Little Alamance Creek Stream Restoration, Alamance County State Construction Office Project Number: D07050S



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Prepared For:

North Carolina Department of Environment and Natural Resources Ecosystem Enhancement Program 1652 Mail Service Center Raleigh, North Carolina 27699-1652



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Executive Summary

1. Project Goals and Objectives

The primary goals of this stream restoration project focus on improving water quality, enhancing flood attenuation, and restoring aquatic habitat and will be accomplished by:

- Reducing non-point sources of pollution associated with historic lawn maintenance in the park area by providing a vegetative buffer adjacent to Little Alamance Creek and its unnamed tributary and the installation of stormwater best management practices to treat surface runoff. The riparian buffer will remain in a State-owned conservation easement in perpetuity.
- Reducing sedimentation on-site and in downstream receiving waters through a reduction of bank erosion associated with current vegetation maintenance practices and through providing a forested vegetative buffer adjacent to Little Alamance Creek and its tributary.
- Reestablishing stream stability and the capacity to transport watershed flows and sediment loads by restoring stable dimension, pattern, and profile.
- Promoting floodwater attenuation through increased flood storage capacity by construction of bankfull benches along Little Alamance Creek and its tributary.
- Improving aquatic habitat by enhancing stream bed variability.

The EEP is currently in Phase II of developing a Local Watershed Plan (LWP) in the Little Alamance, Travis, and Tickle Creek watersheds with interested stakeholders. The LWP goals include both short- and long-term strategies to restore, manage, and protect vital functions in the watershed. The Little Alamance stream project was identified through the LWP process and the projects meet three of the six planning goals, which include:

- a) Increase local government awareness of the impacts of urban growth on water resources;
- b) Improve water quality through stormwater management;
- c) Identify and rank parcels for retrofits, stream repair, preservation, and/or conservation.

2. Existing Amount of Streams and Wetlands

The existing length of Little Alamance Creek within City Park is approximately 2,636 linear feet. The existing length of the Unnamed Tributary is approximately 422 linear feet. These distances were measured along the streams' thalwegs within the project limits. There are no wetlands located on the project site.

Bare root seedlings of tree and shrub species will be planted within the buffer at a density up to 555 stems per acre (10-foot centers). Appropriate species on live stakes will be installed within the bankfull

channel. Planting will be performed between December and March to allow plants to stabilize during the dormant period and set roots during the spring.

Existing non-native exotics within the proposed buffer will be removed during construction. Exotic shrubs will be removed with construction equipment or cut and the stumps treated with an appropriate herbicide.

Little Alamance Creek and its unnamed tributary are located in City Park and are easily accessible by the public. This setting can provide an excellent opportunity for environmental education promoting the importance of water quality, riparian buffers, terrestrial and aquatic habitats, and reduction of point and non-point source pollution. There is also the opportunity to have students from Walter M. Williams High School involved in future project monitoring. The school is located just northeast of City Park.

3. Amount of Streams and Wetlands Designed

The stream restoration will reduce the total stream length of Little Alamance Creek by approximately 3 feet, to a length of 2,633 linear feet. The reduction is the result of increasing the radius of curvature of one tight bend near the ball field. The length of the unnamed tributary will not change. Jurisdictional wetlands are not proposed as part of this mitigation design.

1.0 Project Site Identification and Location

The project is located in City Park in the City of Burlington, Alamance County, North Carolina (Figure 1).

Directions to Project Site

From Interstate 40/85, take North Carolina Highway 62 (NC 62)/Alamance Road north (exit 143). Go approximately 1 mile to the intersection with US Highway 70 (S. Church Street) and make a right (east). City Park is located approximately 0.25 mile on the right.

1.1 USGS Hydrologic Unit Codes (8 and 14-digit)

Little Alamance Creek and its unnamed tributary are located in the 8-digit Hydrologic Unit Code (HUC) 03030002 and the 14-digit Local Watershed Unit HUC 03030002040010. Both streams are located in the North Carolina Division of Water Quality (NCDWQ) Subbasin 03-06-03 (NCDWQ 2005). The NCEEP identifies this HUC as a targeted Local Watershed in their Watershed Restoration Plan for the Cape Fear River Basin (NCDENR 2001). Watersheds in this plan exhibit the need and opportunity for stream and riparian buffer restoration.

1.2 NCDWQ River Basin Designations

Little Alamance Creek and its unnamed tributary are located in the Cape Fear River Basin. The information presented in the following section is derived from the Cape Fear Basinwide Assessment Report (NCDWQ 2005a). The Cape Fear River Basin drains the middle portion of North Carolina and includes portions of 26 counties and 115 municipalities. It is one of four river basins completely contained within North Carolina state boundaries. It is the state's largest river basin (9,322 square miles) and flows southeast from the north-central Piedmont region near Greensboro to the Atlantic Ocean near Wilmington (NCDWQ 2005a).

2.0 Watershed Characterization

2.1 Drainage Area

The drainage area of Little Alamance Creek is approximately 4.2 square miles. Approximately 40 to 50 percent of the drainage area is impervious. The majority of the drainage area (approximately 80 percent) is urban residential consisting of single family homes. The remaining 20 percent of the watershed is comprised of city streets, businesses, light industrial and natural/undeveloped areas. There are several impoundments located in the watershed; the two largest are Mays Lake and Gant Lake (Figure 2).

The drainage area of the unnamed tributary is approximately 0.1 square mile. Nearly 100 percent of the drainage area is urban residential and city streets. Approximately 50 to 60 percent of the drainage area is impervious. Table 1 outlines the drainage area of both streams, and Table 2 describes the land uses.

Major point sources of contaminants that contribute to the degradation of water quality in the drainage area are stormwater culverts draining city streets. These culverts carry all contaminants deposited by

vehicles and residents on city streets to the stream system. Non-point sources included runoff from residential yards. Runoff from residential yards includes fertilizers, herbicides, and pesticides.

2.2 Surface Water Classification

Best usage classification for surface waters is determined by NCDWQ. Both Little Alamance Creek and its unnamed tributary are classified as Class C, nutrient sensitive waters (NSW). Class C denotes waters that are suitable for aquatic life propagation, wildlife, secondary recreation, and agriculture.

Section 303(d) of the Clean Water Act (CWA) requires states to develop a comprehensive public accounting of all impaired waters. The list includes waters impaired by pollutants, such as nitrogen, phosphorus and fecal coliform bacteria, and by pollution, such as hydromodification and habitat degradation. The source of impairment might be from point sources, nonpoint sources, or atmospheric deposition. Little Alamance Creek is listed on the North Carolina 303(d) List as impaired due to impaired biological integrity (NCDWQ 2006). The impairment is due to fair and poor benthic community ratings at benthic monitoring sites BB388, BB193, BB131, and BB78. An NCDWQ total maximum daily load (TMDL) stressor study found that urban runoff from large impervious surface areas in the watershed has caused stream channelization with associated habitat degradation. Pollutants associated with urban runoff as well as riparian area removals are also noted stressors to the benthic community. The report noted that streambank erosion exists and many storm sewers discharge into Little Alamance Creek (NCDWQ 2005).

2.3 Physiography, Geology and Soils

The Little Alamance Creek restoration site is located in the Piedmont Physiography Province of North Carolina. The Piedmont Province occupies about 45 percent of the area of the state and consists of generally rolling, well-rounded hills and ridges with a few hundred feet of elevation difference between the hills and valleys. Elevations in the Piedmont range from 300 to 600 feet above mean sea level (ft msl) near its border with the Coastal Plain to 1,500 feet at the foot of the Blue Ridge (NCGS 2004). The Piedmont includes some relatively low mountains, including the South Mountains and the Uwharrie Mountains. Elevations within the restoration site range from approximately 575 to 610 ft msl.

According to the Natural Resources Conservation Service (NRCS) Official Soil Series Descriptions (OSD) webpage, four soil series are located on the project site (Cecil, Enon, Lloyd, and Urban). No hydric soils are located within the restoration site (Figure 3). Table 3 discusses these soil series.

2.4 Historical Land Uses and Development Trends

As far back as could be researched, the site has been in its current condition. A city directory search of the area conducted by Environmental Data Resources, Inc. (EDR 2007) shows that the city pool, city recreational center, and the City Department of Recreation and Parks were located at their current site in 1964. Currently, the watershed is nearly fully developed by a light industrial residential mosaic. The characteristics of the watershed are not expected to change in the near future.

2.5 Endangered and Threatened Species

Some populations of fauna and flora have declined, or are in the process of declining due to either natural forces or their inability to coexist with humans. Federal law (under the provisions of Section 7 of the Endangered Species Act of 1973, as amended [ESA]) requires that any action likely to adversely affect a species classified as federally protected is subject to review by the United States Fish and Wildlife Service (USFWS). Other species may receive additional protection under state laws.

ARCADIS conducted a file review at the North Carolina Natural Heritage Program's (NHP) records to help identify the presence of federally protected species. Protected species lists for Alamance County were also obtained from the USFWS and NHP internet sites.

As of May 10, 2007, the USFWS lists no federally threatened or endangered species as potentially occurring in Alamance County. The USFWS lists six federal species of concern (FSC) for Alamance County (USFWS 2007). Federal species of concern are not protected under the provisions of the ESA. FSC species are defined as species that are under consideration for listing, but for which there is insufficient information to support listing as threatened or endangered (formerly C2 candidate species). The status of these species may be upgraded at any time, thus they are included here for consideration.

Table 4 describes FSC species for Alamance County, their habitat requirements, and if suitable habitat is available.

2.5.1 Biological Conclusion

The proposed restoration project will have no effect on federally listed threatened or endangered species.

2.5.2 Federal Designated Critical Habitat

The USFWS designates critical habitats that are deemed necessary for the survival of federally listed threatened or endanger species. Activities within these designated areas are subject to federal review and approval.

2.5.3 Habitat Description

There are no designated critical habitats within the project area (USFWS 2007).

2.5.4 Biological Conclusion

The proposed restoration project will have no effect on federally designated critical habitats.

A formal letter was sent to the USFWS on April 10, 2007, requesting a site review. ARCADIS has not received a response to date from USFWS. A formal letter was sent to North Carolina Wildlife Resources Commission (NCWRC) on April 10, 2007, requesting a site review. The NCWRC recommend preserving as many mature trees as possible (ARCADIS 2007).

2.6 Cultural Resources

A review of North Carolina Historic Preservation Office (NCHPO) files was conducted, and no resources were identified within the project area. Several historic structures were identified outside the project area.

2.6.1 Potential for Historic Architectural Resources

A known historic carousel is located within the City Park site. This carousel is located near the northern portion of the site, approximately 100 feet from Little Alamance Creek.

2.6.2 Potential for Archaeological Resources

No potential archaeological resources were identified at the City Park site.

2.6.3 SHPO/THPO Concurrence

A formal letter was sent to NCHPO on April 10, 2007, for a project site review. A response was received on May 10, 2007. NCHPO had no comment on the City Park site.

2.6.4 Other Compliance Issues

There are no other compliance issues associated with this site.

2.7 Constraint Analysis

Constraints affecting stream restoration options include: five pedestrian crossings, two train crossings, train tracks (and other park facilities, including the historic carousel), sanitary sewer lines, stormwater culverts, water lines, construction window related to park operations, and existing mature trees.

2.7.1 Property Ownership and Site Access

The site is located on property owned and operated by the City of Burlington. The City has agreed to the stream restoration project. The site is easily accessed from City streets or parking lots. City Park is heavily used by the public. A carousel and an amusement train railway are located within the park. The carousel is outside of the potential area of disturbance. However, the train tracks cross the stream in two locations. There are also five pedestrian crossings, one of which is also used for City vehicle access.

2.7.2 Environmental Screening

An EDR report was obtained for the site. The EDR report summarizes a search of available environmental records for the evaluation of environmental risks associated with a parcel or real estate. The EDR report for the Little Alamance Creek site identified 7 underground storage tanks (UST) and 12 leaking underground storage tanks (LUST) with 1/4 mile of the site. Six of the USTs have been permanently closed, and one is currently in use (Fairway One Stop gas station, 1382 South Church Street). Of the 12 LUSTs, 8 have been closed out and 4 were in the response phase (ARCADIS 2007).

2.7.3 Utilities and Easements

There are several sanitary sewer lines within City Park. In most cases the sewer lines are located an adequate distance from the streams so that they will not affect restoration practices. There are locations where the sewer line is located within 15 feet of the top of stream bank. The design will be adjusted to avoid damaging the sewer lines. Portions of the sewer lines are located within the proposed 50-foot buffer. Based on conversation with City of Burlington staff, planting can occur over the sewer lines. However, access to the manholes within the buffer needs to be maintained. Access could consist of a 10-to 12-foot wide path from edge of the buffer to the manhole.

Two exposed sewer lines cross Little Alamance Creek, both near the carousel. The sewer line crossings are ductile iron pipe, which is very durable and can tolerate high stream flows. These crossings will have little effect on the stream restoration design. At most, the stream banks may require some riprap armoring in these areas.

2.7.4 FEMA / Hydrological Trespass

Restoration activities will involve stabilizing the stream in its current location. Stream banks will be regraded to a more gradual slope and a riparian buffer established. The stream bed elevation will not be raised. Stormwater BMPs will likely consist of dry detention ponds or wet gardens that dry quickly after storm events. Hydrological trespass is not a concern due to the fact that wetland mitigation will not take place at the site.

A No Impact Study will be conducted to determine any impacts to the floodplain from restoration. If negative impacts to the floodplain are discovered, a Conditional Letter of Map Revision may need to be prepared and submitted to FEMA in order to obtain a work permit for the project.

3.0 Project Site Streams (Existing Conditions)

Little Alamance Creek is located in City Park in the City of Burlington. The project begins at a culvert beneath South Church Street and continues approximately 2,640 feet through City Park and ends at a bridge at Overbrook Road. An unnamed tributary enters Little Alamance Creek approximately 500 feet downstream of South Church Street. The approximately 500-foot-long unnamed tributary begins at a culvert under Overbrook Road. Little Alamance Creek is approximately 30 to 60 feet wide at the top of bank with banks ranging between 4 and 8 feet high and bank heights ratios between 1.0 and 1.4. The unnamed tributary is approximately 5 to 10 feet wide at the top of bank with bank heights ratios between 1.0 and 1.3.

Little Alamance Creek flows through a maintained park setting, with several large mature trees outside the stream banks. A paved walking trail winds among the trees and crosses Little Alamance Creek in five locations. The majority of the park is regularly mowed. However, the City recently implemented a no mow policy on the stream banks and 10 to 20 feet beyond the top of the stream bank. The intent of the no mow area is to allow a buffer to reestablish naturally along the stream. An amusement train railway crosses the stream in two locations in this area. Downstream of the amusement train railway, the stream flows south of a historic carousel and then north of a baseball/softball field. Little Alamance Creek then exits the site under Overbrook Road. A beaver dam was identified near the downstream end of the project after the detailed stream survey. The design sheets do not show the beaver dam and associated backwater.

Several areas along Little Alamance Creek are experiencing severe bank erosion. The most severe areas include the left bank, just downstream of the first pedestrian crosswalk, the left bank in the tight radius downstream of the first trestle, and the right bank adjacent to the baseball field and upstream of the last pedestrian crosswalk. These areas are experiencing high to extreme Near Bank Stress (NBS) resulting from high, vertical banks and/or tight radii. The Bank Erosion Hazard Index (BEHI) rating for these areas ranged from very high to extreme due to a lack of woody vegetation and high, vertical banks. Bank erosion has caused the stream to become overly wide in these sections and transverse and/or central bars have developed because the stream lacks the capacity to transport sediment through these reaches. Herbaceous vegetation has become well established throughout the entire restoration reach; however, the rooting depth is very shallow.

There are also several sections of Little Alamance Creek that are fairly stable due to the presence of woody vegetation along the banks and/or lower bank angles. The left bank along the last 500 feet of the proposed restoration reach is stabilized by a fairly dense stand of woody vegetation. Mature hardwoods along with several shrubby species such as tag alder (*Alnus serrulata*) also serve to stabilize the banks in the tight meander just upstream of the third pedestrian crosswalk. Riprap is also providing bank protection in this tight meander. The right bank, downstream of the third pedestrian crosswalk, is also vegetated and fairly stable.

The unnamed tributary is slightly incised and exhibits bank erosion along the majority of this reach. There is very little woody vegetation to support the stream banks aside from a few scattered black willow (*Salix nigra*) trees at the upstream end. Herbaceous vegetation dominates the stream banks along this reach of the unnamed tributary; however, the shallow rooting depth has led to bank erosion along the lower portions of the banks and created several undercut banks. Photographs of the site are included in Appendix 1.

3.1 Channel Classification

Little Alamance Creek is classified as a C/E5/1 stream type. The C5 stream type is a slightly entrenched, meandering, sand dominated, riffle/pool channel with a well developed floodplain (Rosgen 1996). The E5 stream type is characterized by low to moderate sinuosity, gentle to moderately steep gradients with very low channel width to depth ratios (Rosgen 1996). The substrate of an E5/1 or C5/1 stream type is comprised mainly of sand, with the occurrence of bedrock. The hybrid classification given to Little Alamance Creek reflects the range of channel dimensions found throughout the site.

The unnamed tributary to Little Alamance Creek is classified as an E4/1 stream type; however, it is slightly incised throughout most of the reach with bank height ratios ranging between 1.0 and 1.3. The E4 stream type is characterized by low to moderate sinuosity, gentle to moderately steep gradients with very low channel width to depth ratios (Rosgen 1996). The substrate of an E4/1 stream type is comprised mainly of gravel with the occurrence of bedrock. NCDWQ Stream Classification Forms were prepared for both streams. Little Alamance Creek scored 47.5. The unnamed tributary scored 33. Stream classification forms are presented in Appendix 2.

3.2 Discharge (bankfull, trends)

The bankfull discharge was determined by first calculated the streams average velocity. Using the Manning's equation with a calculated Manning's "n" value of 0.054 ft ^{1/6}, the average velocity for the channel is 2.5 feet per second. Applying this velocity over the average cross sectional area of the channel (95.0 square feet [ft²]), a discharge of 237.5 cubic feet per second (cfs) is calculated. This discharge was then compared to the revised North Carolina Rural Curves developed by Haywood County Natural Resources Conservation Service. Based on the curve the bankfull discharge for a 4.2 square mile drainage area is approximately 188.0 cfs. This value is less than calculated discharge. However, as with the bankfull cross sectional area discussion above, the bankfull discharge taken from the regional curve is expected to be lower than the actual stream discharge because of the urban setting. The calculated discharge is likely to be more accurate than the discharge determined by the regional curves since it reflects actual on site conditions.

3.3 Channel Morphology (Pattern, Dimension, Profile)

The channel dimension was measured by taking cross section surveys of the channel at representative locations. Six riffle cross sections were measured on Little Alamance Creek; two on the unnamed tributary. The upstream-most cross section on Little Alamance Creek was not used in the channel morphology assessment. This cross section is located just downstream of the box culvert under South Church Street. This proximity to the culvert has resulted in an excessively wide channel which is not representative of the entire reach. Stream dimension information was obtained from the remaining cross sections. Little Alamance Creek's cross sectional area ranged between 79.3 ft² and 125.0 s ft² with an average of 95.0 ft². Channel width ranged from 31.8 feet to 42.5 feet with an average of 36.2 feet, and mean depth ranged between 2.2 feet and 2.9 feet, with an average of 2.6 feet. The width to depth ratio ranged between 11.6 and 17.0 with an average of 14.0. The unnamed tributary's cross sectional area ranged between 14.8 ft² and 16.7 ft² with an average of 15.8 ft². Channel width ranged from 10.9 feet to 13.0 feet with an average of 1.3 feet. The width to depth ratio ranged between 1.1 feet and 1.5 feet with an average of 9.3.

Sinuosity is the measure of the pattern or the curviness of a stream channel. Sinuosity is calculated by dividing the stream length by the valley length or dividing the valley slope by the stream slope. The sinuosity of Little Alamance Creek calculated to be 1.2. The sinuosity of the unnamed tributary calculated to be 1.1.

The average water surface slope of Little Alamance Creek is 0.0024 ft/ft (0.24 percent). Little Alamance Creek is a pool-dominated system with approximately 65 percent of the stream length being comprised of pools. In the middle section of the project reach, the pools are separated by fairly short and steep bed rock steps.

The average water surface slope of the unnamed tributary is 0.0095 ft/ft (0.95 percent). The upper reach immediately downstream of Overbrook Road is steeper than the lower reach at the confluence with Little Alamance Creek. The lower reach is located in the relatively flat floodplain of Little Alamance Creek.

The particle size distribution of Little Alamance Creek's substrate is: $D_{16} = 0.2 \text{ mm}$ $D_{35} = 0.7 \text{ mm}$ $D_{50} = 2.4 \text{ mm}$ $D_{84} = 138.0 \text{ mm} D_{95} = 216.0 \text{ mm}$ The particle size distribution of the Unnamed Tributary's substrate is: $D_{16} = 0.2 \text{ mm}$ $D_{35} = 0.5 \text{ mm}$ $D_{50} = 3.4 \text{ mm}$ $D_{84} = 19.0 \text{ mm}$ $D_{95} = 53.0 \text{ mm}$

Tables 3 and 4 and sheets 3 and 3A of the design sheets show the channel morphology measurements.

