONS & DEMOLITION PLAN				
C2.0	EROSION CONTROL PLAN - PHASE 1				
C2.1	EROSION CONTROL PLAN - PHASE 2				
C2.2	EROSION CONTROL DETAILS				
C2.3	EROSION CONTROL DETAILS				
C2.4	EROSION CONTROL DETAILS				
C2.5	NCG01 GROUND STABILIZATION AND MATERIALS HANDLING				
C2.6	NCG01 SELF-INSPECTION, RECORDKEEPING AND REPORTING				
C3.0	SITE PLAN				
C3.1	ADJACENT INTERSECTION LOCATIONS				
C3.2	OVERALL US15-501 CROSS SECTION MAP				
C3.3	US15-501 CROSS SECTIONS				
C3.4	US15-501 CROSS SECTIONS				
C3.5	US15-501 CROSS SECTIONS				
C3.6	INTERSECTION SIGHT DISTANCE PROFILE				
C3.7	INTERSECTION SIGHT DISTANCE PROFILE				
C4.0	GRADING AND STORM DRAINAGE PLAN				
C4.1	BIORETENTION PLAN & PROFILE				
C4.3	PRE-DEVELOPMENT DRAINAGE AREA MAP				
C4.4	POST-DEVELOPMENT DRAINAGE AREA MAP				
C5.0	UTILITY PLAN				
C6.0	NOTES AND DETAILS				
C6.1	NOTES AND DETAILS				
C6.2	NOTES AND DETAILS				
C6.3	NOTES & DETAILS				
C6.4	NOTES AND DETAILS				
C6.5	NOTES AND DETAILS				
C6.6	NOTES AND DETAILS				
L1.0	OVERALL LANDSCAPE PLAN				
L1.1	LANDSCAPE DETAIL PLAN				



LEGEND





- Contains all erosion control measures
- Between legend and callouts, everything accounted for
- All applicable sheets have own legends (grading, utility, etc.)



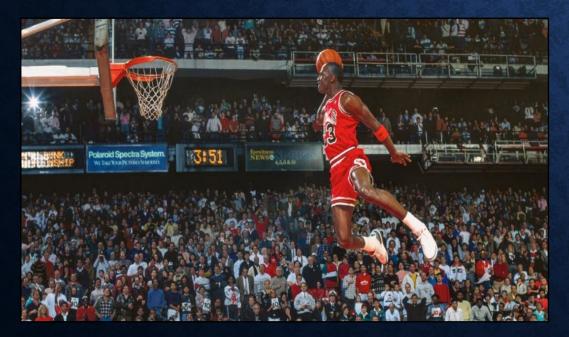
PLAN REVIEW

CONCLUSION

A good plan can be the difference between environmental compliance and environmental damage.

Make your plan work for you so your planned measures:

Perform like this:



And not like this:





THANK YOU FOR YOUR ATTENTION!

Please remember to complete the end of workshop evaluation:

https://bit.ly/2025EscEval