3.4 Channel Stability Assessment

A BEHI analysis was performed on Little Alamance Creek and its unnamed tributary. The ratings ranged from low to extreme on Little Alamance Creek and from low to very high on the unnamed tributary. Contributing to the high, very high and extreme ratings were high bank heights, shallow rooting depths, and low rooting densities (a function of the lack of woody vegetation). Near bank stress (NBS) ranged from low to extreme on both Little Alamance Creek and the unnamed tributary. Extreme NBS ratings were due to high banks, central bars, and tight meander bends. Based on these ratings, an estimated 694 tons of sediment per year are being contributed by this reach of Little Alamance Creek, and the unnamed tributary is contributing an additional 55 tons of sediment per year. The BEHI and NBS data sheets are included in Appendix 3.

The modified channel stability rating for both streams was determined to be poor (unstable) according to the Pfankuch channel stability rating.

3.5 Bankfull Verification

The riffle cross sectional areas were compared to the revised North Carolina Rural Curves developed by Haywood County Natural Resources Conservation Service. The revised curve predicts the cross sectional area for Little Alamance creek and the unnamed tributary to be 46.1 ft^2 and 3.2 ft^2 , respectively. The actual average cross sectional area was measured to be 95.0 ft^2 on Little Alamance Creek and 15.8 ft^2 on the unnamed tributary. This is exactly what was expected to occur because of the streams urban setting. In order to validate the bankfull determination, cross section measurement was taken on Brown Branch (the adjacent watershed to the east) and several other urban streams. These areas were plotted against the North Carolina Rural Curves to see where they would fall in relation to the curve and each other. They were all consistently 2 to 3 times greater than the curve. Based on this relationship, it is expected that the correct bankfull indicator was selected on all streams surveyed.

3.6 Vegetation

Dominant woody vegetation observed on site includes: willow oak (*Quercus phellos*), sweet gum (*Liquidambar styraciflua*), tulip popular (*Liriodendron tulipifera*), red maple (*Acer rubrum*), white mulberry (*Morus alba*), tag alder (*Alnus serrulata*), Chinese privet (*Ligustrum sinense*), American elm (*Ulmus Americana*), black willow (*Salix nigra*), weeping willow (*Salix babylonica*), eastern red cedar (*Juniperus virginiana*), green ash (*Fraxinus pennsylvanica*), box elder (*Acer nugundo*), redbud (*Cercis canadensis*), trumpet creeper (*Campsis radicans*), mimosa (*Albizia julibrissin*), persimmons (*Diospyros virginiana*), winged elm (*Ulmus alata*), Virginia creeper (*Parthenocissus quinquefolia*), tree of heaven (*Ailanthus altissima*), Flowering dogwood (*Cornus florida*), river birch (*Betula nigra*), Virginia pine (*Pinus virginiana*), and poison ivy (*Toxicodendron radicans*).

Herbaceous vegetation on site includes various jewel weed (*Impatiens capensis*), poke weed (*Phytolacca americana*), Japanese stiltgrass (*Microstegium vimineum*), tear-thumb (*Polygonum saggitafolia*), false nettle (*Boehmeria cylindrica*), dodder vine (*Cuscuta gronovii*), and fescue grasses (*Festuca* sp.).

4.0 Reference Streams

Due to the confined nature of Little Alamance Creek and the unnamed tributary, in combination with the urban setting, data used to develop the restoration plan were derived from several reference reaches and several restored streams in Greensboro, North Carolina. These streams were chosen because they exhibited valley types and watersheds similar to those at the restoration site. Data from a reach of Brown Branch in the City of Burlington were also used. The majority of the proposed work on site will consist of Enhancement I. Only one short section of the channel thalweg will be realigned. Therefore, several geomorphic measurements were not taken on the reference streams. These include stream pattern, stream profile, and several stream feature cross sections. Figure 4 shows the location of the reference streams used. Morphological measurements of the reference streams are shown in Table 4 and 5 and design sheets 3 and 3A.

4.1 Watershed Characterization

The Greensboro reference streams are located in urban watersheds. The watersheds ranged from urban residential to urban industrial. Watershed sizes ranged from 0.18 square mile to 0.77 square mile.

4.2 Channel Classification

The reference streams used in the design are classified as C4, E4, and C/E4 streams. Characteristics of C4 and E4 streams are discussed in section 3.1.

4.3 Discharge (bankfull, trends)

Bankfull discharges were obtained from the revised North Carolina Rural Curves. These discharges ranged from 14.1 cfs to 45.5 cfs. As with the reach of Little Alamance Creek and the unnamed tributary to be restored, the discharges of the reference streams taken from the regional curves are expected to be lower than the true discharge because of the urban setting.

4.4 Channel Morphology (pattern, dimension, profile)

Channel pattern and profile measurements were not collected on the reference streams. Due to the restoration site constraints, significant channel realignment is not proposed. Therefore, pattern and profile measurements were not required. Bankfull cross sectional areas ranged from 13.0 ft^2 to 35.3 ft^2 , bankfull width ranged from 9.5 feet to 20.9 feet, mean depth ranged from 1.4 feet to 1.7 feet and width to depth ratios ranged between 6.9 and 12.4.

4.5 Channel Stability Assessment

Visual assessments of channel stability were made during the site visit to each reference stream. In general, the banks of all the reaches assessed appeared to be stable. However, areas of bank erosion were

observed at all the sites, including the newly restored sites. The cross section measurements were taken at stable sections well outside the influence of the bank erosion areas.

4.6 Bankfull Verification

A cross section was taken at a typical riffle on each stream. The bankfull cross sectional areas were compared to the revised North Carolina rural curves. The cross sectional area of each stream was consistently above the regression line. The cross sectional areas were two to three times higher than the value predicted by the regression equation.

4.7 Vegetation

Woody vegetation at the reference sites varied. Vegetation at the recently restored site consisted of young, newly planted hardwood species. The bare root plantings were too young to identify. Vegetation at the reference sites was typical of urban areas consisting of red maple, black walnut and Chinese privet. Vegetation at the segment of Brown Branch was dominated by bamboo.

5.0 **Project Site Wetlands**

There are no wetlands present on the site.

6.0 **Project Site Restoration Plan**

6.1 Restoration Project Goals and Objectives

The goals of the restoration project are to improve water quality and improve aquatic habitat diversity. Water quality will be improved by stabilizing eroding stream banks, thereby reducing sedimentation in the streams and incorporating stormwater BMPs to treat stormwater prior to discharging into the streams. A continuous riparian buffer will be established to provide shading of the streams, slow sheet flow, and reduce erosion and sedimentation. Aquatic habitat will be improved by creating a riffle/pool complex and providing in-stream woody debris. Rock structures will be installed to provide bank protection, establish grade control, and promote pool formation. In-stream woody debris will be provided by utilizing root wads, brush mattresses, and/or log vanes throughout the reach. Table 5 outlines the project restoration structure and objectives.

6.1.1 Designed Channel Classification

A C 4/1 channel is proposed at the restoration site. Sections of Little Alamance Creek currently classify as a C4/1 stream; however, it is slightly incised with bank height ratios ranging from 1.0 to 1.4. In the sections where the bank height ratio is greater than 1.0, a bankfull bench will be excavated to the extent possible to obtain a bank height ratio of 1.0. Due to the constraints on site, the channel sinuosity will not be altered. One short section of the channel thalweg will be realigned and a center bar removed. The realignment was not based on reference reach data. The channel pattern for this short section was based on values that fit within the existing design constraints and still provide a stable channel. Reference reach data could not be used because the reference reach area did not contain the same site constraints as the project site. Instream structures will be installed to increase habitat diversity and provide grade control and bank protection.

The unnamed tributary will be restored to a low sinuosity C 4/1 channel. The site constraints prohibit changing the channel alignment. The unnamed tributary is slightly incised with bank height ratios between 1.0 and 1.3. In the sections where the bank height ratio is greater than 1.0, a bankfull bench will be excavated to the extent possible to obtain a bank height ratio of 1.0. Instream structures will be installed to increase habitat diversity, provide grade control and bank protection. Design sheets are included in Appendix 4.

The vegetation community established on site will closely resemble a Piedmont/Low Mountain Alluvial Forest (Schafale and Weakley 1990). Alluvial Forests are located on smaller streams and, therefore, smaller floodplains than Piedmont/Mountain Bottomland Forests and lack the development of the depositional fluvial landforms (levees, sloughs and ridges) (Schafale and Weakley 1990). Woody species planted within the floodplain will include river birch (*Betula nigra*), sycamore (*Platanus occidentalis*), American elm, green ash, hackberry (*Celtis laevigata*), American holly (*Ilex opaca*), and ironwood (*Carpinus caroliniana*). Woody species on the stream banks will be comprised of the above and also include black willow, tag alder, buttonbush (*Cephalanthus occidentalis*), and elderberry (*Sambucus canadensis*). The width of the buffer will depend on the results of discussions with the City of Burlington.

Bare root seedlings of tree and shrub species will be planted within the buffer at a density up to 555 stems per acre (10-foot centers). Appropriate species on live stakes will be installed within the bankfull channel. Planting will be performed between December and March to allow plants to stabilize during the dormant period and set roots during the spring. Existing non-native exotics within the proposed buffer will be removed during construction. Exotic trees and shrubs will be removed with construction equipment or cut and the stumps treated with an appropriate herbicide.

6.2 Sediment Transport Analysis

Sediment transport analysis is used to predict if the designed channel will be able to move the bedload that is supplied to the channel. It compares the proposed channel morphological parameters to the bed load material in the channel and determines if the proposed channel is capable of moving the material.

6.2.1 Methodology

Sediment transport analysis was conducted by calculating the proposed channel shear stress then comparing it to the Shields curve (Leopold, Wolman and Miller 1964). The Shields curve estimates the largest size particle capable of moving at a given shear stress. This size particle is then compared to the particle size within the stream bed. If the Shields curve particle size estimated is significantly higher than the actual particle size in the stream, then the stream is degrading. If the Shields particle size estimate is significantly smaller than the particle size in the stream, then the stream, then the stream is aggrading. If the Shields particle size is near the same size as the particle in the channel, then the stream is stable.

In order to validate the above calculations, the critical dimensionless shear stress was calculated. The critical dimensionless shear stress estimates the mean bankfull depth and bankfull water surface slope required to transport the bed material. This depth and slope are compared to the proposed depth and slope to determine channel stability.

6.2.2 Calculations and Discussion

The calculated shear stress for the proposed Little Alamance Creek is 0.37 lb/ft^2 (see calculations below). The particle size moveable at this shear stress according to the Shields curve is 80 mm. The largest particle from the pavement sample is 80 mm. These particles are identical in size.

• = •RS	Where	 = bankfull shear stress (lb/ft²) = specific weight of water (lbs/ft³) R = hydraulic radius of bankfull channel (ft) S = average water surface slope (ft/ft)
• = $62.4 \text{ lbs/ft}^3 \text{ x } 2$ = 0.37 lb/		ft/ft

The calculated shear stress for the proposed unnamed tributary is 0.71 lb/ft^2 (see calculations below). The particle size moveable at this shear stress according to the Shields curve is 55 mm. The largest particle from the pavement sample is 48 mm. These particles are very near the same size.

$\bullet = \bullet RS$	Where	• = bankfull shear stress (lb/ft^2)
		• = specific weight of water (lbs/ft^3)
		R = hydraulic radius of bankfull channel (ft)
		S = average water surface slope (ft/ft)
• = $62.4 \text{ lbs/ft}^3 \text{ x}$ = 0.71 b/f		ìt

Based on the shear stress calculations and the Shields curve predictions of the moveable particle size, both designed channels will be able to transport the existing bedload.

6.3 HEC-RAS Analysis

A "No-Impact" Study will be performed on Little Alamance Creek in order to obtain a floodplain development permit for the project. The currently effective model will be duplicated, and subsequent model(s) will be created with the US Army Corps of Engineers hydraulic modeling software, HEC-RAS.

6.3.1 No-rise, LOMR, CLOMR

Little Alamance Creek is located in Federal Emergency Management Agency (FEMA) regulated floodways where base flood elevations have been determined. The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1 percent annual chance flood (100 year) can be carried without an increase in 100-year water surface elevation. Work within the floodway requires the preparation of a No Impact Study during the design phase and a Letter of Map Revision (LOMR) at the completion of work if the work results in a change in the base flood elevations.

6.4 Stormwater Best Management Practices

6.4.1 Narrative of Site-Specific Stormwater Concerns

The main stormwater concern at the Little Alamance Creek site is stormwater discharging directly into the stream channels. Stormwater culverts are located on the channel banks and discharge directly into the channel in several locations. These discharges deliver pollutants carried from lawns and city streets to Little Alamance Creek and its unnamed tributary. This untreated urban runoff contributes to the water quality degradation of the watershed.

6.4.2 Device Description and Application

BMP devices expected to be used on site are grassed swales, level spreaders, wet gardens, and filter strips. Stormwater culverts that discharge directly into Little Alamance Creek and the unnamed tributary will be removed and replaced with grassed swales. Grassed swales are trapezoidal or parabolic earthen channels covered with a dense growth of a hardy grass. Grassed swales take up some pollutants and help filter sediment and other solid particles out of runoff. They convey stormwater and provide some stormwater management for small storms by retarding peak flow rates, lowering velocities of runoff and by infiltrating runoff water into the soil. Because of their limited pollutant removal ability, grass swales are assumed to have a total suspended solid removal of 35 percent (NCDWQ 2007). The estimate removal rate is 29 percent for total phosphorous and 38 percent for nitrate nitrogen (USEPA 2007).

Level spreaders will be installed at stormwater culvert outlets or where runoff from adjacent streets enters the site in a concentrated area. Level spreaders convert concentrated flow to sheet flow and release it uniformly over a stabilized area, namely a filter strip. For this project, the level spreaders were designed to accommodate the 10-year storm event, thereby eliminating the necessity of a high flow by-pass system (NCDWQ 2007). Filter strips are sections of vegetation designed to reduce pollutants in stormwater runoff before the runoff enters a steam. They remove pollutants from runoff by the filtering action of the vegetation, infiltration of pollutant-carrying water and sediment deposition. Properly constructed forested and grassed filter strips can be expected to remove a minimum of 25 to 40 percent of total suspended solids. The estimated removal rate is 30 to 42 percent for total phosphorous and 85 percent for total nitrogen though studies have shown their effectiveness to vary widely (USEPA 2007). In this application, the stabilized area will be the restored riparian buffer.

7.0 Performance Criteria

7.1 Streams

Success criteria need to be established to determine if the restoration project is meeting the designed goals and objectives. These will include changes in the dimension, pattern, profile, bed material, and vegetation over the 5-year monitoring period. Stream performance monitoring will be conducted following protocols outlined in the US Army Corps of Engineers Stream Mitigation Guidelines (April 2003).

7.1.1 Dimension

The stream cross section measurements should not significantly change from the baseline cross section. Minor adjustment in the cross section within specified tolerances of the construction documents is expected. The adjustment is due to the lack of precision of large heavy machinery. The lack of permanent vegetation can also contribute to adjustments in the channel dimension.

7.1.2 Pattern

The stability of stream pattern will be measured using stream sinuosity (the ratio of stream length divided by valley length or approximated by the ratio of valley slope divided by stream slope). If there is a significant change in sinuosity, then belt width, radius of curvature, and meander length will be evaluated to determine where the adjustment occurred that affected the sinuosity.

7.1.3 Profile

The channel profile is not expected to significantly change over the monitoring period. The baseline average water surface slope will be used as a measure of profile stability. The average water surface slope will be determined by taking water surface elevation readings at the beginning and the end of the monitored reach, at the same feature (head of riffle, head of pool, etc.), determining the elevation difference between the two and dividing the difference by the stream length between the two features.

Another measure of channel profile stability is pool-to-pool spacing. This is the stream distance between the same features on sequential pools. The measurements are usually taken between heads of pools. Baseline pool-to-pool spacing will be measured and recorded.

7.1.4 Material

Usually the particle size distribution of the bed material becomes coarser as a result of stream restoration. This is a result of adjusting the shear stress and stabilizing the existing stream banks. The change in the substrate material will be measured over the 5-year monitoring period.

7.1.5 Photo Points

Permanent photo points will be established on the site. The photographs should show the succession of vegetation growth and no significant changes in the stream configuration or structure stability.

7.2 Stormwater Management Devices

Stormwater management devices will be visually evaluated during the monitoring site visits. They will be inspected and any evidence of erosion, sediment build up, lack of vegetation establishment, vandalism will be noted. Water quality monitoring will not be performed.

7.3 Wetlands

Wetlands are not proposed on the site. Therefore, wetland monitoring is not required.

7.4 Vegetation

Vegetation monitoring protocol will follow the CVS-EEP Protocol for Recording Vegetation; Level 1-2 Plot Sampling Only, Version 4.0 (Lee et al., 2006). This protocol establishes monitoring plots based on the size of the buffer area planted and documents the development of the buffer.

Vegetation success criteria are based on the US Army Corps of Engineers Stream Mitigation Guidelines April 2003, which is the survival of at least 320 stems per acre through year 3; 288 stems per acre in year 4; and 260 stems per acre in year 5. Stems tallied on site will include all "Character Tree Species." Character Tree Species include all planted species and those listed by Schafale and Weakley (1990) as likely to occur in this forest type community.

As with most urban restoration projects, the presence of non-native, exotic vegetation is a concern. This vegetation typically responds well to disturbance and may affect planted vegetation success. To reduce the potential of non-native vegetation out-competing the desired species, existing non-native exotics within the proposed buffer will be removed during construction. Exotic trees and shrubs will be removed with construction equipment or cut and the stumps treated with an appropriate herbicide. The establishment of non-native vegetation during the monitoring period will also be monitored and remedial actions will be taken if deemed necessary to meet establish success criteria.

7.5 Schedule/Reporting

The baseline and all subsequent annual monitoring reports will be submitted to the NCEEP prior to November 1 of all monitoring years.

8.0 References

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USEPA Menu of Stormwater BMPS grass swales

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USEPA Menu of Stormwater BMPS riparian buffers

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Tables

Table 1. Drainage Areas,Project Number 060685501 (Little Alamance Creek)

Reach	Drainage Area (Square Mile)
Little Alamance Creek	4.2
Unnamed Tributary	0.12
Total	4.2*

*Total drainage area for Little Alamance Creek measured at down stream end of project.

Table 2. Land Use of Watershed, Project Number 060685501 (Little Alamance Creek)

Land Use	Acreage	Percentage
Urban Residential	2,150	80%
City streets, businesses, light industrial and natural/undeveloped areas	538	20%

TABLE 3: MORPHOLOGICAL CHARACTERISTICS OF THE EXISTING AND PROPOSED CHANNEL WITH GAGE STATION AND REFERENCE REACH DATA (Adapted from Rosgen, 1996)

Restoration Site: Reference Reach:		nance Creek, Burl Park, Park @ Frier	u ,	Branch u/s						
					Benbow Reference		Park @ F Reference		Brown Bra	nch u/s
Variables	Existing C	Channel	Proposed	Reach	Reach		Reach		Reference	Reach
1. Stream Type		C/E5/1		C 4/1	C4	4	E4	ŀ	C/E	4
2. Drainage Area (sq. mi)		4.2		4.2	0.1	7	0.2	2	0.7	7
	Mean:	36.2	Mean:	36.2	Mean:	20.9	Mean:	9.5	Mean:	15.
3. Bankfull Width (Wbkf) ft	Range:	31.8 - 42.5	Range:		Range:		Range:		Range:	
4. Bankfull Mean Depth	Mean:	2.6	Mean:	2.6	Mean:	1.7	Mean:	1.4	Mean:	1.
(dbkf) ft	Range:	2.2 - 2.9	Range:		Range:		Range:		Range:	
5. Width/Depth Ratio	Mean:	14.0	Mean:	13.8	Mean:	12.4	Mean:	6.9	Mean:	9.
(Wbkf/dbkf)	Range:	11.6 - 17.0	Range:		Range:		Range:		Range:	
 Bankfull Cross-Sectional 	Mean:	95.0	Mean:	95.0	Mean:	35.3	Mean:	13.0	Mean:	24
Area (Abkf) sq ft	Range:	79.3 - 125.0	Range:		Range:		Range:		Range:	
7 Dephé II Mana Malacity	Mean:	2.5	Mean:	2.5	Mean:	*	Mean:	*	Mean:	. *
 Bankfull Mean Velocity (Vbkf) fps 	Range:		Range:		Range:		Range:		Range:	
	Mean:	237.5	Mean:	237.5	Mean:	*	Mean:	*	Mean:	*
 Bankfull Discharge, cfs (Qbkf) 	Range:		Range:		Range:		Range:		Range:	
	Mean:	4.0	Mean:	4.0	Mean:	3.1	Mean:	2.0	Mean:	2.
9. Maximum Bankfull Depth (dmax) ft	Range:	3.9 - 4.1	Range:	3.9 - 4.1	Range:		Range:		Range:	
10. Ratio of Low Bank	Mean:	1.2	Mean:	1.0	Mean:	1.0	Mean:	1.4	Mean:	1.
Height to Max. Bankfull Depth (Bhlow/dmax)	Range:	1.0 - 1.4	Range:		Range:		Range:		Range:	
	Mean:	94.0	Mean:	> 80.0	Mean:	40.0	Mean:	30.0	Mean:	30
11. Width of Flood Prone Area (Wfpa) ft	Range:	70.0 - 120.0	Range:		Range:		Range:		Range:	
	Mean:	2.6	Mean:	> 2,2	Mean:	1.9	Mean:	3.2	Mean:	2.
12. Entrenchment Ratio (Wfpa/Wbkf)	Range:	2.1 - 3.8	Range:		Range:		Range:		Range:	
	Mean:	361.0	Mean:	361.0	Mean:	*	Mean:	*	Mean:	*
13. Meander Length (Lm) ft	Range:	227.0 - 559.0	Range:	227.0 - 559.0	Range:		Range:		Range:	
	Mean:	10.0	Mean:	10.0	Mean:	*	Mean:	*	- Mean:	*
14. Ratio of Meander Length to Bankfull Width (Lm/Wbkf)	Range:	6.3 - 15.4	Range:	6.3 - 15.4	Range:		Range:		Range:	
	Mean:	115.0	Mean:	115.0	Mean:	*	Mean:	*	Mean:	*
 Raduis of Curvature (Rc) 	Range:	45.0 - 220.0	Range:	45.0 - 220.0	Range:		Range:		Range:	
16. Ratio of Radius of	Mean:	3.2	Mean:	3.2	Mean:	*	Mean:	*	Mean;	
Curvature to Bankfull Width Row	Range:	1.2-6.1	Range:	1.2-6.1	Range:		Range:		Range:	
. ,	Mean:	70.0	Mean:	70	Mean:	*	Mean:	*	Mean:	*
7. Belt Width (Wblt) ft	Range:	33.0 - 255.0	Range:	33.0 - 255.0	Range:		Range:		Range:	
	Mean:	1.9	Mean:	1.9	Mean:	*	Mean:	*	Mean:	*
18. Meander Width Ratio Wblt/Wbkf)	Range:	0.9 - 7.0	Range:	0.9 - 7.0	Range:		Range:		Range:	

19. Sinuosity (Stream	Mean:	1.2	Mean:	1.2	Mean:	*	Mean:	*	Mean:	*
length/valley distance) (k)	Range:		Range:		Range:		Range:		Range:	
	Mean:	0.0028	Mean:	0.0028	Mean:	*	Mean:	*	Mean:	*
20. Valley Slope (ft/ft) 21. Average Water Surface	Range:		Range:		Range:		Range:		Range:	
Slope or Bankful Slope for	Mean:	0.0024	Mean:	0.0024	Mean:	*	Mean:	*	Mean:	*
Reach (Sbkf or Savg)=(Svalley/k) ft / ft	Range:		Range:		Range:		Range:		Range:	
	Mean:	0.0005	Mean:	0.0	Mean:	*	Mean:	*	Mean:	*
22. Pool Slope (Spool) ft / ft	Range:	0.0 - 0.0015	Range:		Range:		Range:		Range:	
22. Datis of Deal Signa to	Mean:	0.2	Mean:	0.0	Mean:	*	Mean:	*	Mean:	*
23. Ratio of Pool Slope to Average Slope (Spool/Sbkf)	Range:	0.0 - 0.6	Range:		Range:		Range:		Range:	
24 Maximum Baal Danth	Mean:	6.1	Mean:	6.1	Mean:	*	Mean:	*	Mean:	*
24. Maximum Pool Depth (dpool) ft	Range:	5.5 - 6.9	Range:	5.5 - 6.9	Range:		Range:		Range:	
25. Ratio of Maximum Pool Depth to Bankfull Mean	Mean:	2.3	Mean:	2.3	Mean:	*	Mean:	*	Mean:	*
Depth (dpool/dbkf)	Range:	2.1 - 2.7	Range:	2.1 - 2.7	Range:		Range:		Range:	
	Mean:	37.6	Mean:	37.6	Mean:	*	Mean:	*	Mean:	*
26. Pool Width (Wpool) ft	Range:	32.3 - 42.3	Range:	32.3 - 42.3	Range:		Range:		Range:	
	Mean:	1.0	Mean:	1.0	Mean:	*	Mean:	*	Mean:	*
27, Ratio of Pool Width to Bankfull Width (Wpool/Wbkf)	Range:	0.9 - 1.2	Range:	0.9 - 1.2	Range:		Range:		Range:	
DR. Destroll Green configural	Mean:	140.2	Mean:	140.2	Mean:	*	Mean:	*	Mean:	*
28. Bankfull Cross-sectional Area at Pool (Apool) sq ft	Range:	121.0 - 156.6	Range:	121.0 - 156.6	Range:		Range:		Range:	
	Mean:	1.0	Mean:	1.0	Mean:	*	Mean:	*	Mean:	*
29. Ratio of Pool Area to Bankfull Area (Apool/Abkf)	Range:	1.3 - 1.7	Range:	1.3 - 1.7	Range:		Range:		Range:	
20 - Deal to Bool Creasing (n	Mean:	473.1	Mean:	473.1	Mean:	*	Mean:	*	Mean:	*
30. Pool to Pool Spacing (p- p) ft	Range:	313.7 - 749.5	Range:	313.7 - 749.5	Range:		Range:		Range:	
31. Ratio of Pool-to-Pool	Mean:	13.1	Mean:	13.1	Mean:	*	Mean:	*	Mean:	*
Spacing to Bankfull Width (p- p/Wbkf)	Range:	8.7 - 20.7	Range:	8.7 - 20.7	Range:		Range:		Range:	
	Mean:	293.7	Mean:	293.7	Mean:	*	Mean:	*	Mean:	*
32. Pool Length (Lp) ft	Range:	107.9 - 505.4	Range:	107.9 - 505.4	Range:		Range:		Range:	
33. Ratio of Pool Length to	Mean:	8.1	Mean:	8.1	Mean:	*	Mean:	*	Mean:	*
Bankfull Width (Lp/Wbkf)	Range:	3.0 - 14.0	Range:	3.0 - 14.0	Range:		Range:		Range:	
	Mean:	0.0126	Mean:	0.0126	Mean:	*	Mean:	*	Mean:	*
34. Riffle Slope (Sriff) ft / ft	Range:	0.0028 - 0.0254	Range:	0.0028 - 0.0254	Range:		Range:		Range:	
25 Defin of Diffle Slope to	Mean:	5.2	Mean:	5.2	Mean:	*	Mean:	*	Mean:	*
35. Ratio of Riffle Slope to Average Slope (Sriff/Sbkf)	Range:	1.2 - 10.6	Range:	1.2 - 10.6	Range:		Range:		Range:	
00 Maximum Diffe Danih	Mean:	4.0	Mean:	4.0	Mean:	*	Mean:	*	Mean:	*
36. Maximum Riffle Depth (driff) ft	Range:	3.9 - 4.1	Range:	3.9 - 4.1	Range:		Range:		Range:	
37. Ratio of Riffle Depth to	Mean:	1.5	Mean:	1.5	Mean:	*	Mean:	*	Mean:	*
Bankfull Mean Depth (driff/dbkf)	Range:	1.5 - 1.6	Range:	1.5 - 1.6	Range:		Range:		Range:	
	Mean:	0.0032	Mean:	0.0032	Mean:	*	Mean:	*	Mean:	*
38. Run Slope (Srun) ft / ft	Range:	0.0 - 0.0090	Range:	0.0 - 0.0090	Range:		Range:		Range:	

39. Ratio of Run Slope to	Mean:	1.3	Mean:	1.3	Mean:	*	Mean:	*	Mean:	*
Average Slope (Srun/Sbkf)	Range:	0.0 - 3.7	Range:	0.0 - 3.7	Range:		Range:		Range:	
40. Maximum Run Depth	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
(drun) ft	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
41. Ratio of Run Depth to Bankfull Mean Depth	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
(drun/dbkf)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
	Mean:	0.0039	Mean:	0.0039	Mean:	*	Mean:	*	Mean:	*
42. Slope of Glide (Sgl) ft / ft	Range:	0.0 - 0.0107	Range:	0.0 - 0.0107	Range:		Range:		Range:	
43. Ratio of Glide Slope to Average Water Surface	Mean:	1.6	Mean:	1.6	Mean:	*	Mean:	*	Mean:	*
Slope (Sgl/Sws)	Range:	0.0 - 4.5	Range:	0.0 - 4.5	Range:		Range:		Range:	
44. Maximum Glide Depth	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
(dgl) ft	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
45. Ratio of Glide Depth to Bankfull Mean Depth	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
(dgl/dbkf)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
46. Step Slope (Sst)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
47. Ratio of Step Slope to Average Water Surface	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
Slope (Sst/Savg)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
49 Movimum Stop Donth	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
 48. Maximum Step Depth (dst) 	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
49. Ratio of Step Depth to	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
Bankfull Mean Depth (dst/dbkf)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	

* Geomorphic data not collected due to existing design constraints. The majority of the work on site will consist of Enhancement I. Ranges for restoration areas were developed around the existing site constraints.

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Materials: Particle Size Distribution of Channel Material (mm)			
D16		0.2	
D35		0.7	
D50		2.4	
D84		138.0	
D95 Particle Size Distribution of Bar Material	Р	216.0 SP	
D16	15.0		<2
D35	22.4		<2
D50	29.8		<2
D84	99.6		17.9
D95	118.4		81.9
Largest Size Particle on Bar	80.0		

* Geomorphic data not collected due to existing design constraints. The majority of the work on site will consist of Enhancement I. Ranges for restoration areas were developed around the existing site constraints.

Sediment Transport:

Sediment Transport Validation (Based on Bankfuil Shear Stress)	Existing	Proposed
Calculated value (mm) from curve	30	30
Value from Shields Curve (lb/ft2)	0.43	0.43
Critical dimensionless shear stress	0.021	0.021
Minimal mean dbkf (ft) catculated using critical dimensionless shear stress equations	1.6	1.6

TABLE 4: MORPHOLOGICAL CHARACTERISTICS OF THE EXISTING AND PROPOSED CHANNEL WITH GAGE STATION AND REFERENCE REACH DATA (Adapted from Rosgen, 1996)

					Benbow P	ark	Park @ Fi		Brown Branch u/s			
Variables	Existing C	hannel	Proposed	Reach	Reference	Reach	Reference Reach		Reference	Reach		
1. Stream Type		E 4 /1		C 4/1	C4	4	E4		C/E	:4		
2. Drainage Area (sq. mi)		0.12		0.12	0.1	7	0.2		0.7	7		
	Mean:	12.0	Mean:	12.0	Mean:	20.9	Mean:	9.5	Mean:	15.1		
3. Bankfull Width (Wbkf) ft	Range:	10.9 - 13.0	Range:	10.9 - 13.0	Range:		Range:		Range:			
4. Bankfull Mean Depth	Mean:	1.3	Mean:	1.3	Mean:	1.7	Mean:	1.4	Mean:	1.6		
(dbkf) ft	Range:	1.1 - 1.5	Range:	1.1 - 1.5	Range:		Range:		Range:			
5. Width/Depth Ratio (Wbkf/dbkf)	Mean:	9.3	Mean:	9.3	Mean:	12.4	Mean:	6.9	Mean:	9.3		
	Range:	7,1 - 11.5	Range:	7.1 - 11.5	Range:		Range:		Range:			
C. Davider Conne	Mean:	15.8	Mean:	15.8	Mean:	35.3	Mean:	13.0	Mean:	nce Reach C/E4 0.77 15.1 1.6 9.3 24.3 * * 2.6 1.0 30.0 2.0 * *		
6. Bankfull Cross- Sectional Area (Abkf) sq ft	Range:	14.8 - 16.7	Range:	14.8 - 16.7	Range:		Range:		Range:			
7. Bankfull Mean Velocity (Vbkf) fps	Mean:	4.4	Mean:	4.4	Mean:	*	Mean:	*	Mean:	*		
-	Range:		Range:		Range:		Range:		Range:			
	Mean:	68.7	Mean:	68.7	Mean:	*	Mean:	*	Mean:	*		
 Bankfull Discharge, cfs (Qbkf) 	Range:		Range:		Range:		Range:		Range:			
9. Maximum Bankfull Depth (dmax) ft	Mean:	2.0	Mean:	2.0	Mean:	3.1	Mean:	2.0	Mean:	2.6		
	Range:	2.0 - 2.1	Range:	2.0 - 2.1	Range:		Range:		Range:			
10. Ratio of Low Bank	Mean:	1.2	Mean:	1.0	Mean:	1.0	Mean:	1.4	Mean:	1.0		
Height to Max. Bankfull Depth (Bhlow/dmax)	Range:	1.0 - 1.3	Range:		Range:		Range:		Range:			
	Mean:	33.5	Mean:	33.5	Mean:	40.0	Mean:	30.0	Mean:	30.		
 Width of Flood Prone Area (Wfpa) ft 	Range:	27.0 - 40.0	Range:	27.0 - 40.0	Range:		Range:		Range:			
	Mean:	2.9	Mean:	2.9	Mean:	1.9	Mean:	3.2	Mean:	2.0		
 Entrenchment Ratio (Wfpa/Wbkf) 	Range:	2.1 - 3.7	Range:	2.1 - 3.7	Range:		Range:		Range:			
	Mean:	83.9	Mean:	83.9	Mean:	*	Mean:	*	Mean:	*		
13. Meander Length (Lm) ft	Range:	55.8 - 111.9	Range:	55.8 - 111.9	Range:		Range:		Range:			
14. Ratio of Meander	Mean:	7.0	Mean:	7.0	Mean:	*	Mean:	*	Mean:	*		
Length to Bankfull Width (Lm/Wbkf)	Range:	4.7 - 9.3	Range:	4.7 - 9.3	Range:		Range:		Range:			
	Mean:	29.0	Mean:	29.0	Mean:	*	Mean:	*	Mean:	*		
 Raduis of Curvature (Rc) ft 	Range:	15.0 - 55.0	Range:	15.0 - 55.0	Range:		Range:		Range:			
16. Ratio of Radius of	Mean:	2.4	Mean:	2.4	Mean:	*	Mean:	*	Mean:	*		
Curvature to Bankfull Width (Rc/Wbkf)	Range:	1,2 - 4.6	Range:	1.2 - 4.6	Range:		Range:		Range:			
	Mean:	24.6	Mean:	24.6	Mean:	*	Mean:	*	Mean:	*		
17. Belt Width (Wblt) ft	Range:	13.5 - 33.7	Range:	13.5 - 33.7	Range:		Range:		Range:			
. ,	Mean:	2.0	Mean:	2.0	Mean:	*	Mean:	*	Mean:	*		
 Meander Width Ratio (Wblt/Wbkf) 	Range:	1.1 - 2.8	Range:	1.1 - 2.8	Range:		Range:		Range:			

10 Sinuarity (Stream	Mean:	1.1	Mean;	1.1	Mean:	*	Mean:	*	Mean:	*
19. Sinuosity (Stream ength/valley distance) (k)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
	Mean:	0.0106	Mean:	0.0106	Mean:	*	Mean:	*	Mean:	*
20. Valley Slope (ft/ft)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
21. Average Water Surface Slope or Bankful	Mean:	0.0095	Mean:	0.0095	Mean:	*	Mean:	*	Mean:	*
Slope for Reach (Sbkf or Savg)=(Svalley/k) ft / ft	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
22. Pool Slope (Spool) ft /	Mean:	0.0077	Mean:	0.0077	Mean:	*	Mean:	*	Mean:	,
t	Range:	0.0 - 0.0174	Range:	0.0 - 0.0174	Range:		Range:		Range:	
23. Ratio of Pool Slope to Average Slope	Mean:	0.8	Mean:	0.8	Mean:	*	Mean:	*	Mean:	Y
(Spool/Sbkf)	Range:	0.0 - 1.8	Range:	0.0 - 1.8	Range:		Range:		Range:	
A Maximum Real Donth	Mean:	2.4	Mean:	2.4	Mean:	*	Mean:	*	Mean:	-
24. Maximum Pool Depth (dpool) ft	Range:		Range:		Range:		Range:		Range:	
25. Ratio of Maximum	Mean:	1.8	Mean:	1.8	Mean:	*	Mean:	*	Mean:	•
Pool Depth to Bankfull Mean Depth (dpool/dbkf)	Range:		Range:		Range:		Range:		Range:	
	Mean:	6.1	Mean:	6.1	Mean:	*	Mean:	*	Mean:	n: * ge: n: * ge: n: *
26. Pool Width (Wpool) ft	Range;		Range:		Range:		Range:		Range:	
27. Ratio of Pool Width to	Mean:	0.5	Mean:	0.5	Mean:	*	Mean:	*	Mean:	•
Bankfull Width (Wpool/Wbkf)	Range:		Range:		Range:		Range:		Range:	
28. Bankfull Cross-	Mean:	9.5	Mean:	9.5	Mean:	*	Mean:	*	Mean:	-
sectional Area at Pool (Apool) sq ft	Range:		Range:		Range:		Range:		Range:	
	Mean:	0.6	Mean:	0.6	Mean:	*	Mean:	*	Mean:	,
29. Ratio of Pool Area to 3ankfull Area (Apool/Abkf)	Range:		Range:		Range:		Range:		Range:	
	Mean:	34.1	Mean:	34.1	Mean:	*	Mean:	*	Mean:	,
30. Pool to Pool Spacing [[p-p] ft	Range:	23.4 - 54.8	Range:	23.4 - 54.8	Range:		Range:		Range:	
31. Ratio of Pool-to-Pool	Mean:	2.8	Mean:	2.8	Mean:	*	Mean:	*	Mean:	,
Spacing to Bankfull Width (p-p/Wbkf)	Range:	2.0 - 4.6	Range:	2.0 - 4.6	Range:		Range:		Range:	
	Mean:	18.2	Mean:	18.2	Mean:	*	Mean:	*	Mean:	,
32. Pool Length (Lp) ft	Range:	4.0 - 163.0	Range:	4.0 - 163.0	Range:		Range:		Range:	
33. Ratio of Pool Length	Mean:	1.5	Mean:	1.5	Mean:	*	Mean:	*	Mean:	,
to Bankfull Width (Lp/Wbkf)	Range:	0.3 - 13.6	Range:	0.3 - 13.6	Range:		Range:		Range:	
	Mean:	0.0252	Mean:	0.0252	Mean:	*	Mean:	*	Mean:	1
34. Riffle Slope (Sriff) ft / ft	Range:	0.0145 - 0.0498	Range:	0.0145 - 0.0498	Range:		Range:		Range:	
35. Ratio of Riffle Slope	Mean:	2.6	Mean:	2.6	Mean:	*	Mean:	*	Mean:	
to Average Slope (Sriff/Sbkf)	Range:	1.5 - 5.2	Range:	1,5 - 5.2	Range:		Range:		Range:	
	Mean:	2.0	Mean:	2.0	Mean:	*	Mean:	*	Mean:	,
36. Maximum Riffle Depth (driff) ft	Range:	2.0-2.1	Range:	2.0-2.1	Range:		Range:		Range:	
	Mean:	1.6	Mean:	1.6	Mean:	*	Mean:	*	Mean:	
Ratio of Riffle Depth			Denser	1.5 - 1.6	Range:		Range:		Range:	
37. Ratio of Riffle Depth o Bankfull Mean Depth (driff/dbkf)	Range:	1.5 - 1.6	Range:	1.0 - 1.0	rtange.		munge.			
o Bankfull Mean Depth	Range: Mean:	1.5 - 1.6 N/A	Kange: Mean:	N/A	Mean:	*	Mean:	*	Mean:	*

.

39. Ratio of Run Slope to Average Slope	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
(Srun/Sbkf)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
40. Maximum Run Depth	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
(drun) ft	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
41. Ratio of Run Depth to Bankfull Mean Depth	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
(drun/dbkf)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
42. Slope of Glide (Sgl) ft	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
/ ft	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
43. Ratio of Glide Slope to Average Water Surface	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
Slope (Sgl/Sws)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
44. Maximum Glide	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
Depth (dgl) ft	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
45. Ratio of Glide Depth to Bankfull Mean Depth	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
(dgl/dbkf)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
46. Step Slope (Sst)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
47. Ratio of Step Slope to	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
Average Water Surface Slope (Sst/Savg)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
49 Maximum Stan Danth	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
48. Maximum Step Depth (dst)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
49. Ratio of Step Depth to	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
Bankfull Mean Depth (dst/dbkf)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	

* Geomorphic data not collected due to existing design constraints. The majority of the work on site will consist of Enhancement I. Ranges for restoration areas were developed around the existing site constraints.

Materials:

Particle Size Distribution of Channel Material (mm)			
D16		0.2	
D35		0.5	
D50		3.4	
D84		19.0	
D95 Particle Size Distribution of Bar Material	Ρ	53.0 SP	
D16	, 7,6	01	<2
D35	12.4		4.5
D50	17.8		10.0
D84	40.7		54.4
D95 Largest Size Particle on	55.6		96.7
Bar	48.0		

*	*	*
*	*	*
*	*	*
*	*	*
*	*	*
*	*	*
*	*	*
*	*	*
*	*	*
*	*	×
*	*	*

* Geomorphic data not collected due to existing design constraints. The majority of the work on site will consist of Enhancement I. Ranges for restoration areas were developed around the existing site constraints.

Sediment Transport: Sediment Transport Validation (Based on Bankfull Shear Stress)	Existing	Proposed
Calculated value (mm) from curve	55	55
Value from Shields Curve (lb/ft2)	0.71	0.71
Critical dimensionless shear stress	0.016	0.016
Minimal mean dbkf (ft) calculated using critical dimensionless shear stress equations	0.4	0.4



Table 5a. Substrate Particle Size Distribution of Little Alamance Creek

Pebble Count,	Little Alamance Cr & UT		Burlington	Note: Pebble Count for UT 85% Pool, 15% Riffle		Pebble Count, Little Alamance Cr & UT																0.01 0.1 10 10 1000 10000 1	Particle Size (mm)	•	Size percent less than (mm) Percent by substrate type	D35 D50 D84 D95 silt/clay sand gravel cobble boulder bedrock	
		#	#	#	#		100%	# OO%	*	# 80%	191 191	H L EOW	i S Jau	# [E 20%	# 60%	and Second	ξ ν	# 20%	# 10%	/80 #	5	#	#	#		D16	0.153
	Count	# #	10 # #	*	10 # #	#	# # #	# # 7	#	#	#	# #	#	3 #	#	#	# #	#	#	#	# #	#		#	# #	19 #	100
	Ö		÷	10	Ť	с,	4	7	4	6 	4	2	ŝ		m	8	2										
	€ (mm)	0.062	0.13	0.25	0.5	-	2	4	9	ω	11	16	22	32	45	64	6	128	180	256	362	512	1024	2048	4096		cle Coun
	Size Range (mm)	0	0.062	0.13	0.25	0.5	-	2	4	9	8	11	16	22	32	45	64	06	128	180	256	362	512	1024	2048		Total Particle Count-
Pebble Count	Material	silt/clay	very fine sand	fine sand	medium sand	coarse sand	very coarse sand	very fine gravel	fine gravel	fine gravel	medium gravel	medium gravel	coarse gravel	coarse gravel	very coarse gravel	very coarse gravel	smail cobble	medium cobble	large cobble	very large cobble	small boulder	small boulder	medium boulder	large boulder	very large boulder	bedrock	



Table 6a. Pavement and Sub-pavement Particle Size Distribution of Little Alamance Creek





29.75 99.61 118.35 NA

Table 6b. Pavement and Sub-pavement Particle Size Distribution of UT to Little Alamance Creek




Map Unit	Soil Series	Slope	Drainage	General Characteristics
CbC2	Cecil fine sandy loam	2-6%	Well drained	Cecil series consists of very deep, moderately permeable soils on ridges and side slopes of the Piedmont uplands. They are deep to saprolite and very deep to bedrock.
EdB2	Enon fine sandy loam	2-6%	Well drained	Enon Series consists of very deep, slowly permeable soils on ridge tops and side slopes in the Piedmont.
LbB2	Lloyd loam	2-6%	Well drained	Lloyd series consists of very deep, moderately permeable soils on uplands in the Southern Piedmont.
Ur	Urban land			Consists of areas more than 85 percent of which are covered with street, buildings of all types, parking lots, railroad yards, and airports. The natural soils were greatly altered by cutting, filling, grading and shaping during the processes of urbanization. The original landscape, topography, and commonly the drainage pattern have been changed.

Table 7. Soils Summary of the Little Alamance Creek Stream Restoration Project, Alamance County

Source: NRCS Official Soil Series Descriptions (OSD) webpage

Common Name	Scientific Name	Federal State Status Status		Habitat Requirements	Habitat Available	
Vertebrates						
American Eel	Anguilla rostrata	FSC	None	Catadramous species, adults live in large rivers or lakes	No	
Carolina darter	Etheostoma collis lepidinion	FSC	None	Small upland creeks with slow to moderate current and substrate of sand, gravel, or bedrock	Yes	
Invertebrates						
Carolina creekshell	Villosa vaughaniana	FSC	Е	Muddy or silty gravel in shallow waters	Yes	
Yellow lampmussel	Lampsilis cariosa	FSC	Е	Large rivers and streams found in sand and gravel	No	
Vascular Plants						
Buttercup phacelia	Phacelia covillei	FSC	SR-T	Floodplains and adjacent forests	Yes	
Sweet pinesap	Monotropsis odorata	FSC	SR-T	Pine woods	No	

Table 8. Federal Species of Concern Known from Alamance County, North Carolina

Status: E – Endangered FSC – Federal species of concern PE – Proposed endangered SR – Significantly rare -T – Throughout their range

Table 9. Project Restoration Structure and Objectives,Project Number 060685501 (Little Alamance Creek)

Restoration Segment / Reach ID	Station Range	Restoration Type	Priority Approach	Existing Linear Footage or Acreage	Designed Linear Footage or Acreage	Comment
Reach I	10+00 – 27+83	Enhancement I	P2	1,783 lf	1,783 lf	Modifying channel dimension and profile with structure placement.
Reach II	27+83 – 29+45	Restoration	P2	160 lf	162 lf	Reach II consists of realigning the stream and removing a center bar.
Reach II	29+45- 36+32	Enhancement I	P2	687	687 lf	Modifying channel dimension and profile with structure placement.
Reach I - Trib	10+00 – 14+22	Enhancement I	P2	422	422 lf	Modifying channel dimension and profile with structure placement.

Figures









Appendix A

Project Site Photographs



Little Alamance Creek looking downstream from S Church Street. Note pedestrian bridge.



Little Alamance Creek at the confluence with the Unnamed Tributary



One of several culverts entering Little Alamance Creek. Note Rip rap banks.



One of two train trestles crossing Little Alamance Creek.



Second train trestle crossing Little Alamance Creek. Pedestrian/vehicle bridge can be seen in the background.



Sewer line and pedestrian/vehicle bridge crossing Little Alamance Creek



Little Alamance Creek with center bar.



Second sewer line crossing Little Alamance Creek.



Typical section of Little Alamance Creek.



Little Alamance Creek downstream end of project at Overbrook Road.



Unnamed tributary to Little Alamance Creek.

Appendix B

Project Site NCDWQ Stream Classification Forms

North Carolina Division of Water Quality – Stream Identification Form; Version 3.1

Date: 4/2/07	Project: Li Hle Alamance Li	_ Latitude:
	Site: C.t. Park	Longitude:
Total Points: Stream is at least intermittent if \geq 19 or perennial if \geq 30 \checkmark 7.5	County: Alamanae	Other Burling ton

A. Geomorphology (Subtotal = 26.5)	Absent	Weak	Moderate	Strong
1 ^a . Continuous bed and bank	0	1	2	3
2. Sinuosity	0	- 1	2	3
3. In-channel structure: riffle-pool sequence	0	1	Ø	3
4. Soil texture or stream substrate sorting	0	1	Ð	3
5. Active/relic floodplain	0	1	2	Ð
6. Depositional bars or benches	0	1	2	3
7. Braided channel	0	Ð	2	3
8. Recent alluvial deposits	0	1	2	3
9 ^a Natural levees	0	Ð	2	3
10. Headcuts	Ø	1	2	3
11. Grade controls	0	0.5	Ð	1.5
12. Natural valley or drainageway	0	0.5	1	(1.5)
13. Second or greater order channel on <u>existing</u> USGS or NRCS map or other documented evidence.	No	No = 0 Yes = 3		= 3)

^a Man-made ditches are not rated; see discussions in manual

B. Hydrology (Subtotal = / 2)

14. Groundwater flow/discharge	0	1	2	<u>O</u>
 Water in channel and > 48 hrs since rain, <u>or</u> Water in channel dry or growing season 	0	1	2	3
16. Leaflitter	(1.5)	1	0.5	0
17. Sediment on plants or debris	0	0.5	Ð	1.5
18. Organic debris lines or piles (Wrack lines)	0	0.5	1	(5)
19. Hydric soils (redoximorphic features) present?	NO	=0	Yes = 1.5	

C. Biology (Subtotal = _____)

20 ^b . Fibrous roots in channel	3	2	1	0
21 ^b . Rooted plants in channel	3	2	1	0
22. Crayfish	0	0.5	1	1.5
23. Bivalves	Ø	1	2	3
24. Fish	0	0.5	1	(1.5)
25. Amphibians	0	0.5	CO.	(15)
26. Macrobenthos (note diversity and abundance)	0	0.5	Ø	1.5
27. Filamentous algae; periphyton	Ø	1	2	3
28. Iron oxidizing bacteria/fungus.	Ø	0.5	1	1.5
29 ^b . Wetland plants in streambed	FAC = 0.5, FA	CW = 0.75; OB	L = 1.5 SAV = 2	2.0; Other = 0

^b Items 20 and 21 focus on the presence of upland plants, Item 29 focuses on the presence of aquatic or wetland plants.

Notes: (use back side of this form for additional notes.)

Sketch:

itles observed in channel. + tu For macro benthe _____ locations. in 142 Scural

North Carolina Division of Water Quality – Stream Identification Form; Version 3.1

Date: 4 /4 /0>	Project: UT to Lifle Alemanee	Latitude:
Evaluator: RSC, BNF	Site: C.ty Park	Longitude:
Total Points:	County: Algmance	Other Bur ling ton e.g. Quad Name:

A. Geomorphology (Subtotal = 19.5)	Absent	Weak	Moderate	Strong
1 ^a . Continuous bed and bank	0	1	2	I
2. Sinuosity	0	1	\mathcal{O}	3
3. In-channel structure: riffle-pool sequence	0	1	Ø	3
4. Soil texture or stream substrate sorting	0	1	0	3
5. Active/relic floodplain	0	1	Ø	3
6. Depositional bars or benches	0	1	Ð	3
7. Braided channel	Ô	1	2	3
8. Recent alluvial deposits	0	1	ð	3
9 ^a Natural levees	0	Ð	2	3
10. Headcuts	0	Ð	2	3
11. Grade controls	0	0.5	2	1.5
12. Natural valley or drainageway	0	0.5	1	(15)
13. Second or greater order channel on <u>existing</u> USGS or NRCS map or other documented evidence.	No	= 0	Yes = 3	

^a Man-made ditches are not rated; see discussions in manual

B. Hydrology (Subtotal = 7.5)

14. Groundwater flow/discharge	0		2	3
 Water in channel and > 48 hrs since rain, <u>or</u> Water in channel dry or growing season 	0	1	2	3
16. Leaflitter	1.5	1	0.5	0
17. Sediment on plants or debris	0	0.5	Ð	1.5
18. Organic debris lines or piles (Wrack lines)	0	0.5	Ð	1.5
19. Hydric soils (redoximorphic features) present?	No	= 0)	Yes = 1.5	

C. Biology (Subtotal = ____ 6)

20 ^b . Fibrous roots in channel	3	Q	1	0
21 ^b . Rooted plants in channel	3	Ð	1	0
22. Crayfish	0	0.5	1	1.5
23. Bivalves	Ø	1	2	3
24. Fish	0	0.5	1	1.5
25. Amphibians	0	0.5	1	1.5
26. Macrobenthos (note diversity and abundance)	0	0.5	1	1.5
27. Filamentous algae; periphyton	Ø	1	2	3
28. Iron oxidizing bacteria/fungus.		0.5	1	1.5
29 ^b . Wetland plants in streambed	FAC = 0.5; FA	CW = 0.75; OB	L = 1.5 SAV = 2	.0; Other = 0

^b Items 20 and 21 focus on the presence of upland plants, Item 29 focuses on the presence of aquatic or wetland plants.

Notes: (use back side of this form for additional notes.)

Sketch:

culvert. 1.ack Stra Der ns a be t velity Fish benthos ater

Appendix C

BEHI and Sediment Export Estimates

Ĵ,	4			Righ	rt Ban	K			
	Summary Form - ANNUAL STREAMBANK EROSION - Estimates / Calculations :Stream: Little Alamance G Observers: BNF Date: 7/18/0-								07
)	Stream Type: 4E5/1 Graph Used: NC Total BANK L						igth: 1,76C		
	l t e m	(1) Station Ft.	(2) BEHI (adjective)	(3) NEAR-BANK STRESS (adjective)	(4) BANK EROSION RATE (Ft/yr)	(5) LENGTH of Bank (Ft)	(6) Bank HEIGHT (Ft)	(7) EROSION SubTotal (4)x(5)x(6) (Ft ³ / yr)	· · · · · · · · · · · · · · · · · · ·
•		10+75	VH	L	,6	65	4.5	175,5	1 -
}	2	11+40 12+00	H	L	,11	60	4,0	26,4	
	3	12+85	M	L	,017	185	3,5	11.0	-
w/ut ->	4	13+85	M	H	, 1	35	4.0	14,0	
+	5	14+35 14+60	L	_/M	.01	132	4,0	10	
)	6	15+15	<u>H</u>	<u>_M</u>	,16	55	14,0	35.2	1
	7	16+75	<u>H</u>		11	160	3,5	61.6	
)	8	17+75 17+75	<u> </u>	Ext.	,38	100	6.0	220	
)	9 + 10	19+00	H M	<u> </u>	-1d	120	40	130	:
I	11	21400	H	<u> </u>	. 17	115	50	67 5	4
r	12	22+15	1/4	Fut	1.6	55	6.0	\$16	1
	13	23700	//		617	50	+ 4.0	3,4	
	14	23+50	VH ,	M	.75	50	4.5	168.8	1
		341206	H	L		45	40	19,8	
	16	24145	VH	M	.75	135	4,5	455,6	\leftarrow
	17	25+80	M	Ľ	,017	130	3.5	7.7	
	18	27+10	VH	M	,75	140	5,0	525	\leftarrow
	Sum (Feet ³ /Year) EROSION <u>Sub-Totals</u> for each BEHI / NBS Combination Convert EROSION (Feet ³ / Year) to (Yards ³ / Year) { divide Total EROSION (Feet ³ /Year) by 27 }						Total Erosion = Feet ³ / Year	2,775.9	
							Total Erosion = $/ \bigcirc \bigcirc_1 & \bigcirc$ Yards ³ /Year		
) to (Tons / Yea ³ / Year) by 1.3		Total Erosion = Tons / Year	133,7	
	{D				IGTH of Channe gth of CHANNEL	_	Total Erosion = Tons / Yr / Ft	0,076	

Right Benk

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am Type: 6	: Alamance IF5/1		9 N /	Total BANK Len	<u>Date:</u> gth: 7 <u>4</u> 5	
(1)	(2)	(3)	(4)	(5)	(6)	, (7)
Station Ft.	BEHI (adjective)	NEAR-BANK STRESS (adjective)	BANK EROSION RATE (Ft/yr)	LENGTH of Bank (Ft)	Bank HEIGHT (Ft)	EROSION SubTotal (4)x(5)x(6) (Ft ³ /yr)
28+50	Ext	Ext	8	65	5.0	2,600
29+15	VH	H	,45	60	5.0	285
24+75	Ext	H	3.5	125	5,0	2,375
31+29	14	H	.2	35	5.0	35
31+35	M	L	,017	35	3.5	2.1
31 470	Ext	H	3.8	105	5,0	1,995
32+75 34400	VH	M	,75	125	4.0	375
34400	Н	L	.11	235	3.5	90,5
		1		,	1	* * *
					• • •	
			· · · · · · · · · · · · · · · · · · ·		<u> </u>	
				Anti-supa i ana		
		1	• • • • • • • • • • • • • • • • • • •		: 	
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					1 1 2 2	· · ·
	ļ 				· ·	
مرور و و و و و و و و و و و و و				•		
m (Feet ³ /Ye	ear) EROSION <u>S</u>	<i>ub-Totals</i> for ea	ach BEHI / NBS	Combination	Total Erosion = Feet ³ / Year	7,757,6
	nvert EROSION { divide Total E	г)	Total Erosion = Yards ³ / Year	287.3		
) to (Tons / Year ³ / Year) by 1.3		Total - Erosion = Tons / Year	373.5
		•	GTH of Channe gth of CHANNEL		Total Erosion = Tons / Yr / Ft	0,476

2	5		Bank		aa / Calculatio	200	
Sumn Stream: / i`t+le		CC CO. Observe	AMBANK EROS	SION - Estimat	Date:	-1.11 In	7
Stream Type:	- 1	Graph Used: /		Total BANK Len	ICMA		•
<u><u>(1)</u></u>	(2)	(3)	(4)	(5)	(6)	(7)	-
l Station t Ft.	BEHI	NEAR-BANK STRESS (adjective)	BANK EROSION RATE (Ft/yr)	LENGTH of Bank (Ft)	Bank HEIGHT (Ft)	EROSION SubTotal (4)x(5)x(6) (Ft ³ /yr)	
m 1 10+20 1 12+40	H			220	+-4.0	96,8	- - -
2 12+40	Ext	- -	3,4	20	5,0	380	<
3 12+60	VH	M	.75	115	4,5	388.1	\leftarrow
4 13+75	Н	M	.16	25	4.5	18	1
5 14 100	$\square \mathcal{M}$	L	,017	115	4.0	7.8	
6 15+15 16+80	H	L	,1/	165	4.0	72.6	i -i
7 16+80 12+75	H	Ext,	,38	95_	5.5	198.6	
8 17+75	H	<u> </u>	12	110_	5.5	121	· • •
9 13+85 19+30	VH	H	-45-	45	5,0	51318	
10 19+30	H .			150	<u>4,0</u>	66	-
11 21+23	<u>H</u>	<u> </u>	, 2	43	15.0	43	:
12 21+70	\mathcal{M}	L	_,017_	4/	+ $+$ $ +$ $+$ $ +$ $+$ $+$ $ +$ $+$ $+$ $+$ $+$ $+$ $+$ $+$ $+$ $+$	3,2	;
$\frac{13}{22+35}$	<u> </u>	Ext,	.38	65	5.0	123.5	. <i>y</i>
$\frac{14}{23 + 30}$	H	Ext	16	- 40	610	-712 - 712	K
15 24 +00	H	-11	16	$\frac{10}{10}$	5.0	56	
10 24+40	VH AA	 		40		190	8
18 25+00	11		017	100	4.0	6,8	•
26700					Total	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	
Sum (Feet ³ /Ye	ar) EROSION <u>S</u>	<u>ub-Totals</u> for e	ach BEHI / NBS	Combination	Erosion = Feet ³ /Year	2,901.5	>
	overt EROSION divide Total E	Total Erosion = Yards ³ / Year	107.5				
	nvert EROSION nultiply Total EF		Total Erosion = Tons / Year	139,7			
Cale {Divide Total ER			IGTH of Channe gth of CHANNEL		Total Erosion = Tons / Yr / Ft	0,088	

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			Left	Bank							
	Sum	nary Form - A	NNUAL STREA	AMBANK EROS	ION - Estimat	es / Calculatio	ns				
Stre	Stream: Little Alamance Cr. Observers: BNF Date: 7/18/07										
Stre	eam Type: 🧹	the constitute programment. And	Graph Used:	an Maria and Anna and	Total BANK Ler		Ft.	i			
:	<u>(1)</u>	<u>(2)</u>	(3)	(4) BANK	(5) LENGTH	(6)	EROSION				
:] . t	Station	BEHI	NEAR-BANK STRESS	EROSION	of	Bank HEIGHT	SubTotal (4)x(5)x(6)	:			
.e m	Ft.	(adjective)	(adjective)	(Ft/yr)	Bank (Ft)	(Ft)	(Ft ³ /yr)	÷			
= 1	26700	H	H	.2	120	4.5	108	: : •			
2	27+20	M	H		37	5,6	18.5				
3	27+57	VH	H	.95	73	5.0	370,5	\leq			
4	28+35	M	<u> </u>	, 1	95	50	47.5				
5	31.700	M	L	. 617	170	4.0	11,6				
6	31+45	<u> </u>	H	12	145	14.0	_116				
7	33+25	4	4	.00	230	3,5	0,8				
8	34+80	<u>H</u>	VH	128	105	10.0	294	\leftarrow			
9	36+35	\mathcal{M}		,017	155	4.0	10,5				
10		· 	· · · · · · · ·								
11	5 	 		es	· · · · · · · · · ·	<u> </u>		i I			
12	}• ••• • •••) 	·			*					
: 13 14	• • • • • • • • • • • • •	; ; ;									
· 15		! 	, 	· · · · · · · · · ·							
16		1977-21 C. P. Manual I	L	••·····	· · · · · · · · · · · · · · · · · · ·						
17		- · · · · · ·	• • • • • • • • • • • • • • •								
18	lan igana iya in tinana ang	i									
S	um (Feet ³ /Ye	ar) EROSION <u>S</u>	<u>Sub-Totals</u> for e	ach BEHI / NBS	Combination	Total Erosion = Feet ³ /Year	977,4				
: : :		{ divide Total E	ROSION (Feet			Total Erosion = Yards ³ /Year	362				
- - - -) to (Tons / Yea s ³ / Year) by 1.3		Total Erosion = Tons / Year	47./				
; ; ; {C				IGTH of Channe igth of CHANNEL		Total Erosion = Tons / Yr / Ft	0.041				

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Estimating Near-Bank Stress (NBS)										
Stream	: / :+	Fle Alam								
Observe		NF, J				••••••••••••••••••••••••••••••••••••••		Date: 5	121/01	
				FOR ESTIN	ATING NE	AR-BANK	STRESS		/	
(1) Tran	sverse ba	r or split chann						Recon	aissance	
		n (Rc/W)						General Prediction		
(3) Ratio	of pool s	lope to average	water surface	e slope (S _p / S	;)	···· ··· ··· · · · · · · · · · · · · ·	Level II	General	Prediction	
		lope to riffle slo						General	Prediction	
(5) Ratio	of near-b	ank maximum	depth to bank	full mean dept	h (d _{nb} /d _{bkf}).	· ····	Level III	Detailed	Prediction	
(6) Ratio	of near-b	ank shear stre	ss to bankfull s	shear stress (τ _{nb} / τ _{bkf})	· · · · · · · · · · · · · · · · · · ·	Level III	Detailed	Prediction	
		s / Isovels / Ve					Level IV	Vali	dation	
Т							·····	NBS = Hig	h / Very High	
Level	(1)					-			3S = Extreme	
Fe				neander migra			#1)	NE	3S = Extreme	
		Radius of Curvature	Bankfull Width		Near-Bank Stress					
	(2)	Rc (feet)	W _{bkf} (feet)	Ratio Rc / W	NBS		•			
			Mu (1011		Ì				
					Near-Bank					
П		Pool Slope	Average Slope	Ratio	Stress		Dom	inant]	
Level II	(3)	Sp	S	Sp / S	NBS		Near-Ba	nk Stress		
							0	\mathcal{M}		
					Near-Bank		·		-	
	(4)	Pool Slope	Riffle Slope		Stress					
	(*)	Sp	Srif	Sp / Srif	NBS					
]				
		Near Bank	Mean	Deffe	Near-Bank Stress					
	(5)	Max Depth d _{nb} (feet)	Depth d (feet)	<i>Ratio</i> d _{nb} / d	NBS					
н		3.5	2.6	1.3	low	1				
III e		5.5	×10	Near-Bank			Bankfull Shear			
Level		Near Bank Max Depth	Near Bank Slope	Shear Stress	Mean	Average	Stress	Ratio	Near-Bank Stress	
	(6)	d _{nb} (feet)	S _{nb}	τ_{nb} (lb/ft ²)	Depth d (feet)	Stope S	τ(lb/ft ²)	τ_{nb}/τ	NBS	
		-110 (4			- (_				
				Near-Bank						
Level IV	(7)	Velocity (ft/s		Stress NBS						
e ((1/3	<u>, , , , , , , , , , , , , , , , , , , </u>							
		I		Conve	rting Values	to a Near-Ba	ank Stress R	ATING		
Moor Do	nk Stran			,	-	ethod Numb				
пеаг-ра	nk Suest	s RATINGS ;	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
	Very L	w	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 1.00	
	Low	>	N/A	2.21 - 3.00	0.20 - 0.40	0.41 - 0.60	1.00 - 1.50	0.80 - 1.05	1.00 - 1.20	
	Modera	ite	N/A	2.01 - 2.20	0.41 - 0.60	0.61 - 0.80	1.51 - 1.80	1.06 - 1.14	1.21 - 1.60	
	High	· · · · · · ·	See	1.81 - 2.00	0.61 - 0.80	0.81 - 1.00	1.81 - 2.50	1.15 - 1.19	1.61 - 2.00	
· •- · ·	Very Hi	gh	(1)	1.50 - 1.80	0.81 - 1.00	1.01 - 1.20	2.51 - 3.00	1.20 - 1.60	2.01 - 2.30	
	Extrem	ie "	Above	< 1.50	> 1.00 _	> 1.20	> 3.00	> 1.60	> 2.30	
				Ove	erall Near-E	Bank Stres	s RATING			

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			Estima	ting Near	-Bank St	ress (N	IBS)		
Stream		He Alav							·
Observe		NF. JE				••••••		Date: 5	5/21/07
				FOR ESTIN	ATING NE	AR-BANK	STRESS		<u>7 a 7 o 1</u>
(1) Tran	sverse bai	r or split chann					and a state of the second s	Recor	naissance
		n (Rc/W)							i Prediction
(3) Ratio	n of pool sl	ope to average	e water surface		Level II		Prediction		
(4) Ratio	n of nool sl	one to riffle sk	$ne/S/S_{-1}$				Level II	e anno 1975 - 1987 - 1987 - 1987 - 1987 - 1987 - 1987 - 1987 - 1987 - 1987 - 1987 - 1987 - 1987 - 1987 - 1987 -	Prediction
		ope to riffle slo							I Prediction
		ank maximum					• • • • • • • • • • • • • • • • • • •	<u></u>	Prediction
		ank shear stre					•••		
(7) Velo	city prome:	s / Isovels / Ve					3		idation h / Very High
ы Б	(1)								BS = Extreme
Level									BS = Extreme
	<u> </u>	Radius of	Bankfull		Near-Bank				
İ		Curvature	Width	Ratio	Stress				
	(2)	Rc (feet)	W _{bkf} (feet)	Rc / W	NBS				
- Level		[Average	1	Near-Bank	i .			_
Levei II	(3)	Pool Slope	Slope	Ratio	Stress		1	inant	
Lev		Sp	<u> </u>	Sp/S	NBS	1	1	nk Stress	-
					<u> </u>	ļ	101	\sim	
					Near-Bank				
	(4)	Pool Slope Sp	Riffle Slope Srif	Ratio Sp / Srif	Stress NBS				
		Op	0m	op/on		1			
	(Near Bank			Near-Bank	l T			
		Max Depth	Mean Depth	Ratio	Stress				
	(5)	d _{nb} (feet)	d (feet)	d _{nb} /d	NBS		•		
H		4.0	2.6	1.5	Jow				
				Near-Bank		 	Bankfull Shear		
Level		Near Bank Max Depth	Near Bank Slope	Shear Stress	Mean Depth	Average Slope	Stress	Ratio	Near-Bank Stress
	(6)	d _{nb} (feet)	S _{nb}	$\tau_{\rm nb}$ (lb/ft ²)	d (feet)	Siope	τ(lb/ft ²)	τ_{nb}/τ	NBS
	1			Sub (Invit)					
							<u> </u>		<u> </u>
			-	Near-Bank					
Level IV	(7)	Velocity (ft/s		Stress NBS					
Lev		(1/2	,,,,,						
		l <u></u>		Convo	ting Values	to a Naar P	ank Stress RA	TING	
				, COINE	=	ethod Numb			
wear-Ba	ink Stress	RATINGS :	(1)	(2)	(3)	(4)	(5)	(6)	<u> (7) </u>
					< 0.20	< 0.40	< 1.00	< 0.80	< 1.00
·····	Very Lo	w	N/A	> 3.00	· 0.20				
	Very Lo	w	N/A N/A		0.20 - 0.40	0.41 - 0.60	1.00 - 1.50	0.80 - 1.05	1.00 - 1.20
· · · ·	······································	>	·····	> 3.00 2.21 - 3.00 2.01 - 2.20		0.41 - 0.60 0.61 - 0.80	1.00 - 1.50		1.00 - 1.20 1.21 - 1.60
· · · · ·	Low	>	N/A	2.21 - 3.00 2.01 - 2.20	0.20 - 0.40	0.61 - 0.80	and the second se	0.80 - 1.05	1.21 - 1.60
· · · ·	Low Modera High) te	N/A N/A	2.21 - 3.00 2.01 - 2.20 1.81 - 2.00	0.20 - 0.40 0.41 - 0.60	0.61 - 0.80 0.81 - 1.00	1.51 - 1.80	0.80 - 1.05 1.06 - 1.14	
· · · ·	Low Modera) te gh	N/A N/A See	2.21 - 3.00 2.01 - 2.20	0.20 - 0.40 0.41 - 0.60 0.61 - 0.80	0.61 - 0.80 0.81 - 1.00	1.51 - 1.80 1.81 - 2.50	0.80 - 1.05 1.06 - 1.14 1.15 - 1.19	1.21 - 1.60 1.61 - 2.00

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-		<u> </u>		Estimat	ing Near-	Bank Str	ess (N	BS)	<u></u>	
Stre	eam:	Lit	He Al	am.	Location	: 1440	10 - 14	720		
Obs	erver	s: <u>B</u> r	VF. TI	<u>st</u>					Date: 5-	21-07
	155 .055	e e marte a fuita.				ATING NE				
	Trans	verse bar	or split channe	el/central bar c	reating NBS/hi	igh velocity gra	adient	a sea consecutive sector sector sector sec		issance
			n (Rc/W)							Prediction
(3)	Ratio	of pool sid	ope to average	water surface	slope(S _p /S)	•••••••••••••••••••••••••••••••••••••••		· · · · · · · · · · · · · · · · · · ·	Prediction
and the second s			ope to riffle slo							Prediction
A			ank maximum						Detailed I	Prediction
			ank shear stres							Prediction
(7)	Veloc	ity profiles	s / Isovels / Vel					<u></u>		ation
	= (
								 ¢1)		
	<u></u>		Radius of	Bankfull		Near-Bank		,		
			Curvature	Width	Ratio	Stress				
		(2)	Rc (feet)	W _{bkf} (feet)	Rc/W	NBS				
	_			Average		Near-Bank			<u> </u>	1
	Level 11	(3)	Pool Slope	Slope	Ratio	Stress NBS		Dom Near-Bar		
-	Lev		Sp	<u> </u>	Sp / S			112	IK OIIE35	
			<u> </u>		,	Near-Bank		L TRIQ	η	1
		-	Pool Sione	Riffle Slope	Ratio	Stress		\sim		
	i	(4)	Sp	Srif	Sp / Srif	NBS				
			Near Bank	Mean		Near-Bank	Í			
		(5)	Max Depth	Depth	Ratio	Stress				
		(\mathbf{J})	d _{nb} (feet)	d (feet)	d _{nb} / d	NBS				
	111						<u> </u>	1		
	Level			Near Bank	Near-Bank	Mean	Average	Bankfull Shear	Ratio	Near-Bank
	<u>.</u>	(6)	Max Depth	Slope	Shear Stress	Depth	Slope	Stress	τ_{nb}/τ	Stress NBS
		(0)	d _{nb} (feet)	S _{nb}	τ_{nb} (lb/ft ²)	d (feet)	S	τ (lb/ft ²)		NDO
,					Near-Bank					
		(7)		Gradient	Stress NBS					
	Level IV	(.)	(ft/:	s/ft)	601					
									ATINO	
				1	Conve	-	to a <i>Near-Ba</i> lethod Numb	ank Stress R. ver	ATING	
Ne	ar-Ba	nk Stres	s RATINGS	(1)	(2)	(3)	(4)	(5)	(6)	(7)
5 10 11		Very L	over an and the second s	N/A	, > 3.00	< 0.20	< 0.40	+ مُنْحَمَّ < 1.00	< 0.80	< 1.00
~		Low		N/A	2.21 - 3.00	0.20 - 0.40	0.41 - 0.60	1.00 - 1.50	0.80 - 1.05	1.00 - 1.20
.		Modera		N/A	2.01 - 2.20		0.61 - 0.80	1.51 - 1.80	1.06 - 1.14	1.21 - 1.60
1	<	High		See	1.81 - 2.00	0.61 - 0.80	0.81 - 1.00	1.81 - 2.50	1.15 - 1.19	1.61 - 2.00
Ĩ		Very Hi		(1)	1.50 - 1.80	0.81 - 1.00	· · · · · · · · · · · · · · · · · · ·	2.51 - 3.00	1.20 - 1.60	2.01 - 2.30
		Extren	CI.e	Above	, < 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.30
					Ov	erall N ear-l	Bank Stre	ss RATING		

G:\TheRAM3-2-2006\1st Field Day-SECTION A\ram06-PAGE-A 27-EstNearBankStress.xls

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			Estimati	ng Near-	Bank Stre	ess (NE	BS)			
Stream:	T:41	c Alam						173		
Observer	· A -	VF. JT			·			Date: 5/	21/07	
	METHODS FOR ESTIMATING NEAR-BANK STRESS									
(1) Trans	verse bar	or split channe	l/central bar cr	eating NBS/hi	gh velocity gra	dlenti	Level I	Reconai	ssance	
(2) Chan	nel pattern	(Rc/W)					Level II	General P	rediction	
(3) Ratio	of pool slo	ope to average	water surface	slope(S _p /S))		Level II	General P	rediction	
(4) Ratio	of pool sid	ope to riffle slop	oe (S _p /S _{rif})				Level II	General P	rediction	
		ank maximum o					Level III	Detailed F	rediction	
		ink shear stres					Level III	Detailed F	rediction	
		/ Isovels / Vel	ocity gradient				Level IV	Valid		
I		Transverse a						.NBS = High	/ Very High	
Level	(1)	Extensive de Chute cutoffs,	position (co	ntinuous, cro seendor migra	ss-channel)					
		Radius of	Bankfull	leander migra	Near-Bank					
		Curvature	Width	Ratio	Stress					
	(2)	Rc (feet)	W _{bkf} (feet)	Rc/W	NBS					
			Average		Near-Bank	,				
Level II	(3)	Pool Slope	Slope	Ratio	Stress NBS	-	Domi Near-Bar			
Leve	(0)	Sp	S	Sp/S			AL JO	0		
					Near-Bank		90000	rare		
		Deal Sland	Riffle Slope	Ratio	Stress		Hig	h		
	(4)	Pool Slope Sp	Srif	Sp / Srif	NBS			* H		
		Near Bank	Mean		Near-Bank					
	/=\	Max Depth	Depth	Ratio	Stress	1				
	(5)	d _{nb} (feet)	d (feet)	d _{nb} /d	NBS	High				
		6	GO.	QQ	Checky	<u>11.20</u>				
Level	ļ	Near Bank	Near Bank	Near-Bank	Mean	Average	Bankfull Shear Stress	Ratio	Near-Bank	
1	(6)	Max Depth	Slope	Shear Stress	Depth d (feet)	Siope S		$\tau_{\sf nb}/\tau$	Stress NBS	
		d _{nb} (feet)	S _{nb}	τ_{nb} (ib/ft ²)	u (leel)		τ(lb/ft ²)			
		<i>.</i>				ļ	L			
_ ►				Near-Bank						
Levei IV	(7)		Gradient	Stress NBS						
Leve		(π/	s/ft)		i					
	<u> </u>			Conve	I	to a Near-Ba	ank Stress R	ATING	··	
				5 COLLAR		lethod Numb				
Near-Bank Stress RATINGS (1)				(2)	(3)	(4)	(5)	(6)	[[7] .	
	Very L	.ow	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 1.00	
Ľ	Low	1	N/A	2.21 - 3.00	0.20 - 0.40	0.41 - 0.60	1.00 - 1.50	0.80 - 1.05	1.00 - 1.20	
-	Moder	Je)	N/A	2.01 - 2.20	0.41 - 0.60	0.61 - 0.80	1.51 - 1.80	1.06 - 1.14	1.21 - 1.60	
		ĭ)	See	1.81 - 2.00	0.61 - 0.80	0.81 - 1.00	and the second s	1.15 - 1.19	1.61 - 2.00	
.	Very F	ligh	(1)	1.50 - 1.80		alles a sector of the sector of	2.51 - 3.00	1.20 - 1.60	2.01 - 2.30	
	Extrer	me	Above	< 1.50	> 1.00	> 1.20	<u>; > 3.00</u>	> 1.60	> 2.30	
					rerali Near-	Bank Stre	ss rating			

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	<u> </u>		Estimat	ing Near	Bank Str	ress (N	BS)		
Stream	/ iHl	e Alamo						15	
	rs: A		ET			la Tatan () - Ala Tali sa		Date: 5/	21/07
			METHODS I	FOR ESTIM	IATING NE	AR-BANK S	TRESS		
(1) Tran	sverse bar	or split channe	el/central bar c	reating NBS/h	igh velocity gra	adient	Level I	Recona	issance
		n (Rc/W)					·	General	Prediction
(3) Ratio	o of pool sk	ope to average	water surface	slope (S _p / S)	•••••••		General I	Prediction
		ope to riffle slo						General	Prediction
(5) Ratio	o of near-ba	ank maximum (depth to bankf	uli mean depti	n (d _{nb} /d _{bkf})		Level III	Detailed	Prediction
(6) Ratio	of near-ba	ank shear stres	s to bankfull s	hear stress (1	ι _{nb} / τ _{bkf})		Level III	Detailed	Prediction
and the second second		s / Isovels / Vel					Level IV	Valio	lation
I								-	
Level	(1)		• •						
	I	Radius of	Bankfull	neanuer migra	Near-Bank	19 110W (1400 #	£1)		
		Curvature	Width	Ratio	Stress				
	(2)	Rc (feet)	W _{bkf} (feet)	Rc / W	NBS				
			Average		Near-Bank	· ·			1
Level II	(3)	Pool Slope	Slope	Ratio	Stress NBS		Dom Near-Bai		
Lev		Sp	<u> </u>	Sp / S				t	10
				·	Near-Bank		4-01-	Modero	10
		Pool Slope	Riffle Slope	Ratio	Stress		Hia	k	
	(4)	Sp	Srif	Sp / Srif	NBS			I V	
							\bigcirc		
		Near Bank	Mean		Near-Bank				
	(5)	Max Depth	Depth	Ratio	Stress NBS		é		
		d _{nb} (feet)	d (feet)	d _{nb} /d	Low	And A	hàh		
		5,5		Near-Bank	2000 9	·		_	1
Leve		Near Bank	Near Bank Slope	Near-Bank Shear Stress	Mean	Average	Bankfull Shear Stress	Ratio	Near-Bank
· ·	(6)	Max Depth d _{nb} (feet)	S _{nb}	τ_{nb} (lb/ft ²)	Depth d (feet)	Slope S	τ (lb/ft ²)	τ_{nb}/τ	Stress NBS
		-110 (_	U (IDAL)		
	[i			
		N / - 1 14	0	Near-Bank Stress					
Level IV	(7)		Gradient s / ft)	NBS					
Le,					1				
	1	<u> </u>	;	Conve	rting Values	to a Near-Ba	ank Stress R	ATING	
Near-B	ank Stres:		:	(N	lethod Numb	er		
	unan san mina	(1.100 - 1.101 - 1.100 - 1.100 - 1.100 - 1.100 - 1.100 - 1.100 - 1.100 - 1.100 - 1.100 - 1.100 - 1.100 - 1.100	(1)	(2)	(3)	(4)	(5)	(6)	$(\underline{0})$
	Very L	~	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 1.00
	Low	<u> </u>	N/A	2.21 - 3.00	· . · . · . · . · · · · · · · · · · · ·		(1.00-1.50)	0.80 - 1.05	1.00 - 1.20
1 2	Modere		N/A	2.01 - 2.20	0.41 - 0.60	0.61 - 0.80 .			1.21 - 1.60
<	High		See (1)	1.81 - 2.00	0.61 - 0.80	0.81 - 1.09		1.15 - 1.19	1.61 - 2.00
	Very Hi		(1) Above	1.50 - 1.80		1.01 - 1.20		1.20 - 1.60	2.01 - 2.30
	Extrem	ie :	Above	< 1.50	<u>> 1.00</u>	> 1.20	> 3.00	> 1.60	> 2.30
					eialintear-l		ss RATING		

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-	Estimating Near-Bank Stress (NBS)											
St	ream:	Litt	le Alan	iance	Location:	Sta 1	6+80.	-18+85	>			
O	oserver	s: BN	F,JT	-					Date: 5/	21/07		
F			N	ETHODS F	OR ESTIM	ATING NEA	R-BANK S	TRESS				
(1)	Trans	verse bar	or split channe	l/central bar cr	eating NBS/hi	gh velocity gra	dient	Level I	Reconai	ssance		
(2)		nei pattern	(Rc/W)		· · · · · · · · · · · · · · · · · · ·			Level II	General P			
(3)	Ratio	of pool slo	pe to average	water surface	slope(S _p /S)		Level II	General P			
(4)			pe to riffle slop					Level II	General P			
(5)	Ratio	of near-ba	nk maximum o	lepth to bankfu	Ill mean depth	(d _{nb} / d _{bkf})	· · · · · · · · · · · · · · · · · · ·	Level III	Detailed F			
(6)			ink shear stres						Detailed F			
(7)) Veloc	ity profiles	/ isoveis / Vel						Valid			
	Ι	(1)	Transverse a Extensive de						NBS = High. NBS	/ very Hign S ≃ Extreme		
	Level	(1)						1)				
-			Radius of	Bankfull		Near-Bank						
			Curvature	Width	Ratio	Stress						
		(2)	Rc (feet)	W _{bkf} (feet)	Rc/W	NBS						
			45	32	1.4	extre	ne					
	Ι			Average	.	Near-Bank Stress		Domi	nant			
	Level II	(3)	Pool Slope Sp	Slope S	<i>Ratio</i> Sp / S	NBS		Near-Bar				
	Le Le							extr	reme			
				2 x n, .		Near-Bank	; 					
•			Pool Slope	Riffle Slope	Ratio	Stress						
		(4)	Sp	Srif	Sp / Srif	NBS						
		(5)	Near Bank	Mean	Ratio	Near-Bank Stress						
			Max Depth d _{nb} (feet)	Depth d (feet)	d _{nb} /d	NBS	<u> </u>	•				
	II		-10 (· ·						
	el III		Neer Doold	Near Bank	Near-Bank	Meen	Average	Bankfull Shear	Ratio	Near-Bank		
	Lev		Near Bank Max Depth	Slope	Shear Stress	Mean Depth	Slope	Stress		Stress		
ł		(6)	d _{nb} (feet)	S _{nb}	τ_{nb} (lb/ft ²)	d (feet)	S	τ (lb/ft ²)	$ au_{\sf nb}$ / $ au$	NBS		
										[
		<u> </u>		<u> </u>	Near-Bank		L	<u>_</u>		L		
	VI		Velocity	Gradient	Stress							
	Level IV	(7)		s/ft)	NBS							
	ت۔ 	L			<u> </u>							
				\$ 2	Conve	-	to a <i>Near-Ba</i> Aethod Numb	ank Stress R.	ATING			
1	Near-B	ank Stres	s RATINGS	(1)	(2)	(3)	(4)	(5)	(6)	(7)		
÷.	Very Low				<u> </u>	< 0.20	< 0.40	< 1.00	< 0.80	< 1.00		
-		Low		N/A	2.21 - 3.00	0.20 - 0.40	0.41 - 0.60	1.00 - 1.50	0.80 - 1.05	1.00 - 1.20		
u .		Moder	and the second second	N/A	2.01 - 2.20	0.41 - 0.60	0.61 - 0.80	1.51 - 1.80	1.06 - 1.14	1.21 - 1.60		
ŀ		High		See	1.81 - 2.00	0.61 - 0.80	0.81 - 1.00	1.81 - 2.50	1.15 - 1.19	1.61 - 2.00		
ĩ		Very H		(1)	1.50 - 1.80	0.81 - 1.00	1.01 - 1.20	2.51 - 3.00	1.20 - 1.60	2.01 - 2.30		
-		Extrer	ne	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	, > 2.30		
-					0	erall Near-	Bank Stre	ss RATING	1			

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· · · · ·			Estima	ting Near	-Bank St	ress (N	BS)		
Stream:	Lity	He Alan	and the second se						
		17, 698			tiv ₩¥kuti v tu	•••••••••••••••		Date: 5/	121/07
		the second s		FOR ESTIM	ATING NE	AR-BANK S	STRESS		
(1) Trans	verse bar	or split channe						Recon	aissance
(2) Chan	nel patter	n (Rc/W)					.; Level II	General	Prediction
(3) Ratio	of pool sl	ope to average	water surface	slope(S _p /S)		, Level II	General	Prediction
		ope to riffle slo						General	Prediction
		ank maximum						Detailed	Prediction
		ank shear stre					. Level III	Detailed	Prediction
(7) Veloc	ity profile:	s / Isovels / Ve					<u></u>	<u> </u>	dation
I	(1)								h / Very High
Level	(1)						#1)		S = Extreme
		Radius of	Bankfull	ficander migre	Near-Bank				
	(0)	Curvature	Width	Ratio	Stress				
	(2)	Rc (feet)	W _{bkf} (feet)	Rc/W	NBS	ļ			
н			Average		Near-Bank			• /	1
Level II	(3)	Pool Slope	Slope S	Ratio	Stress NBS		1	<i>inant</i> nk S tress	
Le,	• •	Sp		Sp / S				Jerate	
		1		·	Near-Bank	J	////		ł
		Pool Slope	Riffle Slope	Ratio	Stress				
	(4)	Sp	Srif	Sp / Srif	NBS				
		Near Bank	Mean		Near-Bank				
	(5)	Max Depth d _{nb} (feet)	Depth	Ratio	Stress NBS		•		
ы		4,5	d (feet)	$\frac{d_{nb}}{d}$					
			<u>2,6</u>	Near-Bank	Mod	ļ	Bankfull Shear		
Leve		Near Bank Max Depth	Near Bank Slope	Shear Stress	Mean Depth	Average Slope	Stress	Ratio	Near-Bank Stress
-	(6)	d _{nb} (feet)	S _{nb}	$ au_{ m nb}$ (lb/ft ²)	d (feet)	S	τ(lb/ft ²)	$ au_{\sf nb}/ au$	NBS
				No on Double			L		
		Velocity	Gradiont	Near-Bank Stress					
Level IV	(7)	(ft/s		NBS					
Le Le									
		1		Conve	rting Values	to a Near-Ba	ank Stress R	ATING	
Near-Ba	nk Stress	RATINGS		1		ethod Numb	1	(0)	(7)
(1)				(2)	(3)	(4)	(5)	(6)	
• •	Very Lo	WC	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 1.00
•	Low	to	N/A N/A	2.21 - 3.00	0.20 - 0.40	0.41 - 0.60	1.00 - 1.50	0.80 - 1.05	1.00 - 1.20 1.21 - 1.60
	Modera		See	2.01 - 2.20 1.81 - 2.00	0.41 - 0.60	0.61 - 0.80	1.81 - 2.50	1.15 - 1.19	1.61 - 2.00
v .	High Very Hi	nh i	(1)	1.81 - 2.00	0.81 - 1.00		2.51 - 3.00	1.20 - 1.60	2.01 - 2.30
a	Extrem		Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.30
		ار				Bank Stres	·		

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River Restoration & Natural Channel Design

Worksheet C-3. Summary form to estimate annuals streambank erosion for various study reaches.

reaches. Stream: UT +	6 1:44 le	Alam (~	Location:	Burli	noton. 1	VC
Graph Used: Co				11-		Bank Date:	5-21-07
Observers: B)			Valley Type:		use would be	Stream Type:	
	(2) (2)	a (19) (3) a a a a	A (4) Second	(6)	Cturdu	(7) Erosion	(8) Erosion
Station (ft)	BEHI rating (Worksheet	NBS rating (Worksheet)	Bank erosion	Length of bank (ft)	Study bank	subtotal	Rate
	A-14)	A-15)	rate (Figure A-		height (ft)	[(4)×(5)×(6)]	(tons/yr/ft) {[(7)/27] ×
	(adjective)	(adjective)	24 or A-25)			(ft ³ /yr)	1.3 / (5)}
			(ft/yr)				
10400-	1. high	ext,	1.5	40	4	240	
AR HOR	ROCK	1300	nation of the second		105 26655		
3/01/0-	Mod	high	142	40	a.	33.6	
10+80-	10W	ION	.036	66	2	43	
5 11+40 -	V. high	ext:	1,5	30	3	136	
6/2+40	high	high	158	70	3	121.8	1990 (1990) (1990) 1990 (1990) (1990) 1990 (1990) 1990 (1990) 1990 (1990)
12+40-	Vi high	high	.58	20	3	34.8	
12+60-	high	high	158	140	3	043.6	
8.14+00	Ingr	<u>I niga</u>					
9							
10.							<u>, 483 (84)</u>
12.							
14.							
					Total		
Sum erosion su	idtotals in Colu	imn (7) for ea	CN BEHI/NBS	compination	(ft³/yr)		
Convert erosion	n in ft ³ /yr to yd	s ³ /yr {divide T	otal erosion (ft ³ /yr) by 27}	Total erosion	2.1	
	<u></u>		and a company of the	na na Trans. Na Na N	(yds ³ /yr) Total		
Convert erosior by 1.3}	n in yds³/yr to t	ons/yr {multi	oly Total erosi	on (yds³/yr)	erosion (tons/yr)	311	
Calculate erosi	on per unit len	gth of channe	I {divide Tota	l erosion	Total		
(tons/yr) by tota				a	erosion (tons/yr/ft)	\mathbf{N}^{I}	

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River Restoration & Natural Channel Design

Worksheet C-3. Summary form to estimate annuals streambank erosion for various study reaches.

reaches. Stream: UT	10 / 1++10	2 Alam. «	Cr.	Location:	Rurlino	Hon, NC	
Graph Used: CC					(40 ft B		5-21-07
Observers: BN	F, JE	ainser i ge	Valley Type:	r - B - 21	vita tire the state	Stream Type:	E4/1
Station (ft)	BEHI rating		Bank	Length of bank (ft)	Study bank height (ft)	(7) Erosion subtotal [(4)×(5)×(6)] (ft ³ /yr)	Erosion Rate (tons/yr/ft) {[(7)/27] × 1.3 / (5)}
10+00-	low.	Jow	,036	40	a.	2.6%	
10+40 -	Mod	10W	.15	40	A		
10180-	VH	high	.58	40	2,77	126	
11+20-	Mod	low	.15	40	2		
11-60-	Med	Jow	16	85		1921S	
12+45-	Mod	low	1.15	30		<u>3</u> 7	- S
14135	high	high	15 %	125		Settin S	
8.							
10.							
12.							44
14.							
Sum erosion su	ibtotals in Coli	umn (7) for ea	ch BEHI/NBS	combination	(ft³/yr)		
Convert erosio	n in ft ³ /ýr to yd	s ³ /yr {divide T	otal erosion	(ft ³ /yr) by 27}	(yds³/yr)		
Convert erosion by 1.3}	n in yds ³ /yr to i	tons/yr {multij	oly Total eros	ion (yds ³ /yr)	Total erosion (tons/yr)		
Calculate erosi (tons/yr) by tota	on per unit len al length of stro	gth of channe eam (ft) surve	I {divide Tota yed}	ll erosion	Total erosion (tons/yr/ft)		

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			Estimat	ing Near-	Bank Str	ess (N	BS)		
Stream:	IT.	to Littl			Bur		N,NC	10400	+010+
Observer		NF, JI		Righ	nt Bar	ik	· · · · · · · · · · · · · · · · · · ·	Date: 5-	2+07
			and the second	the second s	ATING NEA		STRESS		
1) Trans	verse bar	or split channe	l/central bar c	reating NBS/hi	igh velocity gra	dient	Level I	Recona	issance
		n (Rc/W)						General F	rediction
		ope to average						General F	Prediction
		ope to riffle slop					· I should TT	General F	Prediction
		ank maximum (Detailed F	Prediction
and a second second		ank shear stres			Contraction of the second second		Level III	Detailed F	
		s / Isovels / Vel					Level IV	Valid	
7) Veloc	aty promes						3		
M I	(1)								
Level	(±)						¥1)		
		Radius of	Bankfull		Near-Bank				
		Curvature	Width	Ratio	Stress				
	(2)	Rc (feet)	W _{bkf} (feet)	Rc/W	NBS	ľ			
			Average		Near-Bank				
Level II	(3)	Pool Slope	Slope	Ratio	Stress NBS		Dom Near-Bai		
Lev		Sp	S	Sp / S					,
							extr	emer	
-		5 10		Defin	Near-Bank Stress				
	(4)	Pool Slope Sp	Riffle Slope Srif	<i>Ratio</i> Sp / Srif	NBS				
	<u> </u>	Near Bank			Near-Bank				
		Max Depth	Mean Depth	Ratio	Stress				
	(5)	d _{nb} (feet)	d (feet)	d _{nb} / d	NBS			•	
III		4.0	1.0	4	ext.				
o		Near Bank	Near Bank	Near-Bank		Δνοτασο	Bankfull Shear	Ratio	Near-Bank
Lev		Max Depth	Slope	Shear Stress	Mean Depth	Average Slope	Stress	Ναιο	Stress
,	(6)	d _{nb} (feet)	S _{nb}	$ au_{nb}$ (lb/ft^2)	d (feet)	S	τ(lb/ft ²)	τ_{nb}/τ	NBS
									
	<u> </u>	<u> </u>		Nace Deed			<u> </u>		I
Ν		Valant	Gradient	Near-Bank Stress	ł				
Level IV	(7)		s/ft)	NBS					
Le									
	<u></u>	<u></u>		Conve	rting Values	to a Near-B	ank Stress R	ATING	
Noor P-	ante Stean	s RATINGS		1	-	lethod Numb			
wear-Da	arik Stres	6 DMIIN 6	(1)	(2)	(3)	(4)	(5)	(6)	(7)
	Very L	ow	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 1.00
	Low		N/A	2.21 - 3.00	0.20 - 0.40	0.41 - 0.60	1.00 - 1.50	0.80 - 1.05	1.00 - 1.20
	Modera	ate	N/A	2.01 - 2.20	0.41 - 0.60	0.61 - 0.80	1.51 - 1.80	1.06 - 1.14	1.21 - 1.60
	High		See	1.81 - 2.00	0.61 - 0.80	0.81 - 1.00	1.81 - 2.50	1.15 - 1.19	1.61 - 2.00
	Very H	igh	(1)	1.50 - 1.80	0.81 - 1.00	1.01 - 1.20	2.51 - 3.00	1.20 - 1.60	2.01 - 2.30
	Extren		Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.30
			÷ ,	<u> </u>	erall Near-I	· · · · · · · · · · · · · · · · · · ·	ss RATING	ex-	rène

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Estimating Near-Bank Stress (NBS)									
Stream:	UT +	o Little	Alan.	Location	near	X-Se	ct 6		
Observer	Λ.	1 1	T	Iρ	1 1 1 1	nk	~p	Date: 5-	2407
	<u>.</u>		IETHODS F	OR ESTIM			TRESS		
2.000 V2.000	er eta nuzza voren hor	or split channe						Recona	issance
							a and a second and a second a	· •• · · •• · · · · · · · · · · · · · ·	Prediction
		(Rc/W)							Prediction
		ope to average					1	· · · · · · · · · · · · · · · · · · ·	Prediction
and the second sec	The second second second	ope to riffie slop							
		ank maximum (Prediction
(6) Ratio	of near-ba	ank shear stres	s to bankfull s	hear stress (1	_{nb} / τ _{bkf})		Level III		Prediction
(7) Veloc	ity profiles	/ Isovels / Vel					Level IV		lation
I									
Level	(1)	Extensive de					1)		
╞━┻┥				leander migra		IG HOW (INDO #	·)		
		Radius of Curvature	Bankfull Width	Dotto	Near-Bank Stress				
	(2)	Rc (feet)	W _{bkf} (feet)	<i>Ratio</i> Rc / W	NBS				
				<u> </u>	Near-Bank				
	(3)	Pool Slope	Average Slope	Ratio	Stress		Domi	inant	1
Level II		Sp	S	Sp / S	NBS		Near-Bar	nk Stress	
- <u> </u>							Hìo	ih	
F					Near-Bank			<u>,</u>	
	(4)	Pool Slope	Riffle Slope	Ratio	Stress				
		Sp	Srif	Sp / Srif	NBS	1			
		Near Bank	Mean		Near-Bank				
	(5)	Max Depth	Depth	Ratio	Stress NBS				
		d _{nb} (feet)	d (feet)			1			
H		2.1	1.	2.45	high		· · · · · · · · · · · · · · · · · · ·		1
evel		Near Bank		Near-Bank	Mean	Average	Bankfull Shear	Ratio	Near-Bank
Lev .		Max Depth	Slope	Shear Stress	Depth	Slope	Stress	τ_{nb}/τ	Stress
	(6)	d _{nb} (feet)	S _{nb}	τ_{nb} (lb/ft ²)	d (feet)	S	τ (lb/ft ²)	CUD, C	NBS
	l	<u> </u>	<u></u>	Near-Bank					<u></u>
Level IV		Velocitv	Gradient	Stress					
evel	(7)		s/ft)	NBS					
Ľ									
			1	Conve	-		ank Stress R	ATING	
Near-Ba	ank Stres	s RATINGS		,		lethod Numb		(0)	T (
		andre en en en en surre e	(1)	(2)	(3)	(4)	(5)	(6)	<u> (7)</u> ,
Very Low N/A				> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 1.00
u	Low		N/A	2.21 - 3.00		0.41 - 0.60	a and a subscription are an element of	0.80 - 1.05	1.00 - 1.20
ļ.	Modera	ate	N/A	2.01 - 2.20	0.41 - 0.60	0.61 - 0.80	1.51 - 1.80	1.06 - 1.14	1.21 - 1.60
	High	<u>)</u>	See	1.81 - 2.00	0.61 - 0.80	0.81 - 1.00	1.81 - 2.50) 1.15 - 1.19	1.61 - 2.00
	Very H	igh	(1)	1.50 - 1.80	0.81 - 1.00	1.01 - 1.20	2.51 - 3.00	1.20 - 1.60	2.01 - 2.30
	Extren	ne	Above	< 1.50	> 1.00	, > 1.20	> 3.00	> 1.60	> 2.30
				Ov	erall Near-	Bank Stre	ss RATING	h.'a	h
						······			,

G:\TheRAM3-2-2006\1st Field Day-SECTION A\ram06-PAGE-A 27-EstNearBankStress.xls

Estimating Near-Bank Stress (NBS)									
	1	, , , , , , , , , , , , , , , , , , , ,							
1		to Little	المستامين مد	Location		1720	+0 11-		
Observe	rs: B		<u>Fet</u>		Bank			Date: 5-	-3F01
ರ್ ಜನ್ ಹಾನವರ	ito a nu mbata.		METHODS I						
and a second		or split channe							issance
(2) Chan	nel patteri	n (Rc/W)					Level II	an an artistan second a second as	Prediction
(3) Ratio	of pool sl	ope to average	water surface	slope (S _p / S)			د د ه د ۲۰ مد په دهنو د پسور د د .	Prediction
(4) Ratio	of pool sl	ope to riffle slo	pe (S _p / S _{rif})				Level II	General I	Prediction
(5) Ratio	of near-ba	ank maximum	depth to bankf	ull mean depth	n (d _{nb} /d _{bkf})		Level III	Detailed	Prediction
(6) Ratio	of near-ba	ank shear stres	ss to bankfull s	hear stress (1	τ _{nb} / τ _{bkf})		Level III	Detailed	Prediction
(7) Veloc	ity profiles	s / Isovels / Ve	locity gradient.				Level IV	Valid	ation
I									
Level	(1)								
		·		neander migra		ng flow (NBS #	£1)	NB	s = Extreme
	:	Radius of Curvature	Bankfull Width	<u> </u>	Near-Bank Stress				
	(2)	Rc (feet)	Width W _{bkf} (feet)	<i>Ratio</i> Rc / W	NBS				
		15		1,25	011	1			
			10	102	<u>ex</u> +, Near-Bank				
H	(3)	Pool Slope	Average Slope	Ratio	Stress		Dom	inant	
Level II		Sp	S	Sp / S	NBS		Near-Ba	nk Stress	
Ľ							ex+	rene	
					Near-Bank	Υ Ι	<u> </u>		
		Pool Slope	Riffle Slope	Ratio	Stress				
	(4)	Sp	Srif	Sp / Srif	NBS	ļ			
		Near Bank	Mean		Near-Bank				
	(5)	Max Depth	Depth	Ratio	Stress NBS		•		
		d _{nb} (feet)	d (feet)	d _{nb} /d		1			
		L			l	ļ		· ·	
Level		Near Bank	Near Bank		Mean	Average	Bankfull Shear	Ratio	Near-Bank
- -	(6)	Max Depth	Slope	Shear Stress	Depth	Slope	Stress	τ_{nb}/τ	Stress
	(6)	d _{nb} (feet)	S _{nb}	τ_{nb} (lb/ft ²)	d (feet)	S	τ (lb/ft ²)	YND' Y	NBS
					· ·				
		1	••••••	Near-Bank					
Level IV		Velocity	Gradient	Stress					
eve	(7)	(ft/	s/ft)	NBS					
		<u> </u>							
				Conve	-		ank Stress R	ATING	
Near-Bank Stress RATINGS			(4)	(1)		lethod Numb		(6)	(7)
				(2)	(3)	(4)	(5)	(6)	(7)
Very Low N/A			<u>;</u>	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 1.00
.	Low		N/A	2.21 - 3.00		0.41 - 0.60	1.00 - 1.50	0.80 - 1.05	1.00 - 1.20
-	Modera		N/A	2.01 - 2.20	0.41 - 0.60	0.61 - 0.80	1.51 - 1.80	1.06 - 1.14	1.21 - 1.60
 	High		See	1.81 - 2.00	0.61 - 0.80	0.81 - 1.00	1.81 - 2.50	1.15 - 1.19	1.61 - 2.00
.	Very Hi		(1)	1.50 - 1.80	0.81 - 1.00		2.51 - 3.00	1.20 - 1.60	2.01 - 2.30
	Extrem	1e ;	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.30
				Ove	erall Near-I	Bank Stres	ss RATING	1 82	-rene

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		· · · · · · · · · · · · · · · · · · ·			Bank Str			11-0	1	
Stream:	UT	to Little	2 Alarl		: near				to 1279	
Observe	rs: <u>B</u>	NF, J	<u>et</u>		Hright			Date: 5-	21-07	
		N	METHODS I	FOR ESTIM	IATING NE	AR-BANK S	TRESS	· · · · · · · · · · · · · · · · · · ·		
(1) Trans	verse bar	or split channe	l/central bar c	reating NBS/h	igh velocity gra	adient	Level I	Recona	issance	
(2) Chan	nel patter	n (Rc/W)					Level II	General	Prediction	
(3) Ratio	of pool sl	ope to average	water surface	slope (Sp / S)		Level II	General I	Prediction	
		ope to riffle slo						General	Prediction	
		ank maximum						Detailed	Prediction	
		ank shear stres					1 1 797	Detailed	Prediction	
		s / Isovels / Vel					Level IV	Valio	ation	
				and the second state of th			<u> </u>			
el I	(1)		eposition (co							
Level		Chute cutoffs,	down-valley r	neander migra	ation, convergin	ng flow (NBS #	±1)	NB	S = Extreme	
		Radius of	Bankfull		Near-Bank					
	(9)	Curvature	Width	Ratio	Stress					
	(2)	Rc (feet)	W _{bkf} (feet)	Rc/W	NBS					
	(3)		Average		Near-Bank				1	
ПК		(3)	(3)	Pool Slope	Slope	Ratio	Stress		Dom	
Level II	(0)	Sp	S	Sp/S	NBS	1 1	Near-Bai	f		
-							L0	$\overline{\mathcal{W}}$		
					Near-Bank	-				
	(4)	Pool Slope			Stress					
	(-)	Sp	Srif	Sp / Srif	NBS	1				
					· · · · · · · · · · · · · · · · · · ·					
		Near Bank	Mean		Near-Bank					
	(5)	Max Depth	Depth	Ratio	Stress NBS					
L		d _{nb} (feet)	d (feet)	d _{nb} /d		1				
Ш				1.4	LOW	ļ				
Level			Near Bank	Near-Bank	Mean	Average	Bankfull Shear	Ratio	Near-Bank	
_ _		Max Depth	Slope	Shear Stress	Depth	Slope	Stress	τ_{nb}/τ	Stress	
•	(6)	d _{nb} (feet)	S _{nb}	τ_{nb} (lb/ft ²)	d (feet)	S	τ (lb/ft ²)	- 00 · -	NBS	
		\		Near-Bank					· · · · · · · · · · · · · · · · · · ·	
Level IV	/	Velocity	Gradient	Stress						
evel	(7)		s/ft)	NBS	ļ					
Ľ										
			Conve	erting Values	to a Near-Ba	ank Stress R	ATING			
Near-Ba	ank Stres	s RATINGS		، 	1	lethod Numb				
Near-Bank Stress RATINGS (1)		(1)	(2)	(3)	(4)	(5)	(6)	<u>(7)</u>		
	Very L	ow	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 1.00	
	Low		N/A	2.21 - 3.00	0.20 - 0.40	0.41 - 0.60	(1.00 - 1.50)	0.80 - 1.05	1.00 - 1.20	
	Modera	ate	N/A	2.01 - 2.20	0.41 - 0.60	0.61 - 0.80	1.51 - 1.80	1.06 - 1.14	1.21 - 1.60	
•	High		See	1.81 - 2.00	0.61 - 0.80	0.81 - 1.00	1.81 - 2.50	1.15 - 1.19	1.61 - 2.00	
• • •	Very H		(1)	1.50 - 1.80	· · · · · · · · · · · · · · · · · · ·	1.01 - 1.20	2.51 - 3.00	1.20 - 1.60	2.01 - 2.30	
d	Extren	• • • • • • • • • • • • • • • • • • •	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.30	
		·		í		Bank Stre	ss RATING	10	1.	
			1							

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Appendix D

BMP Design Calculations

PAGE 5 DATE: 10/07 Little Alamance Creek BY: ST SUBJECT: Level spreader Calculations ARCADIS SHEET CHKD: DATE: JOB NO

30" HDPE @ station 1100, sheet 2 Q10 = 5.8 cfs DA. to swale = 2 acres ■ Swale length = 100' for every sacre draining to it. Swale length = 100'X Zacres = 200' Regld ~) for site -Make swale 10' - just enough to collect flowat same lo cotton. Design Forebay Surface Area (SA) Contributing impervious area = \$\$, so use DiA. not going to inlet = 0.95 ac × 0.2% = 83.442 Design Level spreader Lip length Design Level - pro-Length is 13' for every I cfs of flow to grass or to thick ground cover filter strip. Length = 13' × 5,8cfs/24 = 75' long 18" KCP @ sta 25+00 D.A. to swale = 0,33 acres Q10 = 7 cfs (capacity) Swale length = 100 x 0,330c = 33 min, read. Design Forebay SA Contributing impervious area = ϕ , so use D.A. to swale 0.33ac × 0.2% = 29 ft² Design Level Spreader Lip length -10-yr, storm 13' × 7cfs/1ft = 91 feet long - thick ground cover filterstrip



JOB NO:

SUBJECT: Little Alamance Creek BY: JT DATE: 10/07 Level Spreader Calculations CHKD:

PAGE 6 SHEET

DATE

Assume unknownpipe is 12" HDPE] D.A.to swale = 0.13 ac [Sta 26 too] Strale length = 100'x Q13 KC Q10= 3cfs = 13 ft. Design Forebay Surface Area Contributing impervious dualhage area = (\$) As use contributing DA. 0.13 ac + 0.43 = 0.56ac to DI and to ditch = x 0,2% = 49 ff 2 Design Level Spreeder Lip for 10-yr, storm and thick ground cover. 13' × 3 cfs/2ft = 39 feet long D.A. to swale = 0.20 ac (8" HOPE @ sta 27.400) swale length = 100'x a20ac Qio= 3cfs DiA to inlet = 0.32ac 12egd, Design Forebay Contributing Impervitus D.A. = 3,380.15 AZ (0,078 ac) X02% = 7ft2 but make 49ft2 like previous pipe. Design level spreader lip 13' × 3 cfs/2A, = 39 feet long

PAGE Little Alamance Creek BY: JT DATE: 10/07 7 SUBJECT: Level Spreader Calculations SHEET CHKD: DATE JOB NO:

12" RCP @ Sta 35+00 DiA. to swale ditch= 0.094ac swale length = 100'x 0,094 ac Q10= 3cts = 9.5 ft. -Leave above -Design Forebay Grassy area toactas Contributing impervious Det = \$7, so use (0.094 ac) filter. $0.094 \times 0.2\% = 8.44^{2}$ Design Level spreader lip 13' por Icfs of flow = 13' × 3 cfs = 39 feet long for 10 yer, thick groud cover fifterstip. 15" PCP @ sta 12+00 Trib DA tosuale= 0,68 ac Swale length = 100'X 0.68 ac Qio= 4.5cfs = 68 feet Design Forebaug contributing impervious D.A. = 1/3 of Q68 = 0.23 ac × 0.2% 20 ft 2 Design Level Spreader Lip 13' x 4.5 cfs/27+ - 59 feet long for 10-m. , thick ground cover

Appendix E

Design Sheets



ROADS & RELATED ITEMS

Prop. Woven Wire Fence	
Prop. Chain Link Fence	
Prop. Barbed Wire Fence	
Silt Fence	
Exist. Guardrail	<u> </u>
Prop. Guardrail	
- Equality Symbol	

RIGHT OF WAY

BOUNDARIES & PROPERTIES

Property Line Surveyed	
Property Line Not Surveyed	
Exist. Iron Pin	8
Property Corner	
Property Monument	æ
Property Number	(2)
Parcel Number	6
Fence Line	\sim
Existing Wetland Boundaries	— — — WLB — — -
Proposed Wetland Boundaries	
Buildings	\mathbf{C}
Foundations	Ē
HYDROLOGY	-
Stream or Body of Water	
Flow Arrow	>

Shoreline Falls, Rapids

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Spring

Storm Sewer Manhole	Approx. Location of Proposed Boulder
Recorded Water Line	Approx. Location of Proposed Rootwad
Sanitary Sewer	Approx. Location of Proposed Stream Plug w/ Root Wads (See Detail)
Storm Sewer	Proposed Oxbox Pond/Wetland (See Detail)
Recorded Telephone Cable	Approximate Limits of Buffer
Recorded U/G Telephone Conduit	Existing Thalweg.
Exist. Water Meter 0	Existing Top of Bank
Exist. Overhead Power Line	Proposed Thalweg
Exist. Underground Utilities	Proposed Bankfull
STRUCTURES	Slope Stake Line <u>C(cut)</u> F(fill)

L	E	GEND	
		TOPOGRAPHY	
	٠	Hard Surface	
	•	Change in Road Surface	
	-0 -	Curb	
	+	Right of Way Symbol	
		Guard Post	
	C	Paved Walk	
	4	Bridge	
	8	Box Culvert or Tunnel	
	Ð	Culvert	
	ø	Footbridge	
	•	Trail, Footpath	
	a		
	\boxtimes	STREAM IMPROVEMENTS	
	Ø	•	
		Approx. Location of Proposed Boulder	
	\succ	Approx. Location of Proposed Boulder	
	0	Cross Vane (See Detail)	
	0	Approx. Location of Proposed Boulder.	
	0	Approx. Location of Proposed Rootwad	

STRUCTURES

	SIRCCIORES
	MAJOR
-	Bridge, Tunnel, or Box Culvert
	Bridge Wing Wall, Head Wall
	MINOR
	Head & End Wall
	Pipe Culvert === :
	Footbridge
	Drainage Boxes

801 Corporate Center Drive, Suite 300

Tel: 919/854-1282 Fax: 919/854-5448

Raleigh, NC 27607-5073

K	
Tunnel, or Box Culvert	
Wing Wall, Head Wall	
End Wall	
R	
z End Wall	
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ge Boxes 🗖 cs	

G & M of North Carolina, Inc. WWW.ARCADIS-US.COM

			CONSTRUCTION
5			
4			
3			
2		1/25/08	RESTORATION PLAN
-	REB	10/24/07	ORAFT RESTORATION PL
	BY	DATE	DESCRIPTION OF REVIS

DESIGN ENGINEER

UTILITIES

Exist. Pole Exist. Power Pole

Exist. Telephone Pole

Exist. Joint Use Pole

Telephone Pedestal

Cable TV Pedestal

Hydrant

Exist. Water Valve

Sewer Clean Out

Power Manhole

Water Manhole

Light Pole

Power Line Tower

Pole with Base

Power Transformer

Guy Wire Anchor

Sanitary Sewer Manhole

VEGETATION

Existing Woods Line	
Existing Tree	

NS v	NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES ECOSYSTEM ENHANCEMENT PROGRAM
	LITTLE ALAMANCE CREEK ALAMANCE COUNTY, NORTH CAROLINA
PLAN	
VISION	LEGEND SHEET NO. 2

MORPHOLOGICAL CHARACTERISTICS OF THE EXISTING AND PROPOSED CHANNEL WITH REFERENCE REACH DATA

LITTLE ALAMANCE CREEK

I. Stream Type C/25/1 C 4/1 C4 E4 2. Dardrove Aver (so, mi) 42 32 Mann 362 Mann 362 Mann 29 Mann 91 4. Borkfull Widh Wohl (bbf / 111 Mann 25 Mann 26 Mann 27 Mann 10 Mann 92 5. Widh/Depth (bbf / 101/1 Mann 25 Mann 26 Mann 12 Mann 14 Mann 14 5. Widh/Depth (bbf / 101/1 Mann 25.0 Mann 25 Mann 26 Mann 13 Mann 14 Mann 69 6. Bonkfull Wan Valcub (Vbf / 11/ps Mann 25 Mann 25 Mann 25 Mann 14 Mann 150 Mann		Park @ Friendly Reference Reach		Benbow	d Reach	Proces	a Channel	Eviction	Variables
2. Drainoge Area (sq.ni) Hean 36.2 Mean 36.2 Mean 0.2 3. Bankfull Weidh (Wbif/11) Mean 36.2 Mean 20.9 Mean 20.9 Mean 95. 4. Bankfull Wein Depth (abk1)11 Mean 22.2.2.9 Monce 12.4 Mean 14 6. Bankfull Wein Depth (abk1)11 Mean Mean 13.8 Mean 12.4 Mean 65.0 6. Bankfull Mean Veits Mean 16.7 Mean 95.0 Mean 13.8 Mean 65.0 7. Bankfull Mean Veits Mean 95.0 Mean 2.5 Mean 12.4 Mean Mean 13.0 Range 7.8 Mean 2.5 Mean 2.5 Mean Mean 10.4 Mean	Reference h			<u>rereren</u>		rr opose			Variables
3. Boakfull Width Wikk1111 Meon: 362 Meon: 209 Meon: 95 4. Bankfull Meon Deph (dbk1)111 Meon: 25 Meon: 12 Meon: 14 Meon: 13 Meon: 14 Meon: 14 Meon: 13 Meon: 14 Meon: 13 Meon: 14 Meon: 13 Meon: 14 Meon: 13 Meon: 130 Meon: <td><u> </u></td> <td><u></u><u>E4</u></td> <td><u>(4</u></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	<u> </u>	<u></u> <u>E4</u>	<u>(4</u>						
Parter Ranges Ranges Ranges Ranges Ranges Ranges 4. Bankfull Mean Depth (abk11111 Mean 22-29 Ranges	0.7				4.2				2. Drainage Area (sq.mi)
4. Bankfull Mean Deph (dbk1/11) Mean: 25 Mean: 17 Mean: 17 Mean: 17 5. Widhn/Deph Ratio (Wbif / dbk1) Mean: 140 Mean: 138 Mean: 124 Mean: 63 6. Bankfull Cross-Sectional Area Mean: 950 Mean: 950 Mean: 950 Mean: 950 Mean: 750 Mean: 750 <t< td=""><td>Mean: Range:</td><td></td><td>20.9</td><td></td><td>362</td><td></td><td>362 318-425</td><td></td><td>3. Bankfull Width (Wbkf) ft</td></t<>	Mean: Range:		20.9		362		362 318-425		3. Bankfull Width (Wbkf) ft
5. Widhr/Depth Ratio (Nbb1/Abk1) Kenne 140 Kenne 153 Kenne 15-70 Range 15-70 Kenne 15-70 Range 2 Range	Mean:	lean: 1.4	17	Mean:	2.6	Mean:	2.6	Mean:	4. Bankfull Mean Depth (dbkf) ft
Bank Uil Cross-Sectional Area (Abt1) sg f1 Range: Range: 79.3~125.0 Range: Range: Range: 79.3~125.0 Range: Range: Range: Range: 79.3~125.0 Range: Ran	Range: Mean:		12,4		13.8				5. Width/Depth Ratio (Wbkf/dbkf)
(Abt/) sg (1) Ronge:	Range;	Range:	75.7	Range:		Range:	II.G-I7.D	Range:	•
Range:	Mean: Range:	Range:		Range:		Range:	79.3-125.0	Range:	(Abkf) sq ft
8. Bankfull Discharge,cfs (0bt1) Hean, 237.5 Mean, 427.5 Mean, 5 Mean, 6 Mean, 7 Mean, 6 Mean, 7 Mean	Mean: Range:		•		2.5		25	Mean: Ranae:	7. Bankfull Mean Velocity (Vbkf) fps
9. NaxImum Bankfull Depth (dnax) /11 Mean: 40 Mean: 40 Mean: 40 Range: 39-41 Range: 39-41 Range: 39-41 Range: 39-41 Range: 39-41 Range: 30-41 Range: 10 Mean: 300 Mean: 500 Mean: 500 Range: 70-1200 Range: 70-1200 Range: 70-1200 Range: 70-1200 Range: 70-1200 Mean: 500 Mean: 600 Mean: 500 Mean: 600 Mean: 500 Mean: 600 Mean: 700 Mean: 600 Mean: 700 Mean: 70	Mean:	lean: *	*	Mean:	237.5	Mean:	237,5	Mean:	8. Bankfull Discharge.cfs (Obkf)
Range: 39-41 Range: 39-41 Range:	Range: Mean:	<u>ionge:</u> Jean• 20	3/		40		40		9 Maximum Backfull Decth (dmax) ft
Benif ull Depth (Bhow/dinax) Range: I/I. (Indit) of Flood Prone Area (Wrpo) fl Mean 940 Mean 800 Range: Range: </td <td>Range:</td> <td>Range:</td> <td></td> <td>Range:</td> <td>3.9-4J</td> <td>Range:</td> <td>3.9-4.1</td> <td>Range:</td> <td></td>	Range:	Range:		Range:	3.9-4J	Range:	3.9-4.1	Range:	
II. Width of Flood Prone Area (Wfpolf) Mean: 940 Mean: 940 Mean: 300 Mean: 300 Range: 700-7200 Range: 720-720 Range: 720-720 Mean: 322 Mean: 322 Mean: 322 13. Meander Length (Lm) If Range: 2270-5590 Mean: 360 Mean: 360 Mean: 360 14. Ratio of Meander Length to Bant uil Width (Lm/Wbir) Mean: 100 Mean: 360 Mean: 400 Mean: 400 15. Raduls of Curvature to Bant uil Width (Lm/Wbir) Range: 450-7200 Range: 450-7200 Range: 70 Mean: 400 16. Ratio of Raduls of Curvature to Bantfull Width (Rc/Wbir) Mean: 150 Mean: 450 Mean: 450 Mean: 450 17. Bell Width Width 111 Mean: 700 Mean: 12 Mean: 450 Range: 12-61 Range: Range: 12-61 Range: 70 18. Meander Width Ratio (Wbir/Wbir) Range: 12-61 Range: 12-61 Range: 70 Mean: 460n Mean: 460n 19. Strusstly (Stream length/voiley Mean: 12 Mean: 12 Mean: 460n	Mean: Range:		QI		I,D				IO. Ratio of Low Bank Height to Max. Bankfull Depth (Bhlow/dmax)
12. Entrenchment Ratio (Wf pa/Wbk1) Hean: 2.5 Mean: > 2.2 Mean: 19 Mean: 3.2 13. Meander Length (Lm) ft Mean: 3610 Mean: Mean: 100 Mean: Mean: 100 Mean: Mean: 1150 Mean: Mean: Mean: 1150 Mean: Mean: 1150 Mean: Mean: 1150 Mean: Mean: 1150 Mean: Mean: 1160 Mean: 1160 Mean: 1160 Mean: 117 Mean: 119 Mean: 117 Mean: </td <td>Mean:</td> <td>lean: 30.0</td> <td>40,0</td> <td>Mean:</td> <td>> 80.0</td> <td>Mean:</td> <td>94,0</td> <td>Mean:</td> <td>II. Width of Flood Prone Area (Wfpa) ft</td>	Mean:	lean: 30.0	40,0	Mean:	> 80.0	Mean:	94,0	Mean:	II. Width of Flood Prone Area (Wfpa) ft
13. Meander Length (Lmi) ff Mean: 3610 Mean: 3610 Mean:	Range: Mean:	tonge: leon: 3.2	1.9		> 2.2		2.6	Mean:	12. Entrenchment Ratio (Wfpa/Wbkf)
Range: 2270-5590 Range:	Range: Mean:				3610			Range:	13 Neorder Length (Im) ft
Bankfull Width (Lm/Wbkf) Range: 6.3-15.4 Range: Sister and the sist	Range:	Range:		Range:	227.D-559.D	Range:	227.D-559.D	Range:	-
15. Raduls of Curvature (Rc) ft Hean: 115.0 Mean: 115.0 Range: 45.0-220.0 Range: Ange: Ange: <td>Mean: Range:</td> <td></td> <td>*</td> <td></td> <td></td> <td></td> <td>10.0 6.3-15.4</td> <td></td> <td></td>	Mean: Range:		*				10.0 6.3-15.4		
16. Ratio of Radius of Curvature to Bankfull Width (Rc/Wbkf) Mean: Range: 32 Range: Mean: Nange: 32 Range: Mean: Range: 32 Range: Mean: Range: Mean: Range: Mean: Range: Mean: Range: Mean: Range: Range: Range: Range: Range: Mean: Range: Mean: Range: </td <td>Mean: Range:</td> <td>lean: *</td> <td>*</td> <td>Mean:</td> <td>II5D</td> <td>Mean:</td> <td><i>II5.</i>0</td> <td>Mean:</td> <td></td>	Mean: Range:	lean: *	*	Mean:	II5D	Mean:	<i>II5.</i> 0	Mean:	
17. Belt Wildth (Wbit) f1 Mean; 700 Mean; 70 Mean; 70 Range; 33.0-255.0 Range; 33.0-255.0 Range; 33.0-255.0 Range; 70 Ra	Mean:	lean: *	*	Mean:	32	Mean:	3.2	Mean:	
Range: 33.0-255.0 Range: 33.0-255.0 Range: Range	Range: Mean:						12-6J 700		
Range: 0.9-7.0 Range: 0.9-7.0 Range: Range: Range: 19. Sinuosity (Stream length/valley distance) (k) Mean: 1.2 Mean: 1.2 Mean: · Mean: ·<	Range:	Range:		Range:	330-2550	Range:	33.0-255.0	Range:	
distance)(k) Range: Range:<	Mean: Range:		-						
20. Valley Slope (f1/f1) Mean: Range: 0.0028 Range: Mean: Range: 0.0028 Range: Mean: Range: * Mean: Range: <td>Mean: Range:</td> <td></td> <td>*</td> <td></td> <td>12</td> <td></td> <td>12</td> <td></td> <td>19. Sinuosity (Stream length/valley distance) (k)</td>	Mean: Range:		*		12		12		19. Sinuosity (Stream length/valley distance) (k)
21. Average Water Surface Slope or Bankful Slope for Reach (Sbkf or Savg)=(Svalley/k) ft / ft Mean: Range: 0.0024 Range: Mean: Range: 0.0024 Range: Mean: Range:	Mean:	lean: *	*	Mean:	0.0028	Mean:	0.0028	Mean:	
Savg)=(Svalley/k)ft/ft Mean: 0.0005 Mean: 0.0 Mean: Mean: Range: Rang	Range: Mean:		*		0.0024		0.0024		21. Average Water Surface Slope or
22. Pool Slope (Spool) ft / ft Mean: 0.0005 Range: 0.0-0.0015 Mean: 0.0 Range: 0.0-0.0015 Mean: 0.0 Range: Range: R	Range:	lange:		Range:		Range:		Range:	Bankful Slope for Reach (Sbkf or Sava)=(Svallev/k) ft / ft
23. Ratio of Pool Slope to Average Slope (Spool/Sbkf) Mean: Range: 0.2 Range: Mean: Range: 0.0 Range: Mean: Range: * Mean: Range: * Range: Range: * Range: Range: * Range: Range: * Range: Range: * Mean: Range: * * Mean: Range:	Mean:		*	Mean:	0.0				22. Pool Slope (Spool) ft / ft
Slope (Spool/Sbkf) Range: Range: Question Range: Range: <th< td=""><td>Range: Mean:</td><td></td><td></td><td></td><td>0.0</td><td></td><td></td><td></td><td>23. Ratio of Pool Slope to Average</td></th<>	Range: Mean:				0.0				23. Ratio of Pool Slope to Average
Range: 5.5-6.9 Range: 5.5-6.9 Range: Range: Range: 25. Ratio of Maximum Pool Depth to Bankfull Mean Depth (dpool/dbkf) Mean: 2.3 Mean: 2.3 Mean: * Range: * * * Range: * * * Range: <td< td=""><td>Range:</td><td>Range:</td><td></td><td>Range:</td><td></td><td>Range:</td><td>20-0</td><td>Range:</td><td>Slope (Spool/Sbkf)</td></td<>	Range:	Range:		Range:		Range:	20-0	Range:	Slope (Spool/Sbkf)
25. Ratio of Maximum Pool Depth to Bankfull Mean Depth (dpool/dbkf) Mean: Range: 2.3 Range: Mean: 2.4-2.7 Range: 2.3 Range: Mean: Range: * * * * * * * * <td>Mean: Range:</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>24. Maximum Pool Depth (dpool) ft</td>	Mean: Range:								24. Maximum Pool Depth (dpool) ft
26. Pool Width (Wpool) ft Mean: 37.6 Range: 32.3-42.3 Mean: 37.6 Range: 32.3-42.3 Mean: 37.6 Range: 32.3-42.3 Mean: * Mean: * Mean: * Range: *	Mean:	lean: *		Mean:	2.3	Mean:	2.3	Mean:	
Range: 32.3-42.3 Range: 32.3-42.3 Range: Range: Range: * Geomorphic data not collected due to existing design constraints. The majority of the work on site will consist of Enhancement I. Ranges for restoration areas were developed around the existing site constraints. Bill Corporate Center Drive, Suite 300 Bill Corporate Center Drive, Suite 300	Range: Mean:								Banki uli Mean Depin (apool/abki) 26. Pool Width (Whool) ft
to existing design constraints. The majority of the work on site will consist of Enhancement I. Ranges for restoration areas were developed around the existing site constraints.	Range:								
to existing design constraints. The majority of the work on site will consist of Enhancement I. Ranges for restoration areas were developed around the existing site constraints.									Conmarable data ant collected due
ind joinly of the work on site with consist of Enhancement I. Ranges for restoration areas were developed around the existing site constraints.	OR CONSTRUCTION					X	ŠAK		o existing design constraints. The
around the existing site constraints. Beloich NC 27507-5033		5			orth Carolina,	M of N	G		consist of Enhancement I. Ranges
Z No V23/0		3		ОМ					for restoration areas were developed
Tel: 919/854-1282 Fax: 919/854-5448 DESIGN ENGINEER BY DATE	07 ORAFT RESTORATION PLA DESCRIPTION OF REVIS		DESIGN ENGINEER	I		854-5448			

Date:2/19/20 Filenome: o:/



/ariables	Existing Channel	Proposed Reach	Benbow Park Reference Reach	Park & Friendly Reference Reach	Brown Branch u/.
			Mean: *	Mean: *	Mean: *
27. Ratio of Pool Width to Bankfull				Range:	Range:
Width (Wpool/Wbkf)	Range: 0.9-1.2		Range:		
28. Bankfull Cross-sectional Area at	Mean: 140.2	Mean: 140.2	INCOM.	muun:	MUUII.
Pool (Apool) sq ft	Range: 121.0-156.6	Range: 121.0-156.6	Range:	Range:	Range:
29. Ratio of Pool Area to Bankfull	Mean: ID	Mean: 1.0	Mean: *	Mean: *	Mean: *
Area (Apool/Abkf)	Range: 1.3-1.7	Range: 1,3-1,7	Range:	Range:	Range:
30. Pool to Pool Spacing (p-p) ft	Mean: 473J	Mean: 473J	Mean: *	Mean: *	Mean: *
	Range: 313.7-749.5	Range: 313.7-749.5	Range:	Range:	Range:
31. Ratio of Pool-to-Pool Spacing to	Mean: I3J	Mean: I3J	Mean: *	Mean: *	Mean: *
Bankfull Width (p-p/Wbkf)	Range: 87-207	Range: 87-207	Range:	Range:	Range:
32. Pool Length (Lp) ft	Mean: 293.7	Mean: 293.7	Mean:	Mean: *	Mean: *
	Range: 107.9-505.4	Range: 107.9-505.4	Range:	Range:	Range:
33. Ratio of Pool Length to Bankfull	Mean: 8J	Mean: 8J	Mean: *	Mean: *	Mean:
	Range: 3.0-14.0	Range: 3.0-14.0	Range:	Range:	Range:
Width (Lp/Wbkf)		Mean: 0.0126	Mean: *	Mean: *	Mean: *
34. Riffle Slope (Sriff) ft / ft	Mean: 0.0126	Range: 0.0028-0.0254		Range	Range:
75 0 41 - 5 01564 - 61 - 4 - 4 - 4	Range: 0.0028-0.0254		Range: Mean: *	Mean: *	Mean: *
35. Ratio of Riffle Slope to Average	Mean: 5.2	Mean: 5.2	10000		
Slope (Sriff/Sbkf)	Range: 1.2-10.6	Range: 1,2-10,6	Range:	Range:	Range:
36. Maximum Riffle Depth (driff) ft	Mean: 4.0	Mean: 4.0	Mean: *	Mean: *	In Cone
	Range: 3.9-4J	Range: 3.9-4J	Range:	Range:	Range:
37. Ratio of Riffle Depth to Bankfull	Mean: 1.5	Mean: 1.5	Mean: *	Mean: *	Mean: *
Mean Depth (driff/dbkf)	Range: 1.5-1.6	Range: 1.5-1.6	Range:	Range:	Range:
38. Run Slope (Srun) ft / ft	Mean: 0.0032	Mean: 0.0032	Mean: *	Mean: *	Mean: *
	Range: 0.0-0.0090	Range: 0.0-0.0090	Range:	Range:	Range:
39. Ratio of Run Slope to Average	Mean: 1.3	Mean: 1.3	Mean: *	Mean: *	Mean: *
Slope (Srun/Sbkf)	Range: 0.0-3.7	Range: 0.0-3.7	Range:	Range:	Range:
40. Maximum Run Depth (drun) ft	Mean: N/A	Mean: N/A	Mean: *	Mean: *	Mean: *
	Range: N/A	Range: N/A	Range:	Range:	Range:
41. Ratio of Run Depth to Bankfull	Mean: N/A	Mean: N/A	Mean: *	Mean: *	Mean: *
Mean Depth (drun/dbkf)	Range: N/A	Range: N/A	Range:	Range:	Range:
42. Slope of Glide (Sgl) ft / ft	Mean: 0.0039	Mean: 0,0039	Mean: *	Mean: *	Mean: *
42. Slope of Glide (Syliti / IT		Range: 0.0-0.0107	Range:	Range:	Range:
17 Datte of Olds Class to Average			Mean: *	Mean: *	Mean: *
43. Ratio of Gilde Slope to Average	Mean: 16	Mean: 16		Range:	Range:
Water Surface Slope (Sgl/Sws)	Range: 0.0-4.5	Range: 0.0-4.5	Range:		
44. Maximum Glide Depth (dgl) ft	Mean: N/A	Mean: N/A	1110014	INCON.	MCON.
	Range: N/A	Range: N/A	Range:	Range:	Range:
45. Ratio of Glide Depth to Bankfull	Mean: N/A	Mean: N/A	Mean: *	Mean: *	Mean: *
Mean Depth (dgl/dbkf)	Range: N/A	Range: N/A	Range:	Range:	Range:
46. Step Slope (Sst)	Mean: N/A	Mean: N/A	Mean: *	Mean: *	Mean: *
·	Range: N/A	Range: N/A	Range:	Range:	Range:
47. Ratio of Step Slope to Average	Mean: N/A	Mean: N/A	Mean: *	Mean: *	Mean: *
Water Surface Slope (Sst/Savg)	Range: N/A	Range: N/A	Range:	Range:	Range:
48. Maximum Step Depth (dst)	Mean: N/A	Mean: N/A	Mean: *	Mean: *	Mean: *
io, movinium orop Dopin (doiv	Range: N/A	Range: N/A	Range:	Range:	Range:
49. Ratio of Step Depth to Bankfull	Mean: N/A	Mean: N/A	Mean: *	Mean: *	Mean: *
Mean Depth (dst/dbkf)	Range: N/A	Range: N/A	Range:	Range:	Range:

ARCADIS

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Raleigh, NC 27607-5073

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* Geomorphic data not collected due to existing design constraints. The majority of the work on site will consist of Enhancement I. Ranges for restoration areas were developed around the existing site constraints.



DESIGN ENGINEER

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	NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES ECOSYSTEM ENHANCEMENT PROGRAM
AN TION PLAN	LITTLE ALAMANCE CREEK ALAMANCE COUNTY, NORTH CAROLINA
F REVISION	MORPHOLOGICAL TABLES -PROPLAC- SHEET NO. 3A

MORPHOLOGICAL CHARACTERISTICS OF THE EXISTING AND PROPOSED CHANNEL WITH REFERENCE REACH DATA

	UN	INAMEL) TRIE	BUTARY	TO LI	TTLE A	LAMANO	CE CRI	EEK		
Variables	Existing (Channel	Propose	d Pacab	Deference	- Pacab	Deference	Pacab	Deference	Deceb	7
I. Stream Type					Referenc		Reference		Reference	Reach	4
2. Drainage Area (sq.mi)		4/I J2		<u>C 4/I</u> 0J2		C4 0.7	<u> </u>	<u> </u>		<u>/E4</u> 37	-{
Z. Drahfwill Wilder (Sq. IIII)											4
3. Bankfull Width (Wbkf) ft	Mean:	120	Mean:	12.0	Mean:	20.9	Mean:	9.5	Mean:	I5J	
		10.9-13.0	Range:	10.9-13.0	Range:		Range:		Range:		4
4. Bankfull Mean Depth (dbkf) ft	Mean:	13	Mean:	13	Mean:	1,7	Mean:	I . 4	Mean:	1,6	
	Range:	11-15	Range:	IJ-1 <u>5</u>	Range:		Range:		Range:		
5. Width/Depth Ratio (Wbkf/dbkf)	Mean:	9.3	Mean:	9.3	Mean:	12.4	Mean:	6.9	Mean:	9.3	
	Range:	7,1-11,5	Range:	71-11.5	Range:		Range:		Range:		_
6. Bankfull Cross-Sectional Area	Mean:	15.8	Mean:	15.8	Mean:	35.3	Mean:	13,0	Mean:	24.3	
(Abkf) sq ft		14.8-16.7	Range:	14.8-16.7	Range:		Range:		Range:		
7. Bankfull Mean Velocity (Vbkf) fps	Mean:	4.4	Mean:	4.4	Mean:	*	Mean:	*	Mean:		
	Range:		Range:		Range:		Range:		Range:		
8. Bankfull Discharge, cfs (Obkf)	Mean:	<i>68.</i> 7	Mean:	68.7	Mean:	*	Mean:	*	Mean:	*	
	Range:		Range:		Range:		Range:		Range:		
9. Maximum Bankfull Depth (dmax) ft	Mean:	2.0	Mean:	2.0	Mean:	31	Mean:	2.0	Mean:	2.6	
	Range:	20-21	Range:	20-2J	Range:		Range:		Range:		
IO. Ratio of Low Bank Height to Max.	Mean:	12	Mean:	LD QI	Mean:	I.D	Mean:	1.4	Mean:	1.0	7
Bankfull Depth (Bhlow/dmax)	Range:	1,0-1,3	Range:		Range:		Range:		Range:	-	
II. Width of Flood Prone Area (Wfpa) ft	Mean;	33.5	Mean:	33.5	Mean:	40.0	Mean:	30.0	Mean:	30,0	7
-	Range:	27.0-40.0	Range:	27.0-40.0	Range:		Range:		Range:		
12. Entrenchment Ratio (Wf pa/Wbkf)	Mean:	2.9	Mean:	2.9	Mean:	1,9	Mean:	32	Mean:	2.0	7
	Range:	21-37	Range:	21-37	Range:		Range:		Range:		
13. Meander Length (Lm) ft	Mean:	83.9	Mean:	83.9	Mean:	*	Mean:	x	Mean:	z	7
······································		55.8-111.9	Range:	55.8-111.9	Range:		Range:		Range:		
14. Ratio of Meander Length to	Mean:	7.0	Mean:	7,0	Mean:	*	Mean:	*	Mean:	#	
Bankfull Width (Lm/Wbkf)	Range:	47-9.3	Range:	47-9.3	Range:		Range:		Range:		
15. Raduis of Curvature (Rc) ft	Mean:	29.0	Mean:	29.0	Mean:	*	Mean:	*	Mean:	*	-
		15.0-55.0	Range:	150-550	Range.		Range:		Range:		
16. Ratio of Radius of Curvature to	Mean:	2.4	Mean:	2.4	Mean:	*	Mean:	*	Mean:	*	-
Bankfull Width (Rc/Wbkf)	Range:	12-46	Range:	12-46	Range:		Range:		Range:		
I7. Belt Width (Wbit) ft	Mean:	24,5	Mean:	24.6	Mean:	*	Mean:	*	Mean:	x	-
		13.5-337	Range:	13.5-33.7	Range:		Range:		Range:		
18. Meander Width Ratio (Wblt/Wbkf)	Mean:	20	Mean:	2.0	Mean:	*	Mean:	*	Mean:	*	-
	Range:	IJ-28	Range:	IJ-2.8	Range:		Range:		Range:		
19. Sinuosity (Stream length/valley	Mean:	11	Mean:		Mean:	*	Mean:	*	Mean:	*	
distance) (k)	Range:	N/A	Range:	NZA	Range:		Range:		Range		
20. Valley Slope (ft/ft)	Mean:	0.0106	Mean:	0.0106	Mean:	*	Mean:	*	Mean:	*	7
	Range:	N/A	Range:	N/A	Ranae:		Range:		Range:		
21. Average Water Surface Slope or	Mean:	0.0095	Mean:	0.0095	Mean:	#	Mean:	#	Mean:	*	1
Bankful Slope for Reach (Sbkf or	Range:	N/A	Range:	N/A	Range:		Range:		Range:		1
Sovg)=(Svalley/k) ft / ft	nonyor				, iongo:		, nongor		, ionge:		1
22. Pool Slope (Spool) ft / ft	Mean:	0.0077	Mean:	0.0077	Mean:	*	Mean;	*	Mean:	*	1
		0,0-0,0174	Range:	0.0-0.0174	Range:		Range:		Range:		
23. Ratio of Pool Slope to Average	Mean:	0.8	Mean:	08	Mean:		Mean:	*	Mean:	£	1
Slope (Spool/Sbkf)	Range:	0,0-1,8	Range:	0,0-1,8	Range:		Range:	-	Range:		
24. Maximum Pool Depth (dpool) ft	Mean:	2.4	Mean:	2.4	Mean:	*	Mean:	*	Mean:		-
	Range:	2.7	Range:	2.7	Range:	-	Range:	-	Range:	~	
25. Ratio of Maximum Pool Depth to	Mean:	1.8	Mean:	18	Mean:	x	Mean:	*	Mean:	z	4
Bankfull Mean Depth (dpool/dbkf)	Range:	10	Range:	10	Range:	-	Range:	-	Range:	-	
26. Pool Width (Wpool) ft	Mean:	6,	Mean:	6.1	Mean:	*		*	Mean:	*	4
	меол: Range:	01	Range:	40	Range:	-	Mean:	-		-	
	nunyo:		nunge:		NUNGE		Range:		Range:	· · ·	
Geomorphic data not collected due to existing design constraints. The majority of the work on site will	G	AR	RCA	DIS							CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RE ECOSYSTEM ENHANCEMENT PROGRAM
consist of Enhancement I. Ranges	ALA	G	& M of No	rth Carolina.I	nc.		5				
or restoration areas were developed		W	WW.ARCA	ADIS-US.CO	DM		4				LITTLE ALAMANCE CREEK
around the existing site constraints.		e Center Dri	ive, Suite 30	00			3 2 WW0	1/25/08	RESTORATION PLAN		ALAMANCE COUNTY, NORTH CAROLINA
-	Raleigh, NC 2	7607-5073					t REB		ORAFT RESTORATION		

Variables	Existing		Propose	d Reach	Referenc	e Reach	Reference	Reach	Reference	Reach
27. Ratio of Pool Width to Bankfull	Mean:	0.5	Mean:	0.5	Mean:	*	Mean:	z	Mean:	*
Width (Wpool/Wbkf)	Range:		Range:		Range:		Range:		Range:	
28. Bankfull Cross-sectional Area at	Mean:	9.5	Mean:	9.5	Mean:	*	Mean:	*	Mean:	*
Pool (Apool) sq ft	Range:		Range:		Range:		Range:		Range:	
29. Ratio of Pool Area to Bankfull	Mean:	6.0	Mean:	3.0	Mean:	*	Mean:	*	Mean:	
Area (Apool/Abkf)	Range:		Range:		Range:		Range:		Range:	
30. Pool to Pool Spacing (p-p) ft	Mean:	34J	Mean:	34,1	Mean:	*	Mean:		Mean:	z
	Ranae:	23.4-54.8	Range:	23.4-54.8	Range:		Range:		Range:	
31. Ratio of Pool-to-Pool Spacing to	Mean:	2.8	Mean:	28	Mean:	*	Mean:		Mean:	z
Bankfull Width (p-p/Wbkf)	Range:	20-46	Range:	20-46	Range:		Range:		Range:	
32. Pool Length (Lp) ft	Mean:	18.2	Mean:	18,2	Mean:	*	Mean:	*	Mean:	x
	Range:	4.0-163.0	Range:	4.0-163.0	Range:		Range:		Ranae:	
33. Ratio of Pool Length to Bankfull	Mean:	15	Mean:	15	Mean:	*	Mean:	*	Mean:	x
Width (Lp/Wbkf)	Range:	0.3-13.6	Range:	0.3-13.6	Range:		Range:		Range:	
34. Riffle Slope (Sriff) ft / ft	Mean:	0.0252	Mean:	0.0252	Mean:	*	Mean:	*	Mean:	
		0,0145-0,0498	Rapae	0,0145-0,0498	Range:		Range:		Range:	
35. Ratio of Riffle Slope to Average	Mean:	2.6	Mean:	26	Mean:	# .	Mean:	*	Mean:	*
Slope (Sriff/Sbkf)	Range:	15-52	Range:	15-52	Range:		Range:		Range:	
36. Maximum Riffle Depth (driff) ft	Mean:	2.0	Mean:	2.0	Mean:	*	Mean:	*	Mean:	
	Range:	20-21	Range	2.0-2.1	Range:		Ranae:		Range:	
37. Ratio of Riffle Depth to Bankfull	Mean:	16	Mean:	16	Mean:	*	Mean:	*	Mean:	*
Mean Depth (driff/dbkf)	Range:	15-16	Range	15-16	Range:		Range:		Ranae:	
38. Run Slope (Srun) ft / ft	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
		N/A N/A	Range:	N/A	Range	-	Range:	-	Range:	
39. Ratio of Run Slope to Average	Range:	<u> </u>		N/A N/A	Mean:	*	Mean:		Mean:	*
	Mean:		Mean:	N/A N/A		•	Range:	-	Range:	-
Slope (Srun/Sbkf)	Range:	<u> </u>	Range:	N/A	Range:	*		x	Mean:	2
40. Maximum Run Depth (drun) ft	Mean:		Mean:		Mean:	•	Mean:	-	Range:	-
4 Datta of D a Death to Death 1	Range:	<u>N/A</u>	Range:	<u>N/A</u>	Range:	*	Range:	*		
41. Ratio of Run Depth to Bankfull	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	-	Mean:	•
Mean Depth (drun/dbkf)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	*
42. Slope of Glide (Sgl) ft / ft	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	
	Range:	N/A	Range:	<u> </u>	Range:		Range:		Range:	
43. Ratio of Glide Slope to Average	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	
Water Surface Slope (Sgl/Sws)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
44. Maximum Glide Depth (dgl) ft	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
45. Ratio of Glide Depth to Bankfull	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
Mean Depth (dgl/dbkf)	Range:	N/A	Range:	N/A	Range:		Range:	-	Range:	
46. Step Slope (Sst)	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	2	Mean:	2
· · ·	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
47. Ratio of Step Slope to Average	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
Water Surface Slope (Sst/Savg)	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
48. Maximum Step Depth (dst)	Mean:	N/A	Mean:	N/A	Mean:	*	Mean:	*	Mean:	*
	Range:	N/A	Range:	N/A	Range:		Range:		Range:	
49. Ratio of Step Depth to Bankfull	Mean:	N/A	Mean:	N/A	Mean:	#	Mean:	z	Mean:	*
Mean Depth (dst/dbkf)	Range:	N/A	Range	N/A	Range:		Range:		Range:	

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801 Corporate Center Drive, Suite 300 Raleigh, NC 27607-5073 Tel: 919/854-1282 Fax: 919/854-5448

G & M of North Carolina, Inc. WWW.ARCADIS-US.COM

* Geomorphic data not collected due to existing design constraints. The majority of the work on site will consist of Enhancement I. Ranges for restoration areas were developed around the existing site constraints.

e:1/25/2008 Name: g:\tro

Date



DESIGN ENGINEER

	NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES ECOSYSTEM ENHANCEMENT PROGRAM
	LITTLE ALAMANCE CREEK
AN .	ALAMANCE COUNTY, NORTH CAROLINA
ION PLAN	1
REVISION	MORPHOLOGICAL TABLE -PROUT- SHEET NO. 3C









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